# BENNING ROAD & BRIDGES TRANSPORTATION IMPROVEMENTS

# **COST ESTIMATES**

FINAL SEPTEMBER 2020



















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# 1.0 Introduction

# 1.1 Proposed Action Overview

The District Department of Transportation (DDOT), in conjunction with the Federal Highway Administration (FHWA), prepared this Section 106 evaluation to support Final Environmental Assessment (EA) for the proposed Benning Road and Bridges Transportation Improvements project (the proposed action) in northeast Washington, DC. The proposed action would: provide safety improvements; extend the H/Benning Streetcar service to the Benning Road Metrorail Station; and improve pedestrian and bicycle facilities along Benning Road between Oklahoma Avenue and the Benning Road Metrorail Station. FHWA is the lead federal agency for the EA, with DDOT (the Applicant) as the local sponsor. The Federal Transit Administration (FTA), the National Capital Planning Commission (NCPC), and the National Park Service (NPS) are cooperating agencies.

The proposed action qualifies as an eligible project for Federal-aid funding under 23 CFR § 810.102 Eligible projects. FHWA concurred with mass transit use of the Benning Road ROW in a letter to DDOT dated April 18, 2013. The proposed action is included in the National Capital Region Transportation Planning Board's adopted Transportation Improvement Program (TIP) and the 2016 Financially Constrained Long-Range Plan for the National Capital Region (CLRP).

This EA is a Federal document and was prepared in accordance with the National Environmental Policy Act of 1969 (NEPA), the Council on Environmental Quality (CEQ) regulations (40 CFR 1500-1508), FHWA's Environmental Impact and Related Procedures (23 CFR 771), FHWA's Technical Advisory Guidance for Preparing and Processing Environmental and Section 4(f) Documents (T6640.8A), FHWA's 2006 SAFETEA-LU Environmental Review Process: Final Guidance, Appendix A of 23 CFR part 450 titled Linking Transportation Planning and NEPA Processes, FTA's 2006 Transit Noise and Vibration Impact Assessment guidance, FHWA's Noise Regulations (23 CFR 772), and DDOT's Environmental Process Manual.

# 1.2 Purpose of the Memorandum

This memorandum was prepared to present and document the cost projections prepared as part of the Benning Road and Bridges Transportation Improvements Environmental Assessment (EA). These projections include both capital and operational costs for each of the Build Alternatives. There are also projections for both of the propulsion systems (wired and wireless), the streetcar vehicles, as well as the professional services that will be needed during future phases of project development.

## 1.3 Selection of DDOT's Preferred Alternative

The Draft EA was released for a 30-day public comment period on May 4, 2016 and a public hearing was held on May 19, 2016. The public and agencies were given the opportunity to review

and comment on the EA until June 2, 2016. Public and agency coordination efforts have continued since the Draft EA and public hearing. DDOT held an Open House for the EA on November 15, 2017. After thorough consideration of input received from the public and agencies after publication of the Draft EA and based on technical analyses and the evaluation of alternatives, DDOT has selected Build Alternative 2-Median Streetcar Alignment with wired propulsion as the Preferred Alternative.

# 2.0 Methodology

Preliminary budget-level cost estimates were prepared for each Build Alternative. These estimates include capital costs of roadway, bridge and streetcar elements/infrastructure, as well as the operating and maintenance (O&M) costs of streetcar service, and contingencies for design and construction management and inspection. Costs were based on available DDOT construction pricing, similar projects and engineering judgment. To ensure accuracy, quantities were derived directly from the conceptual designs for Build Alternative 1 and the Preferred Alternative.

Costs associated with utility impacts and relocation are not included in the estimate. Similarly, cost for right-of-way and support facilities have not been calculated and are not included in the cost estimate.

This report has been divided into three main sections:

- 1. Roadway and Bridge/Structural Improvement Capital Costs
- 2. Streetcar Capital Costs, using FTA Standard Cost Category (SCC) Format
- 3. Operating and Maintenance Cost

A summary of these costs<sup>1</sup> are listed below in **Table 1**.

Table 1: Cost Estimate Summary - Build Alternative 1 and the Preferred Alternative

	Build Alt	ernative 1	Preferred Alternative		
Item	(wired)	(wireless)	(wired)	(wireless)	
Roadway and Bridge Capital Costs	\$115,947,500	\$115,947,500	\$118,275,100	\$118,275,100	
Streetcar Capital Costs	\$62,215,900	\$60,970,400	\$59,810,400	\$59,803,000	
Total Capital Costs*	\$178,163,400	\$176,917,900	\$178,085,500	\$178,078,200	
Annual Operating and Maintenance Cost*	\$4,584,600	\$4,584,600	\$4,584,600	\$4,584,600	

Notes

\* Capital costs are one time expenditures for project construction and implementation

<sup>\*\*</sup> Operating and maintenance costs are ongoing expenditures to provide service

<sup>&</sup>lt;sup>1</sup> Capital Cost and Operating and Maintenance Cost estimates are preliminary and subject to change as project design is refined.

# 3.0 Roadway and Bridge/Structural Improvements

**Table 2** and **Table 3** detail the budget-level cost estimates for roadway and bridge capital costs for Build Alternative 1 and the Preferred Alternative, respectively.

Items associated with roadway improvements include pavement removal, roadway, sidewalk, streetscape, and traffic signals. Appropriate percentage factors were used to account for drainage, signing and striping, and non-streetcar related electrical and lighting. These improvements also include replacement of all bus stops, Americans with Disabilities Act (ADA) improvements and a pedestrian crossing at Kingman Island. Full depth pavement is assumed for entire project area. Roadside planting strip assumes one tree every fifty feet and the median landscape assumes use of perennials as opposed to woody/shrub treatment.

To facilitate streetcar infrastructure and improve pedestrian facilities, the bridges and structures within the project corridor require repair and/or replacement. Costs include retaining wall work, modifications to Bridge No. 52 (over Anacostia River) and Bridge No. 77 (over Kingman Lake), and full replacement of the Lorrain H. Whitlock Memorial Bridge over the CSX Railroad and DC-295 (Bridge No. 503). The cost for full replacement includes demolition and new substructure and superstructure.

These estimates include factors for engineering and construction management/construction inspection (CMI).

To account for construction contingencies and maintenance of traffic, appropriate percentage factors were used based on the project subtotal for roadway and bridge construction. These costs were applied to compute mobilization. Mobilization is calculated only for the roadway and bridge construction items using the DDOT formula for a project greater than one million dollars. Costs for design fee and construction management and inspection are computed using the construction subtotal inclusive of mobilization.

Table 2: Build Alternative 1 Roadway and Structures Cost Estimate

				Unit	
Item	Description	Unit	Quantity	Price	Total
1	Hard Surface Removal	SY	65,923	\$45	\$2,966,525
2	Median Removal	SF	23,537	\$20	\$470,740
3	Curb Removal	LF	22,852	\$15	\$342,780
4	Sidewalk Removal	SF	89,940	\$20	\$1,798,800
5	Full Depth Pavement	SY	50,524	\$150	\$7,578,567
6	Median Curb	LF	8,398	\$30	\$251,940
7	Curb & Gutter	LF	18,907	\$30	\$567,210
8	Curb Ramps	EA	41	\$500	\$20,500
9	Proposed Sidewalk	SY	12,916	\$45	\$581,215
10	Driveway/Parking Lots Entrances	EA	64	\$1,500	\$96,000
11	Bus Stops	EA	22	\$10,000	\$220,000
12	New Traffic Signals Major	EA	2	\$250,000	\$500,000
13	New Traffic Signals Minor	EA	1	\$150,000	\$150,000
14	Existing Traffic Signal Reconfiguration	LS	1	\$1,394,000	\$1,394,000
15	Pedestrian Crossing at Kingman Island	EA	1	\$30,000	\$30,000
16	Roadside Plant Strips	SF	33,771	\$10	\$337,710
17	Misc. Public Realm Improvements	LS	1	N/A	\$50,000
18	Median Landscaped	SY	3,735	\$20	\$74,700
19	Drainage (20% of roadway subtotal)	LS	1	N/A	\$3,486,137
20	Signing & Striping (5% of roadway subtotal)	LS	1	N/A	\$871,534
21	Electrical/Lighting (10% of roadway subtotal)	LS	1	N/A	\$1,743,069
					\$23,531,427
22	STRUCTURES				. , ,
	Retaining Wall	LS	1	\$500,000	\$500,000
	Bridge No. 52 (over Anacostia River)	LS	1	\$1,600,000	\$1,600,000
	Bridge No. 77 (over Kingman Lake)	LS	1	\$200,000	\$200,000
	Bridge No. 503 (Full Replacement)	SF	44,555	\$630	\$28,069,753
	TOTAL STRUCTURES CONSTRUCTION	COST			\$30,369,753
	SUBTOTAL CONSTRUCTION COST				\$53,901,180
23	CONSTR. AND ENG. CONTINGENCY				
	MOT	30% of 1	Items 1-22		\$16,170,354
	Contingency	30% of 1	Items 1-22		\$16,170,354
	Mobilization	DDOT I	Formula		\$4,342,094
	TOTAL CONSTRUCTION COST	•			\$90,583,983
24	ENGINEERING				
			\$4,529,199		
	Final Engineering 8% of Items 1-23		\$7,246,719		
	Construction Engineering 15% of Items 1-23		\$13,587,597		
TOTAL ENGINEERING COST				\$25,363,515	
	TOTAL ROADWAY & BRID	GE COST	Γ		\$115,947,498

Table 3: Preferred Alternative Roadway and Structures Cost Estimate

				Unit	
Item	Description	Unit	Quantity	Price	Total
1	Hard Surface Removal	SY	65,923	\$45	\$2,966,525
2	Median Removal	SF	23,537	\$20	\$470,740
3	Curb Removal	LF	22,852	\$15	\$342,780
4	Sidewalk Removal	SF	89,940	\$20	\$1,798,800
5	Full Depth Pavement	SY	55,358	\$150	\$8,303,733
6	Median Curb	LF	7,541	\$30	\$226,230
7	Curb & Gutter	LF	18,917	\$30	\$567,510
8	Curb Ramps	EA	41	\$500	\$20,500
9	Proposed Sidewalk	SY	12,764	\$45	\$574,365
10	Driveway/Parking Lots Entrances	EA	64	\$1,500	\$96,000
11	Bus Stops	EA	22	\$10,000	\$220,000
12	New Traffic Signals Major	EA	1	\$250,000	\$250,000
13	New Traffic Signals Minor	EA	0	\$150,000	\$0
14	Existing Traffic Signal Reconfiguration	LS	1	\$1,394,000	\$1,394,000
15	Pedestrian Crossing at Kingman Island	EA	1	\$30,000	\$30,000
16	Roadside Plant Strips	SF	38,094	\$10	\$380,940
17	Misc. Public Realm Improvements	LS	1	N/A	\$50,000
18	Median Landscaped	SY	2,841	\$20	\$56,813
19	Drainage (20% of roadway subtotal)	LS	1	N/A	\$3,549,787
20	Signing & Striping (5% of roadway subtotal)	LS	1	N/A	\$887,447
21	Electrical/Lighting (10% of roadway subtotal)	LS	1	N/A	\$1,774,894
TOTALE	TOTAL ROADWAY CONSTRUCTION COST				\$23,961,064
22	STRUCTURES				Ψ20,901,001
	Retaining Wall	LS	1	\$500,000	\$500,000
	Bridge No. 52 (over Anacostia River)	LS	1	\$1,600,000	\$1,600,000
	Bridge No. 77 (over Kingman Lake)	LS	1	\$200,000	\$200,000
	Bridge No. 503 (Full Replacement)	SF	45,591	\$630	\$28,722,538
TOTALS	STRUCTURES CONSTRUCTION COST			4000	\$31,022,538
	AL CONSTRUCTION COST				\$54,983,602
23	CONSTR. AND ENG. CONTINGENCY				1- 11
	MOT	30% of 1	Items 1-22	l	\$16,495,080.
	Contingency	1	Items 1-22		\$16,495,080.
	Mobilization	DDOT	Formula		\$4,428,688.
TOTAL C	CONSTRUCTION COST				\$92,402,451
24	ENGINEERING				. , ,
	Preliminary Engineering 5% of Items 1-23		\$4,620,123		
	Final Engineering 8% of Items 1-23		\$7,392,196		
	Construction Engineering	1	Items 1-23		\$13,860,368
TOTAL ENGINEERING COST				\$25,872,686	
	TOTAL ROADWAY & BRIDGE COST				\$118,275,137

# 4.0 Streetcar Capital Costs

Capital cost estimates for the streetcar were determined based on quantities associated with each Build Alternative and are presented in Federal Transit Administration (FTA) Standard Cost Category (SCC) Format. These costs are summarized in **Table 4** for wired and wireless options.

The estimate assumed purchase of three new streetcar vehicles to account for the service extension to the Benning Road Metrorail Station. Other items associated with streetcar costs are platforms (including fare collection), trackwork for both normal and special segments, propulsion system allowance, and miscellaneous infrastructure.

# 4.1 Category 10 - Guideways and Track Elements

This category includes the trackwork and slab for the streetcar. Values were used per linear foot based on the system wide streetcar studies. Items included: Single Track Feet Curbside Running, Single Track Feet Median Running, Single Track Feet on Aerial Structure, Single Track Feet (CBTC) -Dedicated Guideway, 25 Meter Turnout, 20 Meter Turnout, and Track Diamond.

# 4.2 Category 20 - Stops

This category includes side and center platforms and message boards for each platform type. Cost for fare collection is included in the platform cost.

# 4.3 Category 30 - Yard and Shop (Maintenance Facilities)

A battery shop would be needed for a wireless option. The DC Streetcar Car Barn Training Center yard may accommodate the facility. Assumed costs for engineering, construction, equipment, hazard material handling, testing and training, compliance, etc. would be at least \$1 million.

# 4.4 Category 40 - Sitework and special conditions

This category only includes maintenance of traffic and contractor mobilization associated with streetcar infrastructure construction. Construction costs for roadway and sitework, as well as bridge reconstruction, is accounted in separate cost estimates. Construction costs for utility relocations are not included in this cost estimate.

# 4.5 Category 50 – Systems

This category includes a lump sum cost for propulsion system, including traction power substations (TPSS), overhead elements for power and charging, and accommodation for miscellaneous streetcar infrastructure.

Table 4: Streetcar Capital Costs

ETA Classical Code Code (CCC)	Build Alte	rnative 1	Preferred Alternative	
FTA Standard Cost Categories (SCC)	(wired)	(wireless)	(wired)	(wireless)
10 GUIDEWAY & TRACK ELEMENTS	\$10,550,500	\$10,550,500	\$10,567,000	\$10,567,000
10.02 Guideway: At-grade semi-exclusive (allows cross-traffic) curb	\$8,310,000	\$8,310,000	\$0	\$0
10.02 Guideway: At-grade semi-exclusive (allows cross-traffic) median	\$377,500	\$377,500	\$8,684,000	\$8,684,000
10.04 Guideway: Aerial structure	\$835,500	\$835,500	\$835,500	\$835,500
10.01 Guideway: At-grade exclusive right-of-way	\$302,500	\$302,500	\$302,500	\$302,500
10.12 Track: Special (switches, 25 m turnouts)	\$0.00	\$0.00	\$170,000	\$170,000
10.12 Track: Special (switches, 20 m turnouts)	\$600,000	\$600,000	\$450,000	\$450,000
10.12 Track: Special (track diamond)	\$125,000	\$125,000	\$125,000	\$125,000
20 STATIONS, STOPS, TERMINALS, INTERMODAL	\$2,255,000	\$2,255,000	\$1,475,000	\$1,475,000
20.01 At-grade stop – side platform	\$1,880,000	\$1,880,000	\$0	\$0
20.01 At-grade stop – center platform	\$285,000	\$285,000	\$1,425,000	\$1,425,000
20.01 At-grade stop – message board equipment side platform	\$80,000	\$80,000	\$0	\$0
20.01 At-grade stop – message board equipment center platform	\$10,000	\$10,000	\$50,000	\$50,000
30 SUPPORT FACILITIES: YARDS, SHOPS, ADMIN. BLDGS	\$0	\$1,000,000	\$0	\$1,000,000
30.04 Storage Building - battery shop		\$1,000,000		\$1,000,000
40 SITEWORK & SPECIAL CONDITIONS (MOT and	\$10,000,000	\$10,000,000	\$10,000,000	\$10,000,000
Mobilization)*	\$10,000,000	\$10,000,000	\$10,000,000	\$10,000,000
40.08 Temporary Facilities and other indirect costs during construction	\$10,000,000	\$10,000,000	\$10,000,000	\$10,000,000
50 SYSTEMS	\$9,252,000	\$4,200,000	\$8,442,210	\$4,200,000
50.03 Traction power supply: substations	\$4,000,000	\$4,000,000	\$4,000,000	\$4,000,000
50.04 Traction power distribution: OCS - curb	\$5,052,000			
50.04 Traction power distribution: OCS - median			\$4,242,210	
50.06 System equipment	\$200,000	\$200,000	\$200,000	\$200,000
Construction Subtotal (10-50)**	\$32,057,500	\$28,005,500	\$30,484,210	\$27,242,000
60 ROW, LAND, EXISTING IMPROVEMENTS	\$0 (Not included)	\$0 (Not included)	\$0 (Not included)	\$0 (Not included)
70 VEHICLES (3)	\$12,000,000	\$16,500,000	\$12,000,000	\$16,500,000
70.01 Light Rail (streetcar)	\$12,000,000		\$12,000,000	
70.01 Light Rail (hybrid streetcar)		\$16,500,000		\$16,500,000

# Benning Road and Bridges Transportation Improvements Final Environmental Assessment: Appendix C – Cost Estimates

80 PROFESSIONAL SERVICES (applies to Cats. 10-50)	\$12,502,425	\$10,922,145	\$11,888,842	\$10,624,380
80.01 Project Development	\$2,244,025	\$1,960,385	\$2,133,895	\$1,906,940
80.02 Engineering	\$2,564,600	\$2,240,440	\$2,438,737	\$2,179,360
80.03 Project Management for Design and Construction	\$1,282,300	\$1,120,220	\$1,219,368	\$1,089,680
80.04 Construction Administration & Management	\$4,808,625	\$4,200,825	\$4,572,632	\$4,086,300
80.05 - 80.08 Other Soft Costs	\$1,602,875	\$1,400,275	\$1,524,211	\$1,362,100
Subtotal (10-80)	\$56,559,925	\$55,427,645	\$54,373,052	\$54,366,380
90 UNALLOCATED CONTINGENCY	\$5,655,993	\$5,542,765	\$5,437,305	\$5,436,638
TOTAL PROJECT COST (10-90)	\$62,215,918	\$60,970,410	\$59,810,357	\$59,803,018

### **Notes:**

<sup>\*</sup> Costs shown for Category 40 are only for MOT and mobilization related to streetcar costs. Please refer to separate Roadway and Bridge cost estimates for these sitework associated costs.

<sup>\*\*</sup> Utility relocation costs are NOT included in estimate

# 4.6 Category 60 - Right-of-way

At this time, no costs have been included for right-of-way as the design intent is to remain within public space. However, limited right-of-way may be required and will be determined in subsequent project phases.

# 4.7 Category 70 - Vehicles

For streetcars, a lump sum of \$4 million per vehicle in 2014 dollars was used for the wired option and \$5.5 million for the wireless option assuming a hybrid vehicle.

# 4.8 Category 80 - Professional Services

The following percentages of Category 10-50 costs (including contingencies) are used:

- PE and Planning 7%
- Final Design 8%
- Program Management 4%
- Construction Administration and Management 15%
- Other soft costs 5%

This totals 39%; the percentage being used in the system wide streetcar studies.

# 4.9 Category 90 - Unallocated Contingency

An unallocated contingency of 10% is used for Categories 10-80.

# 5.0 Operating and Maintenance Cost

The methodology used to compute Operating and Maintenance (O&M) costs is based on annualized cost per revenue hour and mile. As provided by DDOT, the 2009 cost per mile was \$5.23 and the 2010 cost per hour was \$216.81; both were escalated at 3% to 2017 values of \$6.63 and \$266.65, respectively. The total revenue hours for the streetcar extension are determined based on the number of streetcars, round trip (cycle) time divided by headway, required by period, multiplied by the hours per period and then annualized. The revenue miles are determined in an analogous manner. These hour and mile-based costs are summed to determine total annualized O&M cost.

The same methodology is used for wired and wireless operation. An O&M cost variance was not assumed for wireless operation associated with vehicle maintenance, energy storage equipment (batteries and supercapacitors) and charging catenary.

The following guidelines were used to determine operating costs for Build Alternative 1 and the Preferred Alternative:

- Headway:
  - o Service at 10 minute headways during all hours of streetcar operation.
- Hours of Operation:
  - o Monday-Thursday 6 AM to 12 AM
  - o Friday 6 AM to 2 AM
  - o Saturday 8 AM to 2 AM
  - o Sunday 8 AM to 10 PM
- Annualization basis (364 operating days):
  - o 204 weekdays
  - o 52 Fridays
  - o 52 Saturdays
  - o 56 Sundays

Annual operating and maintenance costs for the two Build Alternatives are summarized in **Table** 5.

Table 5: Annual Operating & Maintenance Cost Estimate

	Build Alternative 1	Preferred Alternative
Annual Revenue Miles	172,800	172,800
Unit Cost per Revenue Mile	\$6.63	\$6.63
Mileage Based Annual Cost	\$1,144,800	\$1,144,800
Annual Revenue Hours	12,900	12,900
Unit Cost per Revenue Hour	\$266.65	\$266.65
Hourly Based Annual Cost	\$3,439,800	\$3,439,800
Total Annual O&M Costs 2017 dollars	\$4,584,600	\$4,584,600

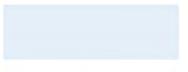
# BENNING ROAD & BRIDGES TRANSPORTATION IMPROVEMENTS

# MAINTENANCE OF TRAFFIC CONCEPT PLAN

FINAL SEPTEMBER 2020









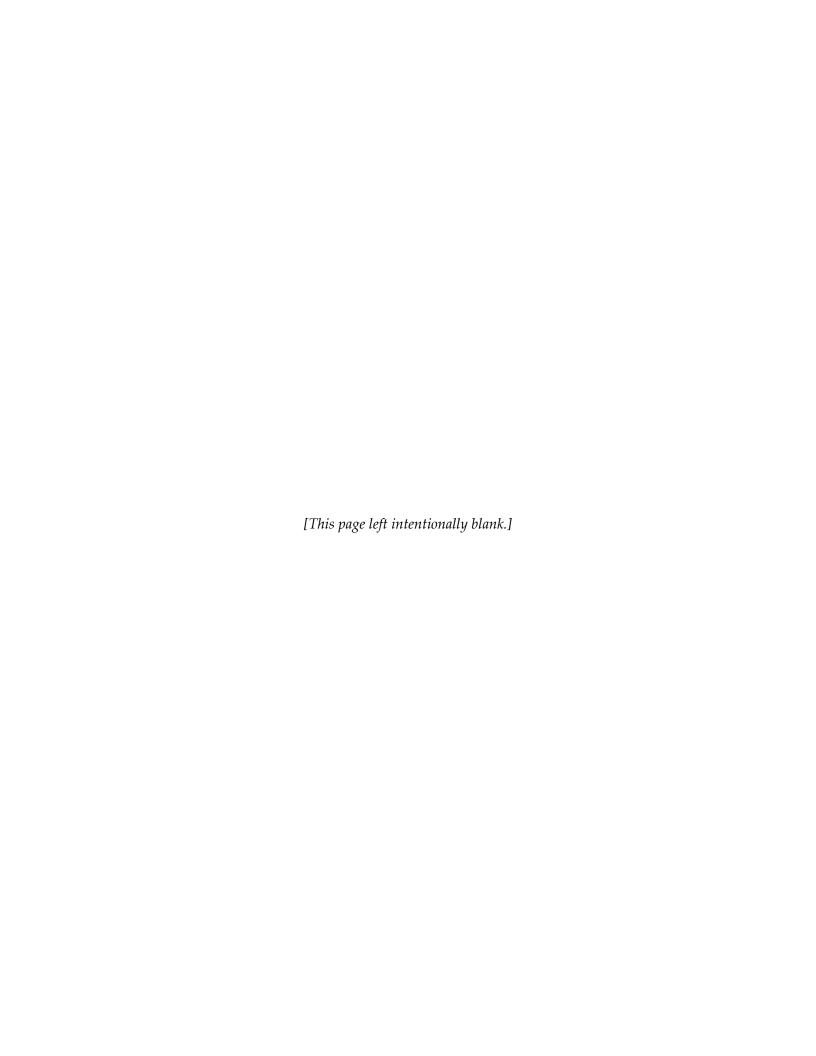






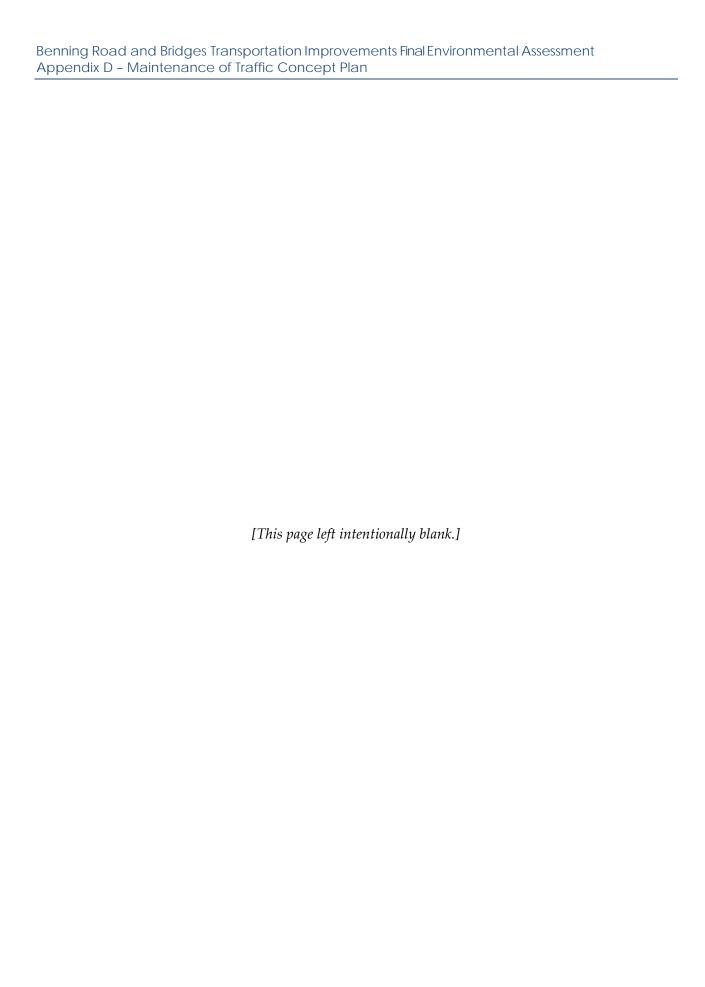






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# 1.0 INTRODUCTION

# 1.1 Proposed Action Overview

The District Department of Transportation (DDOT) in conjunction with the Federal Highway Administration (FHWA), prepared this Maintenance of Traffic Concept Plan to support the Final Environmental Assessment (EA) for the proposed Benning Road and Bridges Transportation Improvements project (the proposed action) in northeast Washington, DC. The proposed action would:

- provide infrastructure and safety improvements;
- extend the H/Benning Streetcar service to the Benning Road Metrorail Station; and
- improve pedestrian and bicycle facilities along Benning Road between Oklahoma Avenue and the Benning Road Metrorail Station.

FHWA is the lead federal agency for the EA, with DDOT (the applicant) as the local sponsor. The Federal Transit Administration (FTA), the National Capital Planning Commission (NCPC), and the National Park Service (NPS) are cooperating agencies.

The proposed action qualifies as an eligible project for Federal-aid funding under 23 CFR § 810.102 Eligible projects. FHWA concurred with mass transit use of the Benning Road ROW in a letter to DDOT dated April 18, 2013. The proposed action is included in the National Capital Region Transportation Planning Board's adopted Transportation Improvement Program (TIP) and the 2016 Financially Constrained Long-Range Plan (CLRP) for the National Capital Region.

This EA is prepared in accordance with the National Environmental Policy Act of 1969 (NEPA), the Council on Environmental Quality (CEQ) regulations (40 CFR 1500-1508), FHWA's Environmental Impact and Related Procedures (23 CFR 771), FHWA's Technical Advisory Guidance for Preparing and Processing Environmental and Section 4(f) Documents (T6640.8A), FHWA's 2006 SAFETEA-LU Environmental Review Process: Final Guidance, Appendix A of 23 CFR part 450 titled Linking Transportation Planning and NEPA Processes, FTA's 2006 Transit Noise and Vibration Impact Assessment guidance, FHWA's Noise Regulations (23 CFR 772), and DDOT's Environmental Process Manual.

# **1.2** Purpose of the Memorandum

This memorandum identifies the temporary traffic control measures DDOT is planning to use to maintain the movement of traffic during the construction of the improvements presented in the Benning Road and Bridges Transportation Improvements EA. The MOT Concept Plan describes key strategies and factors to facilitate traffic flow and safety through and around work zones for the Preferred Alternative. Temporary traffic impacts due to the construction of DDOT's Preferred Alternative include: extended travel times, reduced speed limits, temporary elimination of on-

street parking. Lane closures are also anticipated due to the replacement of the Lorraine H. Whitlock Memorial Bridges (Whitlock Bridge), track construction and intersection improvements.

The MOT Concept Plan provides details on how all modes (automobiles, transit, pedestrians, and bicycles) would be accommodated in each phase and address how access, parking, and loading/unloading operations would be provided or maintained. The Plan also discusses demolition actions, which provide a basis for determining the staging areas for the contractor of the major infrastructure elements. Staging is defined as the positioning of equipment and materials during construction. Phasing is defined as the sequencing of construction activities to complete the project. The following topics are the basis of the MOT Concept Plan:

- Traffic simulation methods;
- Staging areas;
- work hour limitations/restrictions;
- coordination with local churches;
- traffic operations at the intersection of Minnesota Avenue and Benning Road
- pedestrian safety and access;
- project coordination; and
- potential alternative routes.

DDOT is in the process of planning modifications of the Benning Road and DC-295 interchange via a separate Interchange Modification Report (IMR) process. The proposed interchange modifications would introduce a new ramp system that would increase safety and vehicular access to Benning Road and DC-295. It is expected that the proposed improvements will affect the construction and the overall traffic patterns on the Benning Road in the vicinity of DC-295 interchange. Therefore, this appendix includes construction sequencing and MOT measures for the Preferred Alternative, as well as those associated with the Benning Road and DC-295 Interchange project.

### 1.3 Selection of DDOT's Preferred Alternative

The project team released a Draft EA for a 30-day public comment period on May 4, 2016 and held a public hearing on May 19, 2016. The public and agencies were given the opportunity to review and comment on the EA until June 2, 2016. Public and agency coordination efforts have continued since the Draft EA and public hearing. DDOT held an Open House for the EA on November 15, 2017. After thorough consideration of input received from the public and agencies after publication of the Draft EA and based on technical analyses and the evaluation of alternatives, DDOT has selected Build Alternative 2, the median streetcar alignment with wired propulsion, as its Preferred Alternative.

# 2.0 Methodology

# 2.1 Key Modeling Assumptions

The traffic simulations presented in the MOT Concept Plan assume a 36-month construction period with no phasing. **Table D-1** presents the changes in roadway capacity expected to occur during this period. The future (2025) traffic volumes were developed using the MWCOG travel demand model (version 2.3.75), which was also used in the preparation of the Interchange Modification Report (IMR). Highway Capacity Software (HCS7) was used to analyze traffic conditions on freeways, ramps, and limited access arterial facilities. Synchro 10 was used to analyze arterial intersection traffic operations. In addition to capacity reductions within the project limits, the traffic simulations also account for changes in traffic volumes in surrounding areas as motorists attempt to bypass construction zones. The most prevalent alternate routes for traffic diversion are presented in **Attachment A**.

Table D-1. Capacity Reductions by Facility

Construction Location	Construction Impact to Existing Street Network	Duration of Impact
Benning Road (26th Street to 36th Street)	6/8 lanes to 4 lanes	Full duration (36 months)
Benning Road (36th Street to Minnesota Avenue)	4 lanes to 2 lanes	Full duration (36 months)
Benning Road (Minnesota Avenue to East Capitol Street)	4 lanes to 2 lanes	Full duration (36 months)
Northbound DC-295 (East Capitol Street interchange to Nannie Helen Burroughs Avenue interchange) <sup>1</sup>	3 lanes to 2 lanes	Full duration (36 months)
Southbound DC-295 (Nannie Helen Burroughs Avenue interchange to East Capitol Street interchange)	Remain 3 lanes	Full duration (36 months)
Ramp from Northbound DC-295 to westbound Benning Road	Closed	Full duration (36 months)
Ramp from Southbound Kenilworth Avenue to Northbound DC-295	Closed	Full duration (36 months)
Ramp from Eastbound Benning Road to Northbound DC-295	Remains 1 lane with short term closure	Full duration (36 months)
Ramp from Eastbound Benning Road to Southbound DC-295	Remains 1 lane with short term closure	Full duration (36 months)

<sup>&</sup>lt;sup>1</sup> The lane reduction on northbound DC-295 is required for reconstructing the Whitlock Bridge (#503) and the exact limits of lane closure will be determined in the design phase of the project. The assumption for the MOT traffic analysis represents a conservative approach.

# **2.2** Freeway Operations

Highway Capacity Software (HCS7) was used to analyze traffic conditions on freeways, ramps, and limited access arterial facilities. Traffic analyses were conducted using the Freeway Facilities methodology found in the 6th Edition of the Highway Capacity Manual (HCM) for both

directions of DC-295 between East Capitol Street and Nannie Helen Burroughs Avenue NE and the segment of East Capitol Street between C Street NE/Independence Avenue SE and Minnesota Avenue NE.

# 2.3 Arterial Operations

Traffic analyses were conducted using Synchro 10 to evaluate the anticipated construction impacts and proposed mitigations to the arterial network within MOT influence area. For the purposes of this analysis, the Synchro results for 30 signalized and unsignalized intersections were evaluated; these intersections are located along the following road segments:

- Benning Road NE (26th St NE to East Capitol St)
- Minnesota Avenue NE (Dix St NE to Nannie Helen Burroughs Ave NE)
- Nannie Helen Burroughs Ave NE/Deane Avenue NE (Lee St NE to 44th St NE)
- Kenilworth Avenue NE (Deane Ave NE to Foote St NE)
- Oklahoma Avenue NE (Benning Rd NE to C St NE)
- C Street NE (17th St NE to 21st St NE)
- East Capitol Street (19th St NE to 22nd St NE and Texas Ave SE to Benning Rd NE)
- 19th Street NE (Independence Ave SE to C St NE)

# 2.4 22nd Street NE (East Capitol St to C St NE)Staging Areas

To minimize the disruption caused by construction vehicles and equipment, the staging areas would be identified close to the road or structure that is under construction. Identifying appropriate locations is challenging due to the dense urban setting. Near Oklahoma Avenue, the site where the Capitol Grand Prix was held in Anacostia Park Section F, could be used for staging or storage. Additionally, open private land to the north of Benning Road near 42<sup>nd</sup> Street could be used through a negotiated agreement for temporary construction staging.

### 2.5 Work Hour Limitations/Restrictions

Although a construction schedule has not been developed, the proposed action would likely limit nighttime construction activities along certain sections of the project area, particularly in residential areas. However, night work may be necessary for girder erection and other activities that would otherwise require daytime lane closures, which would impact higher volumes of traffic compared to nighttime volumes. Provisions to allow night work could also shorten construction duration and limit overall inconvenience to the community.

### 2.6 Coordination with Local Churches

In consideration of active religious congregations within the project area, DDOT will coordinate with churches to avoid or minimize impacts to church activities. During initial public outreach, concern was raised that construction activities would impact several churches along Benning Road and funeral processions that use Benning Road for hearse loading and unloading. DDOT

will undertake an assessment of potential construction effects on churches and businesses during project design and construction planning to avoid or minimize impacts.

# 2.7 Minnesota Avenue Intersection Operations

Bridge reconstruction and track work installation near and through the intersection is expected to make certain turning movements temporarily inaccessible during construction. Likewise, utility relocations would also impact vehicular movements. Alternative routes would be established and clearly marked to minimize delay. Alternative routes have been identified and are described in **Section 4.0** of this document.

# 2.8 Pedestrian and Bicycle Safety and Access

Throughout construction, safe pedestrian access will be maintained. The south side of Benning Road supports a moderate amount of pedestrian traffic west of Minnesota Avenue. Both the north and south sides of Benning Road support pedestrian movement east of Minnesota Avenue. When the proposed action moves forward into design, DDOT will develop safety and access elements into the final design and construction requirements to prevent unmonitored pedestrian access. Safety fencing, signing, temporary curb ramps, and well-delineated, well-lit pedestrian paths will be provided as needed to safely accommodate pedestrian and bicycle traffic in the project area.

## 2.9 Maintenance of Access Plan

DDOT will develop and implement a Maintenance of Access (MOA) Plan to maintain pedestrian and vehicular access to residences, businesses, and other properties during construction. Bus stop access will be maintained during construction and will be addressed as part of the MOA Plan.

# 3.0 Summary of Construction Impacts & Mitigation

The construction of DDOT's Preferred Alternative is expected to increase congestion within the study area. This increase is primarily driven by two elements of the MOT plan:

- The reduction of capacity on both directions of Benning Road NE corridor and along northbound DC-295; and
- The ramp closure at the Benning Road NE and DC-295 interchange.

As noted earlier, these changes are expected to create an incentive for motorists to utilize alternative routes (see **Attachment A**). The impact summaries which follow incorporate these changes in traffic patterns.

# **3.1** Freeway Operations

### 3.1.1 DC 295

### 3.1.1.1.1 Southbound - AM & PM Peak

Traffic volumes on southbound DC-295 are forecasted to increase by fewer than 100 vehicles on all segments during the AM peak hour and decrease by up to 150 vehicles between Benning Road NE and East Capitol Street during the PM peak hour due to travel pattern shifts associated with construction impact. For example, lane reductions on Benning Road NE are projected to divert traffic away from Benning Road NE west of DC-295 in the westbound and eastbound direction during AM and PM peak, respectively. Accordingly, speed and density differ minimally under 2025 No-Build and MOT volumes. As such, impacts to operations on southbound DC-295 during construction are likely to be negligible.

### 3.1.1.1.1.2 Northbound - AM Peak

Traffic volumes on northbound DC-295 are forecasted to decrease by nearly 200 vehicles south of the Benning Road NE interchange during the AM peak hour due to travel pattern shifts associated with construction impacts (e.g., lane reduction on Benning Road NE and closure of the northbound DC-295 off-ramp to Benning Road). However, with the closure of the northbound DC-295 offramp to westbound Benning Road, traffic volumes north of the interchange are expected to increase by up to 100 vehicles as a result of diverted trips. Given the closure of one northbound through lane on DC-295, operations are expected to deteriorate within and upstream of the two-lane section between East Capitol Street and Benning Road. The merge area associated with the on-ramp from Benning Road is likely to produce more turbulence in the traffic stream under the proposed lane closure configuration. Where the freeway currently operates at LOS C with near free flow speeds during the AM peak hour, speeds are expected to decrease to less than 30 mph with densities of approximately 55 passenger cars per mile per lane. The HCS software estimates that these congested conditions may extend upstream of the East Capitol Street interchange. It should be noted that the limits of NB DC-295 lane closure (from three lanes to two lanes) will be determined in the 65% and final design phase of the project; the limits of lane closures assumed in the MOT analysis is conservative and the segment impacts could be shorter (e.g., just upstream and downstream of Benning Road NE and DC-295 interchange).

### 3.1.1.1.3 Northbound – PM Peak

Traffic volumes on northbound DC-295 are expected to decrease by 400-500 vehicles across all segments during the PM peak hour, primarily driven by diverted trips to alternative routes. The diversion of trips is based on the MWCOG model results. Despite this large decrease in volume, operations under 2025 MOT conditions within and upstream of the proposed freeway lane closure are likely to be impacted substantially. On segments where the freeway currently operates with speeds less than 15 mph and densities greater than 100 passenger cars per mile per lane—largely due to downstream oversaturated conditions—both measures (i.e., speed and density) are expected to improve slightly over the 2025 No-Build conditions. However, the increase in speeds and decrease in density north of the Benning Road interchange are a result of metered traffic flow

through the work zone; the formation of a new bottleneck at the freeway lane closure is expected to propagate congestion upstream of the East Capitol Street interchange.

# **3.2** Arterial Operations

The Synchro analysis of arterial operations indicates that the reduction of one lane on Benning Road will result in significant delay and congestion compared to the 2025 No-Build conditions. **Table D-2** lists the intersection where delay is expected to change substantially.

Table D-2. Intersections with Significant Change in Synchro Intersection Delay [sec]

Intersection	Change in Intersection Delay Relative to the 2025 No-Build		
	AM	PM	
Anacostia Avenue NE at Benning Road NE	+56		
34th Street NE at Benning Road NE	+27		
Minnesota Avenue NE at Benning Road NE		+21	
42nd Street NE at Benning Road NE	+38	+83	
44th Street NE at Benning Road NE	+171	+22	
East Capitol Street at Benning Road NE	+31	+50	
Kenilworth Terrace NE at Deane Avenue NE	+40		
17th Street NE at C Street NE	+25		
19th Street NE at Independence Avenue SE		+23	

Note: Positive change in delay indicates wait times will worsen under the MOT Build condition. Negative change (--) indicates a relative reduction.

# 3.3 Recommended Mitigation

# 3.3.1 Segment Breakdown and Phasing

The traffic simulations prepared in support of this plan assume that all the MOT measures required to accommodate construction will occur simultaneously. As a result, the modeled impacts represent worst-case conditions. One of the principal ways DDOT will reduce the severity of these impacts is to phase the construction process. Since the scope of the construction process varies across project area, the phasing scheme is organized based on project segments (see **Figure D-1**).

Segment A includes the section of Benning Road between 26th Street and 36th Street. In this area, Phase 1 involves constructing: the raised median; the streetcar tracks; and the proposed roadway improvements. During this period, two lanes of traffic will be maintained in both travel directions, as well as a sidewalk outside on existing roadway. Phase 2E and Phase 2W will include the remaining eastside and westside work. Phase 3 will include the work required to finish the raised median and streetcar stop, pavement markings, and signage.

Figure D-1. Project Area Segments



Segment B includes the section of Benning Road between 36th Street and Minnesota Avenue, as well as approximately 3,700 ft of DC 295 (2,400 ft south of Benning Road and 1,300 ft to the north). In this area, Phase 1 involves constructing the eastbound (EB) portion of the Whitlock Bridge. During this process, the existing westbound (WB) structure will be modified to support two-way traffic (one lane each). Pedestrian access will be maintained on the EB structure. Phase 2 includes constructing the WB structure. While this is occurring, vehicular traffic will be shifted to the newly constructed EB structure. During this period, EB and WB traffic will each be provided one lane. Northside roadway approaches and retaining walls will be constructed along with the WB bridge. Phase 3 will include constructing: the raised median, pavement markings, and signage. Work along DC-295 will be completed as apart of Segment B, see Figure D-2 for an illustration of how this work will be competed. For more information on these improvements please see Appendix M of the Benning Roads and Bridges Transportation Improvements Final EA.

Segment C includes the section of Benning Road from Minnesota Avenue to East Capitol Street. In this area, Phase 1 involves completing the proposed improvements to the EB portion of Benning Road. Two-way traffic (one-lane each) would be maintained on the remaining portion of Benning Road. Phase 2 involves shifting traffic to the southside of Benning Road and completing the proposed improvements to the road's WB section. Phase 3 will include constructing: the raised median, streetcar stops, pavement markings, and signage.



Figure D-2. DC 295 - Benning Road Interchange Improvements Phasing Diagram

# 3.3.2 Freeway Operations

For DC-295, it is recommended that at the minimum the merge from eastbound Benning Road onramp to northbound DC 295 be extended to meet AASHTO design standard. To further mitigate the impact to northbound DC 295 traffic during MOT, the freeway lane schematics can be temporarily laid out in a way that the merge from Benning Road on-ramp becomes an add-lane to join the two-lane northbound DC 295 mainline. This may be implemented after the completion of construction phasing of the Whitlock Bridge.

# 3.3.3 Arterial Operations

Based on the current traffic simulation, the following actions are recommended to mitigate the predicted increase in arterial congestion:

- East Capitol Street and Benning Road: provide a dedicated eastbound left-turn lane
- 44th Street and Benning Road: provide a dedicated eastbound left-turn lane
- 42nd Street and Benning Road: provide dedicated eastbound and westbound left turn lanes
- 40th Street and Benning Road: provide a dedicated westbound left-turn lane
- Minnesota Avenue and Benning Road: maintain current eastbound left-turn lane; improve signal operations as part of a corridor-wide signal timing adjustment
- 34th Street and Benning Road: maintain current eastbound / westbound left-turn lanes;
   improve signal operations as part of a corridor-wide signal timing adjustment

 Anacostia Avenue and Benning Road: provide dedicated eastbound and westbound leftturn lanes

# 4.0 Project Coordination

As described above, DDOT will develop and implement coordination, sequencing, and scheduling elements of the construction plan for the proposed action to avoid or minimize motorist delay and impacts on potentially affected businesses and communities. Coordination with non-highway transportation facilities including transit will also be part of the plan. Construction over a long period (36 months) would result in congestion and reduced access to residences and businesses in the vicinity of the project during that period. Emergency services (fire, ambulance, and police services) and bus services (school buses, WMATA buses, and handicapped and elderly transportation) could experience detours and slower travel during construction where access to or passage through existing roads might be restricted.

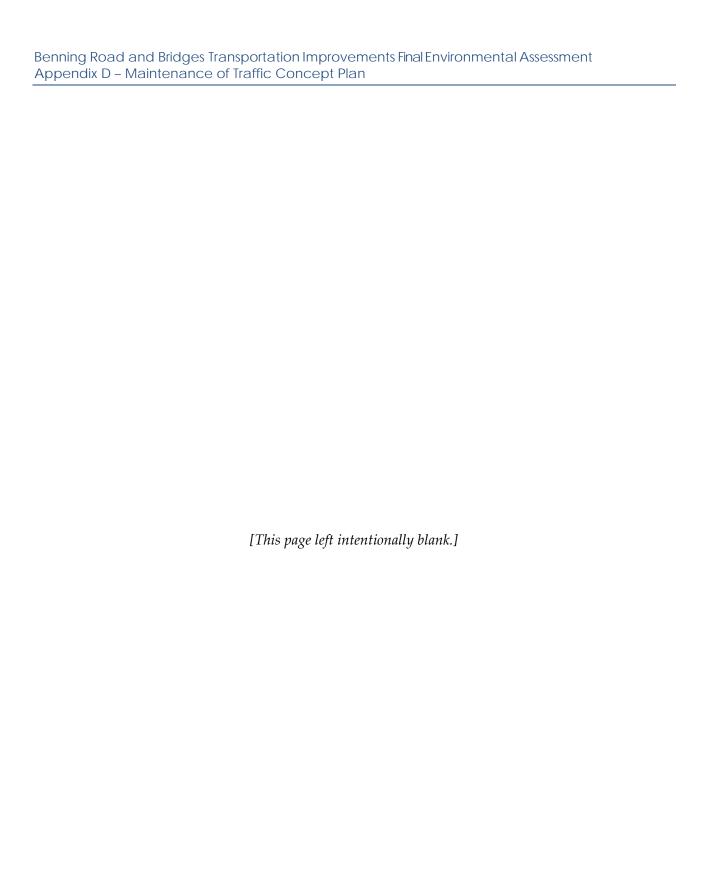
In accordance with DDOT's standard operating procedures, the construction contractor will develop a Construction Management Plan defining measures to minimize construction impacts on traffic. To the extent practicable, access to businesses and residences will always be maintained and at least a lane will be kept open to traffic unless alternative routes are provided. DDOT will provide advance notifications of proposed construction activities in the area via various media like internet, social media, boards etc. Advanced information that describes construction activities and provides alternative transportation routes will be made available to the public.

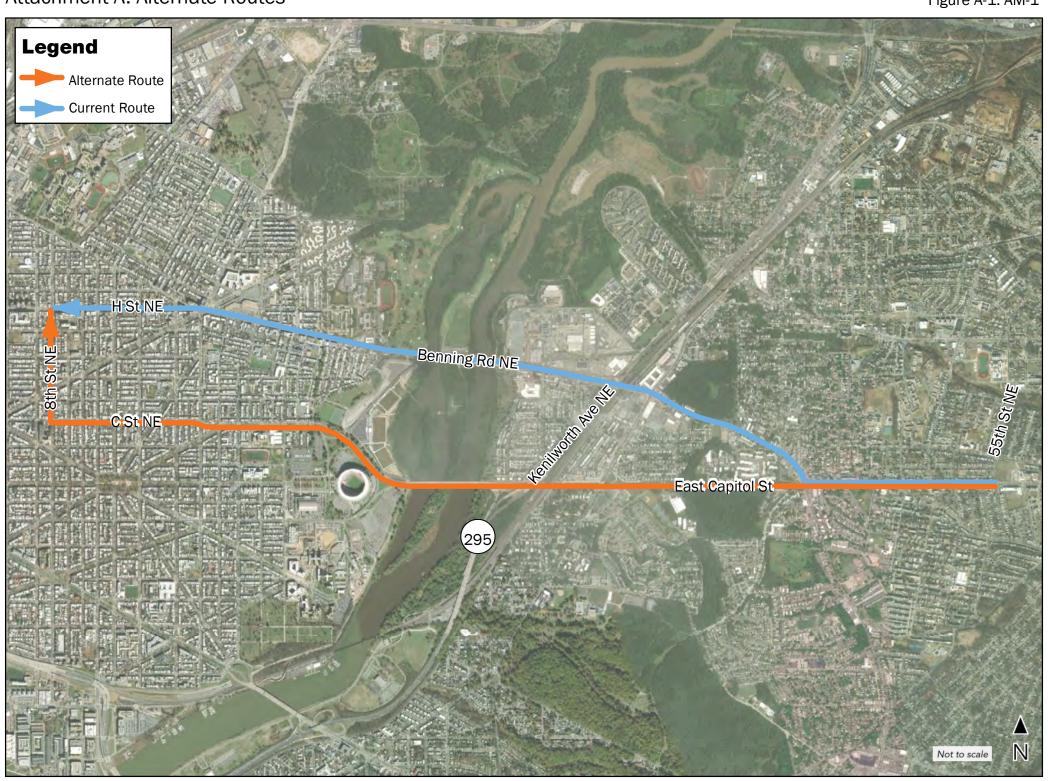
Even with the implementation of the MOT plan, traffic congestion will increase in the short term around the construction area. Lane closures will be short-term and limited to the closures that will be specified in the MOT plan as approved by DDOT before the start of construction. DDOT and the contractor will coordinate with emergency service providers such as fire protection, police and ambulance service, and other community facilities before and throughout the construction to ensure that access for their vehicles is maintained.

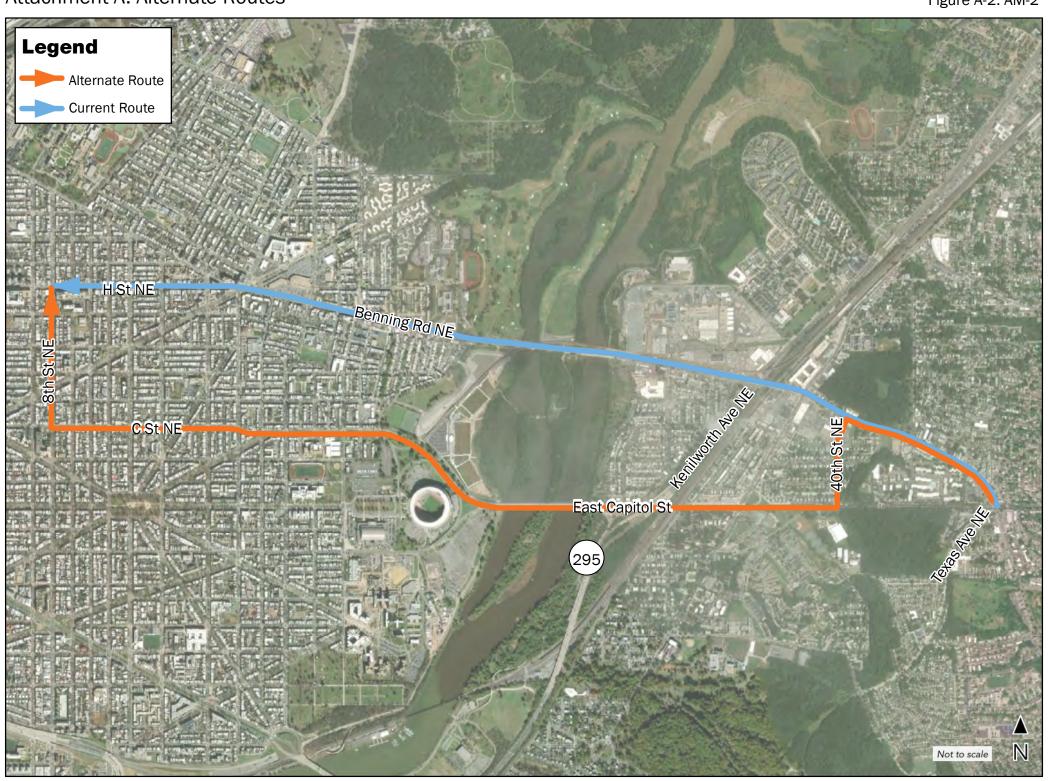
# 5.0 Attachment A

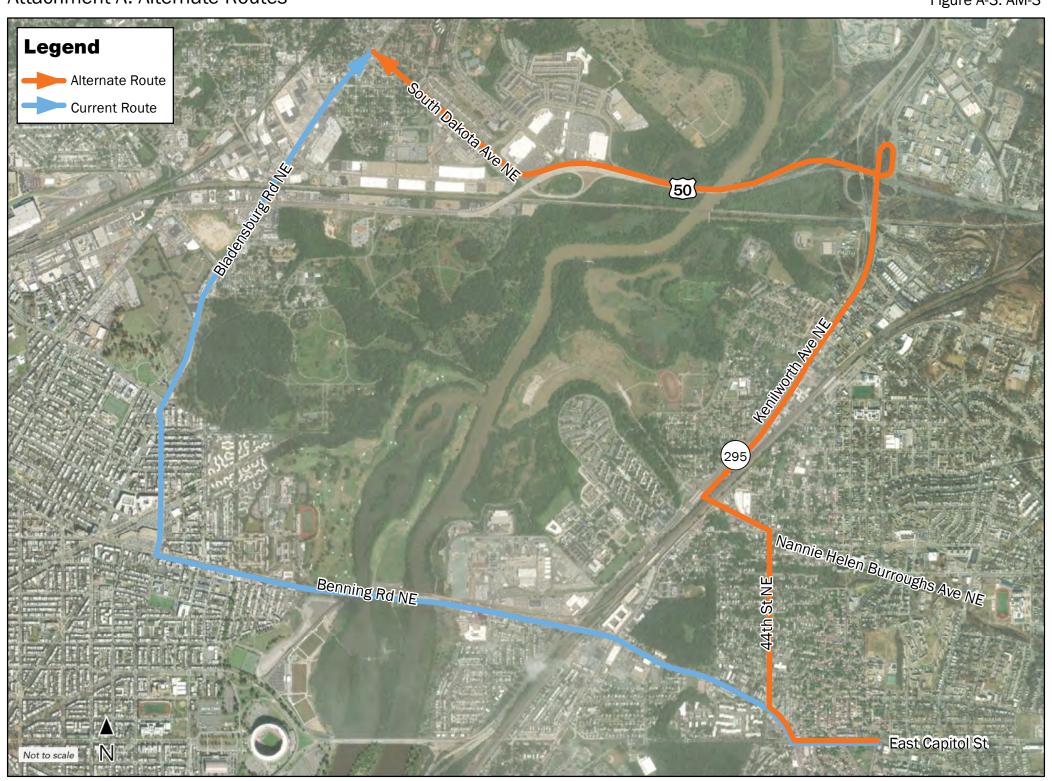
### **List of Attachments**

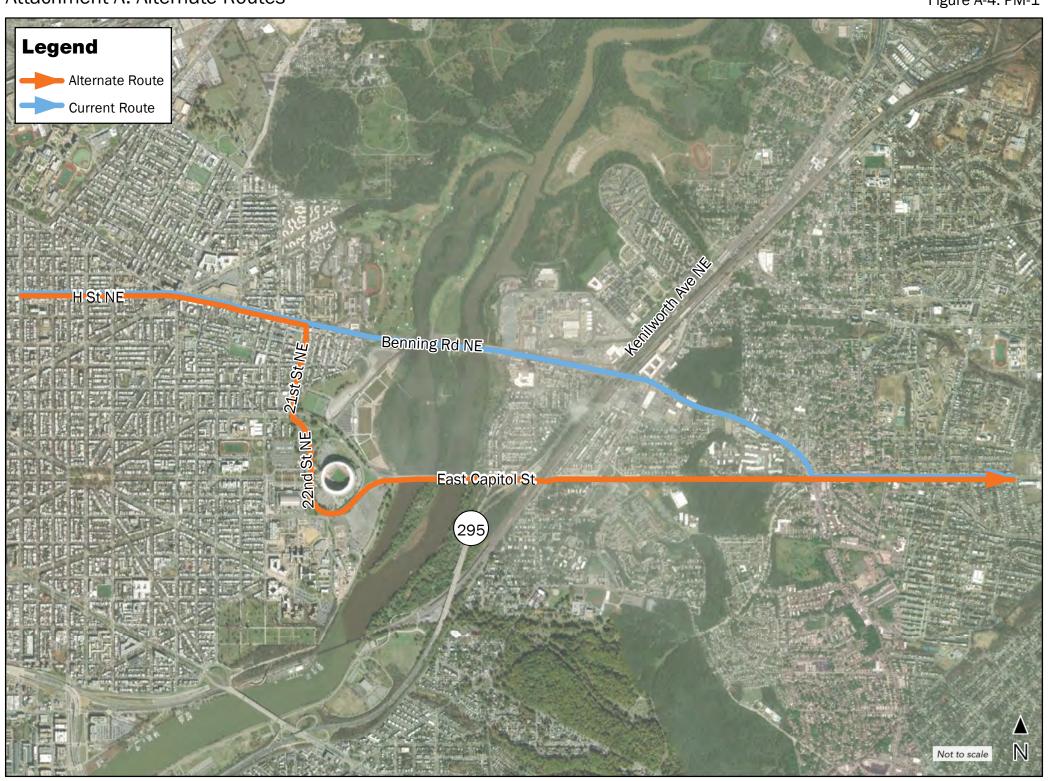
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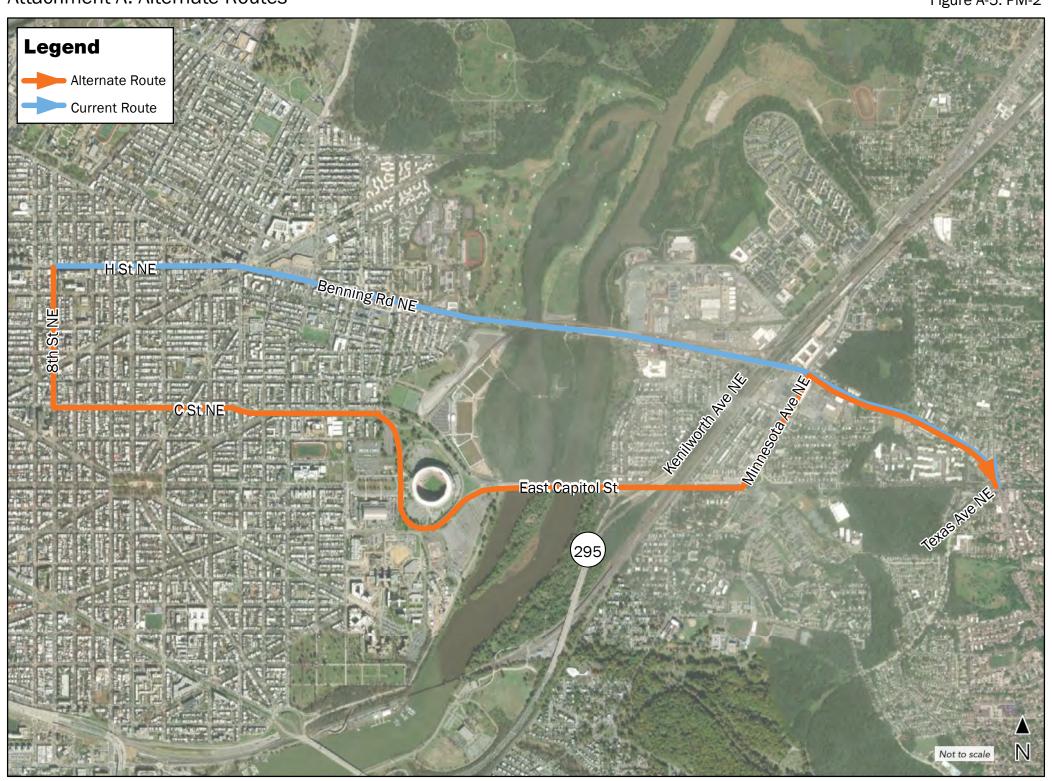


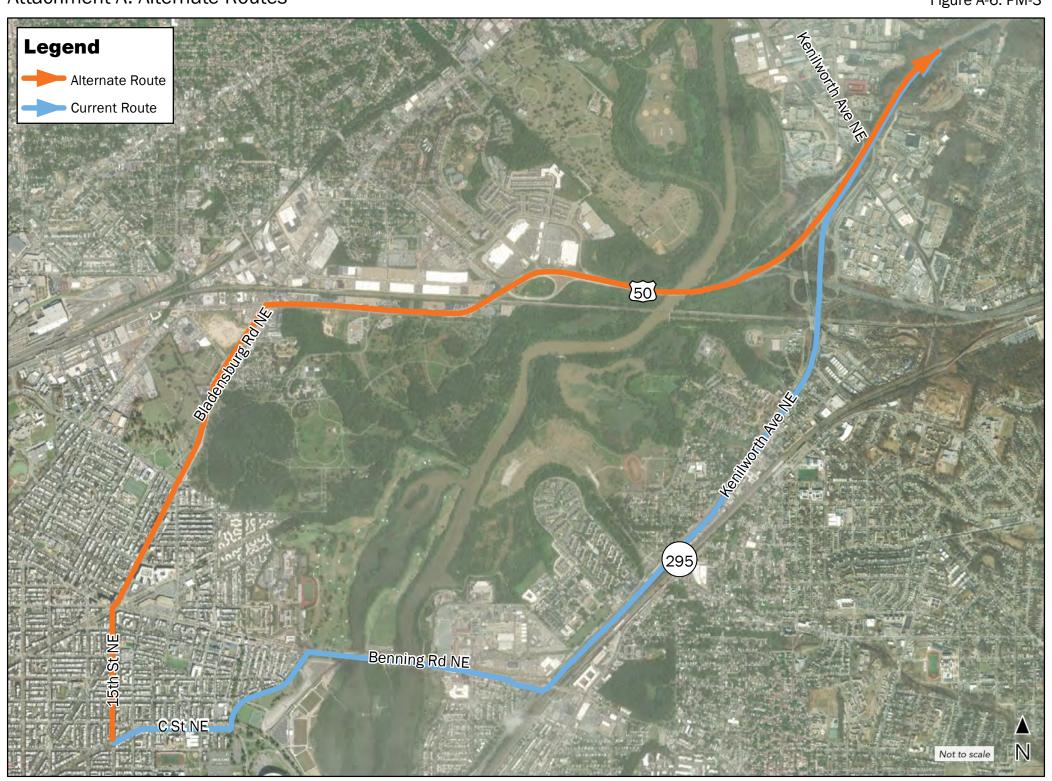


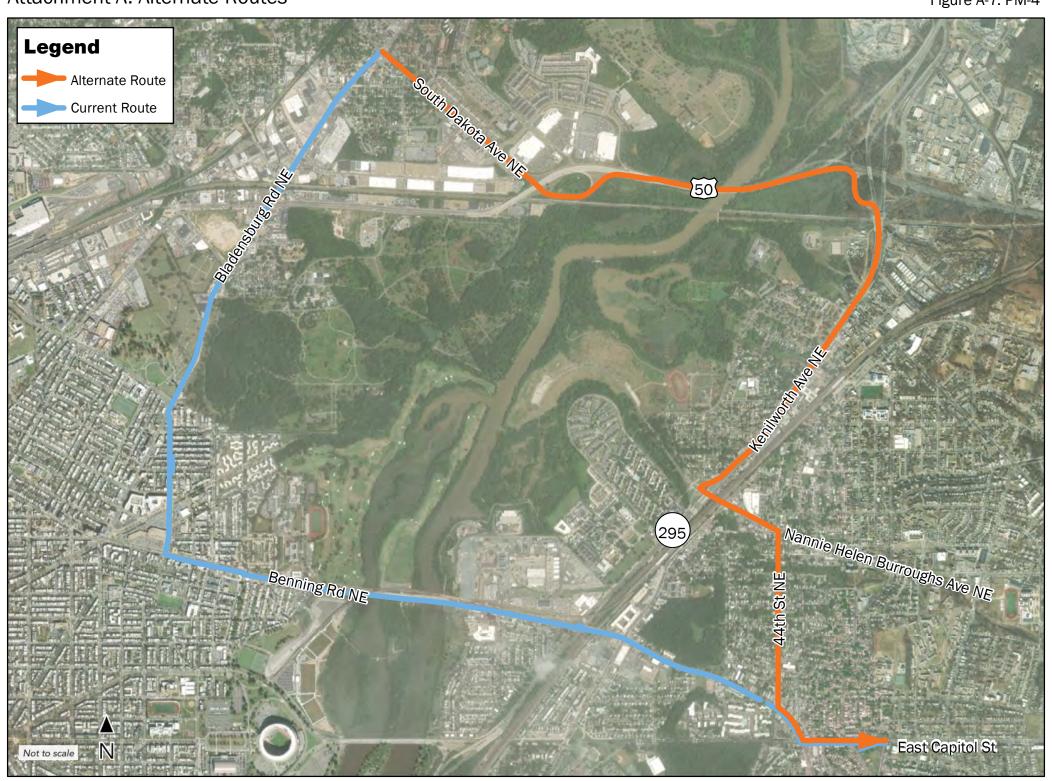


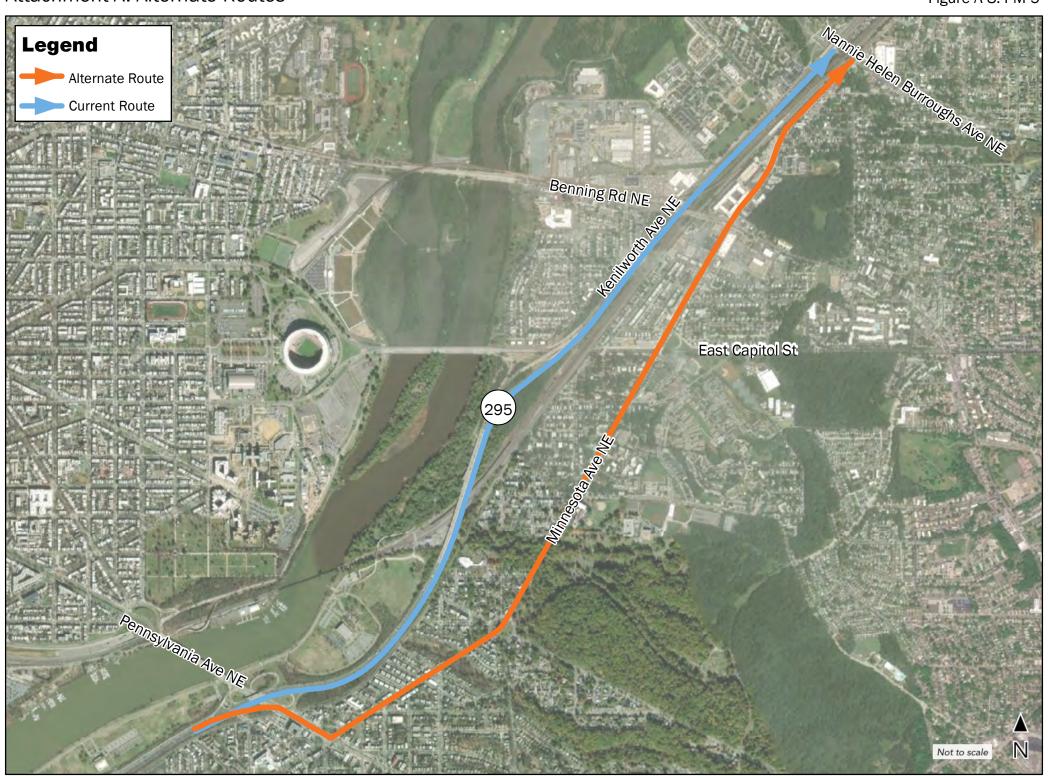


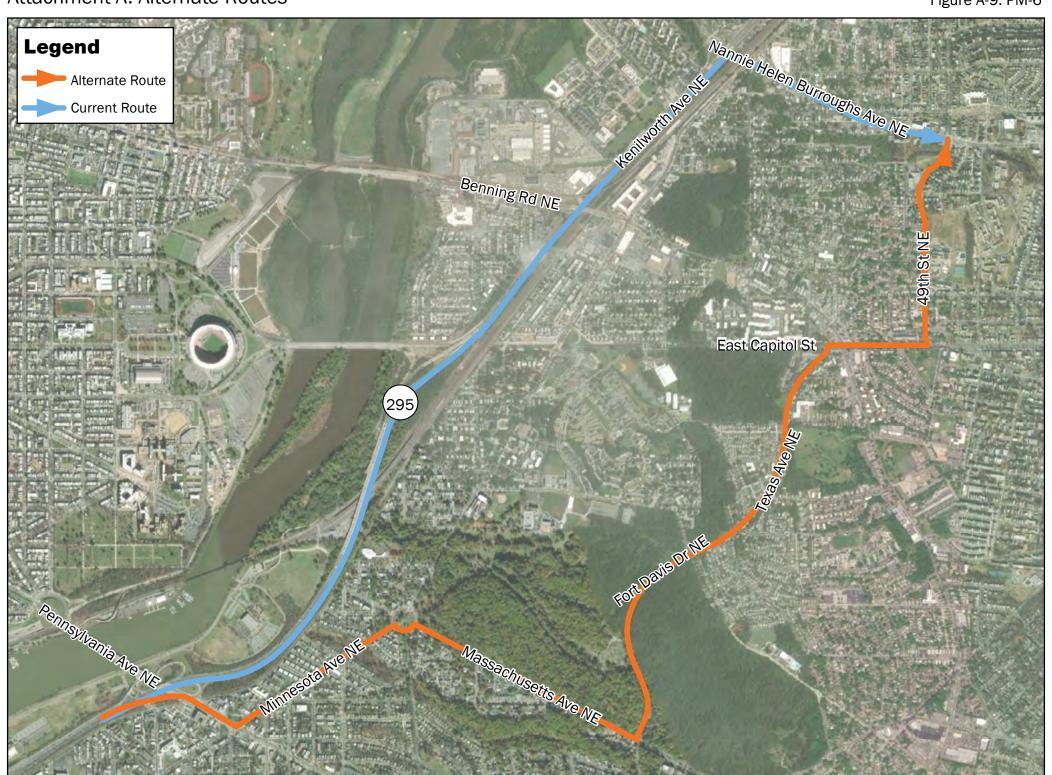












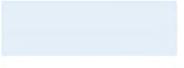
# BENNING ROAD & BRIDGES TRANSPORTATION IMPROVEMENTS

# FINAL TRANSPORTATION TECHNICAL MEMORANDUM

FINAL SEPTEMBER 2020





















# **Executive Summary**

The project team performed analyses for the existing condition year 2019, opening year 2025 and design year 2045 on the No Build and build Alternative 2 - Median Streetcar Alignment with wired propulsion scenarios. Alternative 2 was chosen to be the Preferred Alternative.

The freeway portion of the study network includes approximately three miles of DC-295, from just north of Pennsylvania Avenue SE to just south of US 50, and three interchanges – Nannie Helen Burroughs Avenue NE, Benning Road NE, and East Capitol Street. The arterial portion of the study network includes shorter segments of Deane Avenue NE, Nannie Helen Burroughs Avenue NE, Minnesota Avenue NE, and East Capitol Street, with a focus on the segment of Benning Road NE from 26th Street NE to East Capitol Street. In all, 18 signalized intersections and seven unsignalized intersections were included in the analysis.

Future scenarios include all the transportation improvement projects programmed in MWCOG's CLRP within the study area. To enhance the traffic forecasting accuracy using the MWCOG model, VISUM was used as a supplemental tool to assign traffic and develop forecasts with greater detail of the traffic analysis zones and street network. The two major inputs in the modeling process used in this analysis are land use data and transportation networks. The VISUM model was validated against peak period counts for freeways and arterials, specifically 6:00 to 9:00 AM and 3:00 to 7:00 PM, which is consistent with the peak periods of the MWCOG model. A network-wide representative peak hour was determined for the AM peak (7:45 AM to 8:45 AM) and PM peak (5:00 PM to 6:00 PM) using the traffic count data as well as the travel time data for the Benning Road corridor.

Some measures of performance considered when comparing the 2025 and 2045 build scenario to the 2025 and 2045 No-Build scenario were: LOS; Streetcar Ridership; and Bus Ridership along the entirety of the routes considered.

#### LOS

For the 2025 begin year, of the eight critical intersections (LOS E or F) considered, the build scenario decreased the number of LOS E intersections by one and increased the number of LOS F intersections by one, indicating that the build condition was slightly worse than the No-Build condition. For the 2045 design year, however, of the 12 critical intersections (LOS E or F) considered, the build scenario increased the number of LOS E intersections by two and decreased the number of LOS F intersections by two, indicating that the build condition was slightly better than the No-Build condition.

#### Streetcar Ridership

For the 2025 begin year, models project that the build scenario ridership will increase by 105% over the No-Build scenario, whereas in the 2045 design year, models project that the build

scenario ridership will increase by 112% over the No-Build scenario. The introduction of additional proposed stops account for over half of the ridership increase in both years.

#### Bus Ridership

For the 2025 begin year, models project that the build scenario ridership will decrease by approximately 4% compared to the No-Build scenario, whereas in the 2045 design year, models project that the build scenario ridership will decrease by approximately 6% compared to the No-Build scenario. This decrease is likely due to the introduction of the faster, more frequent Streetcar service, particularly during the off-peak periods because the Streetcar would operate with the same 10-minute headway during both the peak and off-peak, thereby reducing passenger waiting time between the Benning Road Metrorail Station and Union Station.

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### 1.0 Introduction

#### 1.1 Proposed Action Overview

The District Department of Transportation (DDOT), in conjunction with the Federal Highway Administration (FHWA), prepared this Section 106 evaluation to support Final Environmental Assessment (EA) for the proposed Benning Road and Bridges Transportation Improvements project (the proposed action) in northeast Washington, DC. The proposed action would: provide safety improvements; extend the H/Benning Streetcar service to the Benning Road Metrorail Station; and improve pedestrian and bicycle facilities along Benning Road between Oklahoma Avenue and the Benning Road Metrorail Station. FHWA is the lead federal agency for the EA, with DDOT (the Applicant) as the local sponsor. The Federal Transit Administration (FTA), the National Capital Planning Commission (NCPC), and the National Park Service (NPS) are cooperating agencies.

The proposed action qualifies as an eligible project for Federal-aid funding under 23 CFR § 810.102 Eligible projects. FHWA concurred with mass transit use of the Benning Road ROW in a letter to DDOT dated April 18, 2013. The proposed action is included in the National Capital Region Transportation Planning Board's adopted Transportation Improvement Program (TIP) and the 2016 Financially Constrained Long-Range Plan for the National Capital Region (CLRP).

This EA is a Federal document and was prepared in accordance with the National Environmental Policy Act of 1969 (NEPA), the Council on Environmental Quality (CEQ) regulations (40 CFR 1500-1508), FHWA's Environmental Impact and Related Procedures (23 CFR 771), FHWA's Technical Advisory Guidance for Preparing and Processing Environmental and Section 4(f) Documents (T6640.8A), FHWA's 2006 SAFETEA-LU Environmental Review Process: Final Guidance, Appendix A of 23 CFR part 450 titled Linking Transportation Planning and NEPA Processes, FTA's 2006 Transit Noise and Vibration Impact Assessment guidance, FHWA's Noise Regulations (23 CFR 772), and DDOT's Environmental Process Manual.

#### 1.2 Purpose of the Memorandum

This technical memorandum provides an overview of transportation analyses conducted for the Benning Road and Bridges Transportation Improvements Environmental Assessment (EA). This technical memorandum describes the technical methodologies and presents results of the traffic analysis, including the ridership projections for the proposed action. The rest of this technical memorandum is organized as follows:

- Methodology
- Existing Conditions
- Opening Year 2025 Conditions

Design Year 2045 Conditions

#### 1.3 Selection of DDOT's Preferred Alternative

The Draft EA was released for a 30-day public comment period on May 4, 2016 and a public hearing was held on May 19, 2016. The public and agencies were given the opportunity to review and comment on the EA until June 2, 2016. Public and agency coordination efforts have continued since the Draft EA and public hearing. DDOT held an Open House for the EA on November 15, 2017. After thorough consideration of input received from the public and agencies after publication of the Draft EA and based on technical analyses and the evaluation of alternatives, DDOT has selected Build Alternative 2 - Median Streetcar Alignment with wired propulsion as the Preferred Alternative. As a result of this selection, the traffic analyses of the curbside alternative (Alternative 1) were not updated to reflect the revised opening year (2025), design year (2045), or the most recent regional travel demand forecasts from MWCOG. They are included in this document for informational purposes only.

# 2.0 Methodology

This section summarizes the study area, key data, and modeling/ analysis methodology used in travel demand forecasting and traffic modelling for this project.

#### 2.1 Study Area Boundary

Error! Reference source not found. shows the project scope and analysis corridors and intersections. The freeway portion of the study network includes approximately three miles of DC-295, from just north of Pennsylvania Avenue SE to just south of US 50, and three interchanges – Nannie Helen Burroughs Avenue NE, Benning Road NE, and East Capitol Street. The arterial portion of the study network includes shorter segments of Deane Avenue NE, Nannie Helen Burroughs Avenue NE, Minnesota Avenue NE, and East Capitol Street, with a focus on the segment of Benning Road NE from 26th Street NE to East Capitol Street. In all, 18 signalized intersections and seven unsignalized intersections were included in the analysis. This should address the transit and traffic operation resulting from the design of the proposed streetcar extension and reconstruction of Benning Road, including pedestrian and bicycle facilities.

Future scenarios include all the transportation improvement projects programmed in MWCOG's CLRP within the study area. Note that the DC-295 interchange with Benning Road NE project is currently not in the CLRP; therefore, it has not been included in the future scenarios of this analysis. The DC-295 interchange with Benning Road NE represents one of the only few roadway crossings of the Anacostia River connecting the east and west of Washington, D.C. The limited number of waterway crossings, as well as the limited connections at the crossings and between them, results in substantial pressure on the Benning Road NE interchange to accommodate heavy traffic demands and creates significant congestion. Modifications are planned on the Benning

Road/ DC-295 interchange to improve connectivity, to eliminate unsafe conditions, and to manage traffic demands. The interchange improvement project would mainly include: correcting turning radius deficiencies on existing ramps; lengthen merge and weave areas; increase deceleration/acceleration distance for on/off-ramps; provide safe pedestrian/bicycle crossing; maintain structural integrity of bridges 104 and 104-1 that span over Kenilworth Ave; and increase number of movements at the interchange to provide connectivity to the communities located at the east of DC-295. DDOT is assessing the referenced Benning Road/DC-295 interchange improvements through a separate Interchange Modification Report (IMR) process. Alternative analysis is currently ongoing for the IMR. Development of No Build and Build traffic scenarios will follow once the Preferred Alternative for the IMR is finalized. It is expected that the proposed improvements will result in permanent changes to the overall traffic patterns on the Benning Road NE in the vicinity of DC-295 interchange. Therefore, in order to provide a full assessment of traffic scenarios in the study area, DDOT will prepare traffic analysis and Maintenance of Traffic (MOT) document which will evaluate Benning Road infrastructure improvements proposed in this EA, and DC-295 interchange improvements together in the IMR. Please note that this analysis only considers traffic impacts under the no-build and build conditions for the Benning Road infrastructure improvements only.

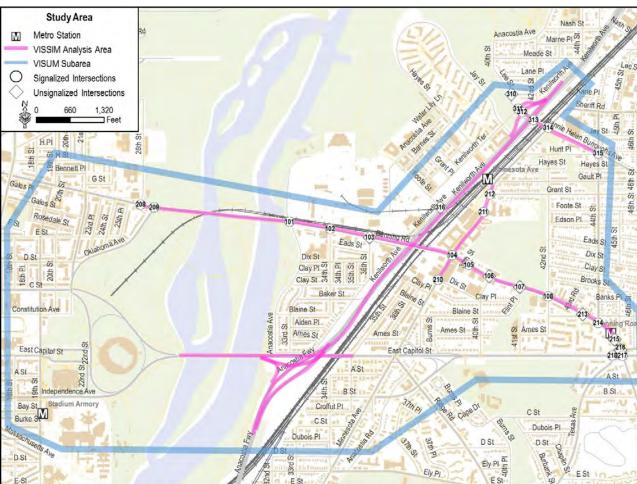


Figure 1: Transportation Study Area

The study area includes the following intersections:

- I-101: Benning Road NE at Anacostia Avenue NE
- I-102: Benning Road NE at 34th Street NE
- I-103: Benning Road NE Ramp to DC-295 at 36th Street NE
- I-104: Benning Road NE at Minnesota Avenue NE
- I-105: Benning Road NE at 39th Street NE/Driveway
- I-106: Benning Road NE at 40th Street NE
- I-107: Benning Road NE at 41st Street NE
- I-108. Benning Road NE at 42<sup>nd</sup> Street NE
- I-208. Benning Road NE at 26th Street NE
- I-209. Benning Road NE at Oklahoma Avenue NE
- I-210: Minnesota Avenue NE at Dix Street NE
- I-211: Minnesota Avenue NE at Bus Exit South
- I-212: Minnesota Avenue NE at Grant Street NE and Bus Entrance North
- I-213: Benning Road NE at Blaine Street NE
- I-214: Benning Road NE at 44th Street NE
- I-215: Benning Road NE at 45th Street NE
- I-216: Benning Road NE at Central Avenue NE
- I-217: Benning Road NE at East Capitol Street SE
- I-218: East Capitol Street SE at Texas Avenue SE
- I-310: Deane Avenue NE at Lee St NE
- I-311: Deane Avenue NE at Kenilworth Terrace NE
- I-312: Deane Avenue NE at Kenilworth Avenue NE
- I-313: Nannie Helen Burroughs Avenue NE at Kenilworth Avenue NE and DC-295 U-Turns
- I-314: Nannie Helen Burroughs Avenue NE at Minnesota Avenue NE
- I-315: Nannie Helen Burroughs Avenue NE at 44th Street NE and Hunt Place NE
- I-316: Kenilworth Avenue NE at Foote Street NE

#### 2.2 Data Collection and Process

The data used for traffic forecasting and analysis included the following:

- Traffic volumes (intersections, freeway/arterial mainlines, ramps)
- Streetlight O-D data
- Travel time data (field collected and Google crowdsourced data)
- Observed queue data
- Multimodal data (pedestrian/bicycle volumes, transit operations)

#### 2.2.1 Existing Traffic count data collection locations

Traffic count data was collected Tuesday through Thursday over the course of two weeks, from April 2<sup>nd</sup> - 4<sup>th</sup> and April 9<sup>th</sup> - 11<sup>th</sup>, 2019. The traffic count data collected included peak period intersection turning movement counts (TMCs) and 72-hour classification counts for arterial

segments, DC-295 ramps, and DC-295 mainline segments. **Figure 2** shows the intersection TMC and arterial classification count locations. **Figure 3** shows the freeway mainline and ramp classification count locations.

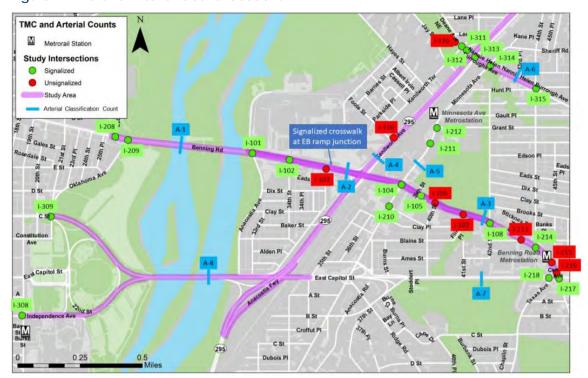
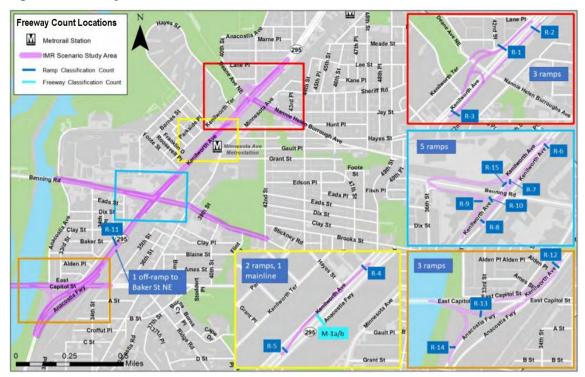


Figure 2: TMC and Arterial Count Locations



**Figure 3: Freeway Count Locations** 

#### 2.3 Modeling Methodology

MWCOG Regional Travel demand forecasts were prepared for the project area that reflect both the regional context and localized land use and transportation characteristics. These forecasts are used to test the general capacity and connectivity of the existing and planned transportation networks and serve as inputs to the detailed traffic simulation model. Detailed information regarding travel demand forecasting and traffic analysis is provided in the following sections.

#### 2.3.1 Travel Demand Forecast

#### 2.3.1.1 MWCOG Regional Travel Demand Model and VISUM Subarea Model

The travel demand forecasting process used the most currently adopted version of the travel demand model (Version 2.3.75) produced and distributed by Metropolitan Washington Council of Governments (MWCOG) and extracted a subarea model in VISUM (Version 18). At a high level, the MWCOG model was the source of traffic analysis zone (TAZ) land use inputs, growth rates, and a seeding O-D trip table that was refined in VISUM. The MWCOG model was calibrated to reflect travel conditions at a regional level. As such, the MWCOG model was validated to daily volumes using cut lines as opposed to roadway links following the requirements outlined in the FHWA Travel Model Validation and Reasonability Checking Manual Second Edition. To enhance the traffic forecasting accuracy using the MWCOG model, VISUM was used as a supplemental tool to assign traffic and develop forecasts with greater detail of the TAZs and street network. VISUM was used for subarea model development and validation, evaluating traffic diversion, and

# Benning Road and Bridges Transportation Improvements Final Environmental Assessment Appendix E – Transportation Technical Memorandum

forecasting future volumes. The subarea model was used for existing (2019) and future year (2025 and 2045) no-build and build scenarios.

A subarea model from the MWCOG model was extracted that encompasses the major roadway network surrounding the study area (see Figure 1). The subarea model was imported into VISUM, at which point the network, TAZ structure, and peak period trip tables were refined. The TAZs and associated trips were split based on census block groups to provide a greater level of detail and more accurate trip loading and assignment to the network. Data such as population and employment were used as the basis for proportioning trips into subdivided TAZs. A comparison of MWCOG and subarea travel demand model TAZ systems was illustrated in

**Figure** 4. The 11 MWCOG TAZs within the project area were disaggregated into 31 analysis zones taking into consideration the impact of walking distances to or from the proposed streetcar stops on mode choices.

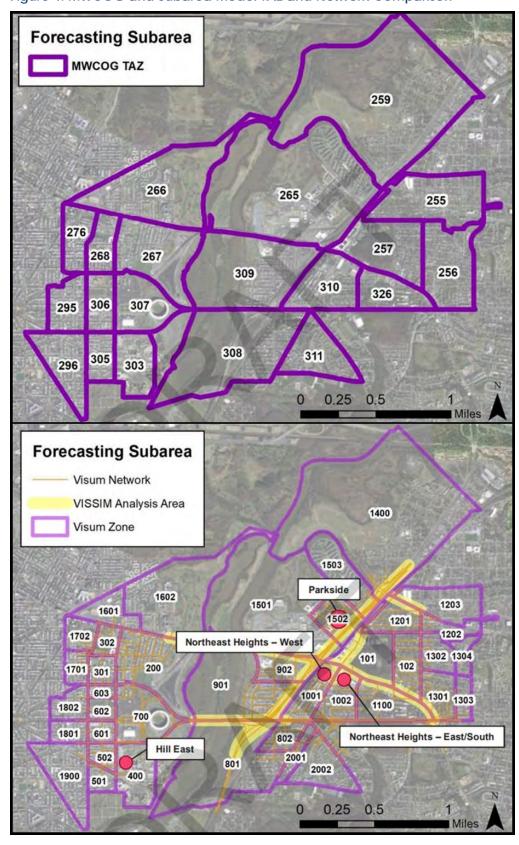


Figure 4: MWCOG and Subarea Model TAZ and Network Comparison

#### 2.3.1.2 Model Validation

The VISUM model was validated against peak period counts for freeways and arterials, specifically 6:00 to 9:00 AM and 3:00 to 7:00 PM, which is consistent with the peak periods of the MWCOG model. Adjustments were made to link free-flow speeds and capacities to calibrate the model to traffic counts. Finally, the built-in VISUM matrix correction procedure, TFlowFuzzy, was used to adjust demand matrix values to link data and turning movement count data in order to obtain the final 2019 O-D matrices. **Table 1** shows the results of validation before and after the matrix correction procedure.

Table 1: VISUM Model Validation Results

Calibration	on Calibration Al				M Peak Period			PM Peak Period		
Measure	Measure Threshold		<b>Before Correction</b>		After Correction		<b>Before Correction</b>		After Correction	
Link Volumes F = Freeway A = Arterial	F	A	F	A	F	A	F	A	F	A
Number of Observations			34	112	34	112	34	114	34	114
Total Volume - Observed			132,676	243,785	132,676	243,785	207,592	326,713	207,592	326,713
Total Volume - Modeled			131,465	251,970	131,075	244,142	213,462	368,870	205,684	327,042
% Difference in Total Volume	7%	15%	-1%	3%	-1%	0%	3%	11%	-1%	0%
R-Squared	0.88	0.88	0.96	0.93	1.00	0.97	0.99	0.82	1.00	0.89
% RMSE	20%	50%	24%	32%	6%	16%	12%	41%	6%	21%
RMSE			915	682	245	350	697	1,173	347	584
Cutli	nes		NB/WB	SB/EB	NB/WB	SB/EB	NB/WB	SB/EB	NB/WB	SB/EB
			We	stbound/l	Eastbound	l across An	acostia Riv	er		
Observed			14,135	4,511	14,135	4,511	7,863	14,969	7,863	14,969
Modeled			16,732	3,260	14,233	4,336	8,872	23,121	7,640	17,180
% Difference	15	5%	18%	-28%	1%	-4%	13%	54%	-3%	15%
		Nortl	hbound/So	uthbound	DC-295 &	k Minneso	ta Ave (Sou	ıth of Benn	ing)	
Observed			10,778	10,958	10,778	10,958	16,396	15,129	16,396	15,129
Modeled			10,227	11,610	10,828	10,783	16,981	14,230	16,427	13,913
% Difference	15	5%	-5%	6%	0%	-2%	4%	-6%	0%	-8%
		Nortl	hbound/So	uthbound	DC-295 &	k Minneso	ta Ave (No:	rth of Benn	ing)	
Observed			9,960	14,345	9,960	14,345	19,742	14,452	19,742	14,452
Modeled			8,859	15,552	10,000	14,056	21,351	15,789	19,976	15,099
% Difference	15	5%	-11%	8%	0%	-2%	8%	9%	1%	4%
		W	estbound/E	astbound	East Capi	tol St/Beni	ning Rd/Bu	rroughs A	ve .	
Observed			10,622	3,605	10,622	3,605	7,624	13,661	7,624	13,661
Modeled			9,757	2,729	10,386	3,063	5,487	13,167	6,721	11,908
% Difference	15	5%	-8%	-24%	-2%	-15%	-28%	-4%	-12%	-13%

The same cut lines used previously to calibrate the MWCOG model were also used during this process. Calibration targets were developed based on guidance from the FHWA Transportation Model Improvement Program (TMIP) Travel Model Validation and Reasonability Checking Manual Second Edition (2010). All calibration thresholds were met for freeway and arterial measures.

#### 2.3.2 Traffic Analysis

VISSIM 11 was used for evaluating multimodal traffic operations, streetcar operations, and queuing impacts at the study intersections and ramps within the VISSIM network limits. VISSIM is a multi-modal microscopic simulation model with the capability of simulating urban traffic and public transit operations, including various transit vehicle types, transit routes, and dwell time of passengers. The simulation was used to evaluate the traffic conditions in the future years as well as the operation of streetcar and its potential impacts on traffic.

#### 2.3.2.1 Simulation Analysis Period Development

A network-wide representative peak hour (herein referred to as the "network peak hour") was determined for the AM peak (7:45 AM to 8:45 AM) and PM peak (5:00 PM to 6:00 PM) using the traffic count data as well as the travel time data for the Benning Road corridor. During these periods, the key corridors of the study network experience the worst traffic operations conditions, characterized by demand greater than capacity, constrained throughput volumes, and plateauing of corridor travel times.

Peak volumes and travel times along Benning Road did not align with those on DC-295. To best serve the purpose and need of the project, the study team placed emphasis on the peak direction of Benning Road when selecting a network peak hour and VISSIM analysis period. Table 2 summarizes the VISSIM seeding, peak, and shoulder periods applied for each peak period of a typical weekday. Given that extensive congestion was observed on DC-295 during the data collection period, the study team utilized one-hour seeding and shoulder periods to allow adequate time for congestion to propagate and dissipate in the model.

**Table 2: VISSIM Simulation Period** 

Time of Day	VISSIM Seeding Period	VISSIM Peak Period	VISSIM Shoulder Period	Network Peak Hour
AM	6:00 AM to 7:00 AM	7:00 AM to 9:00 AM	9:00 AM to 10:00 AM	7:45 AM to 8:45 AM
PM	3:30 PM to 4:30 PM	4:30 PM to 6:30 PM	6:30 PM to 7:30 PM	5:00 PM to 6:00 PM

#### 2.3.2.2 VISSIM Model Development

#### 2.3.2.2.1 Roadway Geometry

The Bing Maps interface within VISSIM 11 was utilized to code network geometry with the following exceptions:

- The number of lanes or length of links at intersections and freeway ramps was modified where justified by field observations (e.g. a de facto right-turn lane was coded on northbound 44<sup>th</sup> Street NE at Nannie Helen Burroughs Avenue NE).
- Construction activity at the intersection of Benning Road NE and Minnesota Avenue NE required that lane assignment be modified from the typical existing geometry.
- Construction activity at the intersection of Benning Road NE and Minnesota Avenue NE required that lane closures be modeled in VISSIM during the PM peak period.

Video footage from April 2, 2019 confirmed that the right lane was closed on eastbound Benning Road NE just downstream of the Minnesota Avenue intersection throughout the entire peak period, while the left-turn lane on northbound Minnesota Avenue was blocked by a construction vehicle until 5:00 PM. These conditions substantially constrained throughput and caused queueing on both approaches to the intersection. Though included as part of the calibration process, these lane closures will not be modeled during alternatives analysis.

#### 2.3.2.2.2 Traffic Control

Key traffic control devices, such as traffic signals, stop signs, lane configurations, and speed limits were coded based on Synchro files provided by DDOT, Google Street View, and field observations. Traffic signal timing and phasing was verified through video footage and required modification in one location:

 The cycle length at the intersection of Benning Road NE and Minnesota Avenue NE during the AM and PM peak period was coded as 120 seconds in the provided Synchro files but observed as 150 seconds in the field.

Desired speed distributions in VISSIM were initially coded based on roadway speed limits but modified as necessary based on supplemental probe data analysis.

#### 2.3.2.2.3 Vehicle Routing

End-to-end vehicular routing was coded in VISSIM and existing balanced volumes combined with Location Services-based origin-destination (O-D) data from StreetLight were used to develop a seed O-D matrix. Existing balanced volumes were converted to target link and turning movement volumes to iteratively adjust this seed matrix in VISUM 18 using the TFlowFuzzy origin-destination matrix estimation procedure. The resultant final O-D matrices were used to load one set of vehicle routes per peak period in VISSIM, as travel patterns were found to not differ substantially between the start and finish of the AM and PM analysis periods. StreetLight data

suggested minimal difference in vehicular trips for major O-D pairs within the network, which supports the use of a single O-D matrix for the entire simulated peak period.

#### 2.3.2.2.4 Vehicle Compositions

Three vehicle classes were assumed to sufficiently define the primary traffic stream and coded in VISSIM: passenger cars, single unit trucks, and tractor-trailers. Separate vehicle compositions were coded for the freeway and arterials during each peak period based on vehicle classification data collected on mainline DC-295, interchange ramps, and arterial segments in the study area.

**Table** 3 summarizes the vehicle compositions used in the AM and PM models. These vehicle compositions were developed from the 72-hour freeway, ramp, and arterial counts, as well as the intersection turning movement counts. In VISSIM, Vehicle Compositions defined the percentage of passenger cars and trucks in the model, whereas the distribution of single unit trucks and tractor-trailers were accounted for in the 2D/3D Model Distributions.

Table 3: Vehicle	Composition	Summary
------------------	-------------	---------

Vehicle Class	Facility Type	Composition
	Emacricary	95.0% (AM)
Descenden Cana	Freeway	98.3% (PM)
Passenger Cars	Arterial	95.5% (AM)
	Artenai	97.3% (PM)
	Ежастиот	4.7% (AM)
Cinala Unit Tructe	Freeway	1.6% (PM)
Single Unit Trucks	Arterial	4.2% (AM)
	Artenai	2.5% (PM)
	Freeway	0.3% (AM)
Tractor-Trailers	Freeway	0.1% (PM)
	Arterial	0.8% (AM)
	Arterial	0.2% (PM)

#### 2.3.2.2.5 Transit Information

Transit vehicles (bus and streetcar) were coded separately using the Public Transport Lines tool in VISSIM. Bus routes and stops in the VISSIM network area were coded using posted maps and schedules. Routes included in the models are shown in Table 4. Bus stop dwell times were set using a normal distribution with a mean of 20 seconds and standard deviation of ten seconds.

**Table 4: Public Transit Route Summary** 

Operator	Bus Route
DDOT	DC Streetcar
Metrobus	X1, X2, X3, X9, V2, V4, V7, V8, U4, U5, U6, U7, 96

#### 2.3.2.2.6 Pedestrian Mode

The magnitude of pedestrian and bicyclist activities was assessed at the intersections adjacent to Metrorail stations or existing streetcar stops and coded in VISSIM accordingly. No dedicated bicycle facilities were included in the models.

#### 2.3.2.3 VISSIM Model Calibration

#### 2.3.2.3.1 Purpose of Calibration

Microsimulation models are developed to assess the impacts of proposed improvement alternatives. However, to establish confidence in model outputs under future build and no-build scenarios, it is critical that a model is capable of accurately replicating existing conditions. Calibration entails the adjustment of various input parameters to improve a model's ability to reproduce such conditions and associated measures of effectiveness (MOEs). VISSIM contains a variety of adjustable parameters that support the calibration process, most notably those related to car-following and lane-changing.

#### 2.3.2.3.2 Calibration Methodology and Targets

The Existing Conditions (2019) VISSIM models were calibrated according to the guidelines presented in FHWA's Traffic Analysis Toolbox (TAT) Volume III (2004). Vehicle throughput, travel time, speed, and queue length were used as calibration MOEs. Since queue length calibration is not defined in FHWA's TAT, the Virginia Department of Transportation's (VDOT) Traffic Operations and Safety Analysis Manual (TOSAM) was referenced for queue length calibration guidance. **Table 5**: provides the calibration measures and targets. The critical locations for queue calibration are identified in **Table 6**.

**Table 5: VISSIM Calibration Criteria and Acceptance Targets** 

Simulated Measure	Calibration Threshold	Calibration Period	
Simulated Traffic Volume – Individual Links (vehicles per hour)  For mainline and interchange ramps, difference targets must be met for a minimum of 85% of mainline segments and ramps  At intersections, difference targets must be met for a minimum of 85% of approaches for the study intersections	Within ± 100 vph for <700 vph Within ± 15% for ≥700 vph to <2,700 vph Within ± 400 for ≥2,700 vph	Peak Hour	
Simulated Traffic Volume – Sum of All Link Flows	Within 5% of sum of all link counts	Peak Hour	
Simulated Traffic Volume – GEH Statistic For mainline and interchange ramps, GEH statistic target must be met for a minimum of 85% of segments	< 5 for individual link flows < 4 for sum of all link flows	Peak Hour	
Simulated Average Link Speed (miles per hour)	Visually inspecting model speed heat map against INRIX or Google 15-minute average speed data for study corridor segments during the entire peak period and shoulder period	Peak Period + Shoulder Period	

Simulated Measure	Calibration Threshold	Calibration Period
Simulated Travel Time (minutes)	Within ± 15% for average observed travel times of Benning Road and DC-295 study	Peak Period
	corridors	T GUILT GITGU
	Visually inspecting bottleneck locations	
	using model speed heat map against	
	INRIX or Google speed data as well as	
Visual Audits		
Bottleneck locations		
Queuing impact at the identified critical	against observed data to verify queues	Peak Period + Shoulder Period
locations that consist of ramps and intersection	that have the potential of impacting:	Silouidel Tellod
approaches	Spillover to an adjacent intersection	
	Spillover from a turn lane	
	The mainline in the case of a signal or	
	junction at the end of a ramp terminal	

Table 6: Critical Locations for Bottleneck and Queue Verification

Location Type	Location Description	
Intersection	All approaches at Benning Road NE/Minnesota Avenue NE	
Intersection	All approaches at Benning Road NE/East Capitol Street	
Intersection	Intersection Westbound left turn at Benning Road NE/Oklahoma Avenue NE	
Mainline	All merge and weave areas on DC-295 at the study area interchanges	
Mainline  AM/PM: Southbound DC-295 freeway basic segments throughout study area PM: Northbound DC-295 freeway basic segments throughout study area		
Ramp	Ramp All ramps associated with the DC-295/Benning Road NE interchange	

As mentioned previously, the intersection of Benning Road NE and Minnesota Avenue NE was under construction throughout the data collection period. In order to calibrate to observed data, network geometry and traffic conditions were modeled to reflect the current phase of construction, which included multiple lane closures. While it was necessary to use construction-constrained traffic volume data to reproduce the queue length, travel time, and throughput data collected in the field, the study team was informed of DDOT's concerns that calibrated behavior at this intersection may not be appropriate for application to subsequent model scenarios. As such, the following were considered during calibration:

- Calibration efforts were focused on system-wide measures, such as Benning Road NE end-to-end corridor travel times.
- The study team attempted to calibrate the model without adjusting arterial driving behavior or by applying the same changes to driving behavior throughout the entire network.

Any construction-specific geometry and driving behavior changes at the intersection of Benning Road NE and Minnesota Avenue NE were coded with the intent to remove for subsequent model scenarios.

#### 2.3.2.3.3 Number of Model Runs

Since microsimulation models are stochastic and will produce unique results from run-to-run, multiple iterations must be conducted with different random seeds and post-processed to determine an "average" state of traffic operations in the study network. Based on FHWA TAT (2004) and VDOT Sample Size Determination Tool methodologies ten runs is an adequate sample size for reporting statistically valid results.

#### 2.3.2.3.4 Calibration Data

Balanced volumes developed from traffic counts as described in previous section as well as field travel time runs conducted on April 2<sup>nd</sup> (arterials) and April 9<sup>th</sup> (freeway) supplemented with crowdsourced probe data from Google were used as targets during the calibration process. However, the following data challenge was encountered:

An incident occurred on northbound DC-295 off-ramp to Benning Road during the AM peak
period on April 9th, so all applicable northbound field travel time runs were deemed insufficient
for model calibration (i.e. the travel time runs were not representative of typical traffic conditions).

Since the crowdsourced probe data provided a substantially larger sample size, the study team decided to use these data for freeway calibration targets based on crowdsourced probe data from April  $9^{th}$  –  $11^{th}$ . Arterial travel time targets were set based on field travel time data runs because field run data closely represents the impact from the specific construction activities on April 2nd.

Given the high variability observed in both sources of travel time data, the probe data was validated by ensuring that the average field travel time fell within the range of travel times observed in the probe data.

#### 2.3.2.3.5 Calibration Parameters and Adjustments

For modification purposes, specific parameters were adjusted to achieve target traffic volumes, speed, travel time, and queue lengths. The primary adjusted parameters that included:

- Driver behavior
- Lane-change distance
- Free-flow speeds
- Traffic demand volume distribution (15-minute interval)
- Replicating external congestions at network termini using INRIX speed data

External congestion that impacts operations within the VISSIM study area exists on DC-295 external to the study area. Time-dependent speed reductions based on probe data from INRIX at the edge of the VISSIM network was used to replicate the extent and duration of reduced speeds.

#### 2.3.2.3.6 Calibration Results Summary

**Table 7** and **Table 8** summarized the AM and PM existing VISSIM results compared with field counts, travel times, speeds, and queue lengths, suggesting that the models were reasonably

calibrated to the standards and guidelines established by FHWA's Traffic Analysis Toolbox Volume III (2004).

The impacts of construction at the intersection of Benning Road NE and Minnesota Avenue NE that were present during data collection raise challenges for direct application of these Existing Conditions models to the development and analysis of future no build and build alternatives. While the Existing Conditions calibration required that prevailing conditions during the days of data collection be reasonably replicated, the study team avoided over-calibrating the model to these temporary conditions by making changes to network geometry and driving behavior sparingly. Calibration to these conditions were required to match calibration targets, but engineering judgement will be used to update the presented Existing Conditions models to become a typical "baseline" model for assessing future conditions.

**Table 7: AM Model Calibration Summary** 

Calibration Item	Basis	Criteria	Total	Percent	Target	Criteria Met
Simulated Traffic Volume (Intersections)	Approaches (n = 87)	Within $\pm$ 100 vph for < 700 vph Within $\pm$ 15% for $\geq$ 700 vph to < 2,700 vph Within $\pm$ 400 vph for $\geq$ 2,700 vph	86	99%	85%	Yes
Simulated Traffic Volume (Freeways)	Segments (n = 29)	Within $\pm$ 100 vph for < 700 vph Within $\pm$ 15% for $\geq$ 700 vph to < 2,700 vph Within $\pm$ 400 vph for $\geq$ 2,700 vph	29	100%	85%	Yes
Simulated Traffic Volume (Sum of All Link Flows)	All Segments/Approaches	Within 5% of sum of all link counts		0%	5%	Yes
Simulated Traffic	Segments/Approaches (n = 116)	< 5 for individual link flows	115	99%	85%	Yes
Volume (GEH Statistic)	All Segments/Approaches	< 4 for the sum of all link flows	0.1		4.0	Yes
Simulated Travel Time	Segments (n=4)	Within ± 15% for average travel time	4	100%	85%	Yes
Maximum Simulated Queue Length	Approaches (n = 14)	Modeled queues qualitatively reflect the impacts of observed queues				Yes
Visual Review of Bottleneck Locations	Targeted Critical Locations	Speed heat maps qualitatively reflect the pattern and duration of congestion				Yes

**Table 8: PM Model Calibration Summary** 

Calibration Item	Basis	Criteria	Total	Percent	Target	Criteria Met
Simulated Traffic Volume (Intersections)	Approaches (n = 87)	Within $\pm$ 100 vph for < 700 vph Within $\pm$ 15% for $\geq$ 700 vph to < 2,700 vph Within $\pm$ 400 vph for $\geq$ 2,700 vph	85	98%	85%	Yes
Simulated Traffic Volume (Freeways)	Segments (n = 29)	Within $\pm$ 100 vph for < 700 vph Within $\pm$ 15% for $\geq$ 700 vph to < 2,700 vph Within $\pm$ 400 vph for $\geq$ 2,700 vph	29	100%	85%	Yes
Simulated Traffic Volume (Sum of All Link Flows)	All Segments/Approaches	Within 5% of sum of all link counts		0%	5%	Yes
Simulated Traffic	Segments/Approaches (n = 116)	< 5 for individual link flows	115	99%	85%	Yes
Volume (GEH Statistic)	All Segments/Approaches	< 4 for the sum of all link flows	0.7		4.0	Yes
Simulated Travel Time	Routes (n=4)	Within ± 15% for average travel time	4	100%	85%	Yes
Maximum Simulated Queue Length	Approaches (n = 14)	Modeled queues qualitatively reflect the impacts of observed queues				Yes
Visual Review of Bottleneck Locations	Targeted Critical Locations	Speed heat maps qualitatively reflect the pattern and duration of congestion				Yes

# 3.0 Existing (2019) Conditions

This section describes the existing transportation conditions in the project area with the first subsection focusing on the travel demand model and the second sub-section focusing on the results of the existing traffic analysis.

#### 3.1 Existing Travel Demand Model

As discussed in previous section, existing conditions analysis is based on the traffic data and observations from site visits and results of the validated travel demand model. 2019 MWCOG Regional Travel Demand model ("Base Model") was used to reflect the existing conditions.

#### 3.1.1 Model Inputs

The two major inputs in the modeling process used in this analysis are land use data and transportation networks. To develop the existing conditions scenario, models were calibrated to match existing traffic counts using the inputs outlined below.

#### 3.1.2 Existing Land Use

For the existing conditions, the analysis used the MWCOG Round 9.1 Cooperative Land Use Forecasts for 2019. Table 9 shows the total population and employment by sector in the project area in 2019.

**Table 9: Existing Population and Employment Condition** 

	2019 Total
Households	18,418
Population	46,878
Employment	6,786

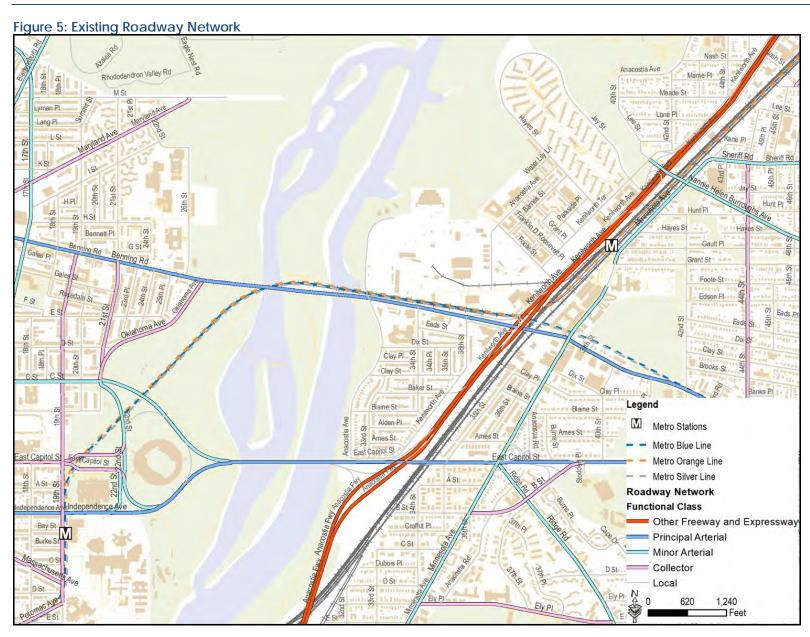
#### 3.1.3 Existing Transportation Network

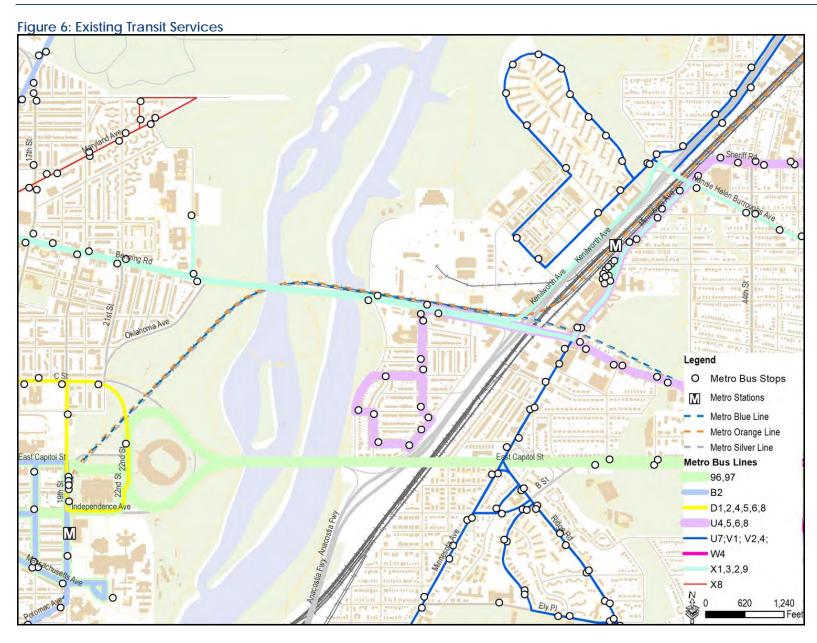
The transportation networks used in the model represent the current roadway and transit systems in the region. **Figure 5** and

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**Figure** 6 show the roadway and transit network in the project area, respectively. The network has a grid layout with additional diagonals along portions of Benning Road and East Capitol Street. The grid is organized into a hierarchy of expressway, arterial, collectors and local streets.

The travel demand model used the MWCOG 2014 roadway network for the region outside of the travel demand analysis area. The MWCOG 2014 transit network was also used to represent the existing transit service in the region.





# 3.2 Existing VISSIM Simulation Modeling

This section provides an assessment of existing traffic conditions in the project area. The traffic data collected within the project area for the analysis of existing conditions included traffic volume counts, pedestrian and bicycle counts, and historic crash data.

# 3.2.1 Existing Volumes

Figure 7,

Figure 8, and

**Figure** 9 show the existing (2019) morning and evening peak hour traffic volumes at the project area freeways and intersections. During the morning peak hour, the traffic is heaviest in the "inbound" or the westbound direction along Benning Road. In the evening peak hour, the travel pattern reverses in which people travel in the "outbound" direction.

# 3.2.2 Existing Conditions

In the morning peak, southbound DC-295 congestion persisted throughout the simulation period and extends the full length of the study area from East Capitol Street to Nannie Helen Burroughs Avenue NE. The primary bottleneck on southbound DC-295 occurred at the East Capitol Street off-ramp, which included a lane drop. Merging, diverging, and weaving behavior at other ramp junctions along southbound DC-295 (e.g. Benning Road NE and Kenilworth Avenue NE) also resulted in reduced speeds and additional congestion. While external to the direct study area, the impact of congestion at the ramp junction with Pennsylvania Avenue SE extended to the southern boundary of the study area. Northbound DC-295 operated under free flow conditions throughout the AM peak period.

In the evening peak, northbound DC-295 congestion persisted throughout the simulation period and extended the full length of the study area from East Capitol Street to Nannie Helen Burroughs Avenue NE. The primary bottlenecks responsible for this congestion included the US 50 interchange (external to the network), the Benning Road NE onramp, and the Kenilworth Avenue NE on-ramp. Likewise, volumes on southbound DC-295 plateaued before PM simulation period and do not substantially rise or fall until after 6:00 PM. Congestion extended from the southern terminus of the network to near the interchange at Nannie Helen Burroughs Avenue NE well before the start of the VISSIM model simulation period due to two primary bottlenecks: the Pennsylvania Avenue SE interchange (external to the network) and the East Capitol Street lane drop.

Arterial network also experienced significant demand in both AM and PM peak.

**Figure** 10 and

**Figure** 11 show the intersection LOS at the project area intersections under existing conditions during the morning and evening peak hours. **Table 10** provides delay and LOS by movement for the critical intersections for the morning and evening peak hours. Critical intersections are defined as those with intersection LOS E or worse. **Table 11** shows the associated maximum queue lengths by approach.

Results show that none of the intersections along Benning Road operated under LOS F in existing morning peak; the only two intersections that are under LOS F in Existing AM peak were Deane Avenue NE at Kenilworth Terrace NE and Kenilworth Avenue NE at Foote Street NE. There were, however, three intersections along Benning Road that operated under LOS F in the evening peak hour. These intersections are:

- Benning Road NE at Anacostia Avenue NE;
- Benning Road NE at 34th Street NE; and
- Benning Road NE at Minnesota Avenue NE

In the morning peak, east on Benning Road NE, congestion was observed in the eastbound direction between 44th Street NE and East Capitol Street. This congestion was primarily caused by queueing from the eastbound approach to East Capitol Street as well as turning volumes from crossing streets (e.g., 44th and 45th Street NE) that are closely spaced to East Capitol Street. Traveling westbound on Benning Road NE, west of the DC-295/Benning Road NE interchange, congestion was observed at Benning Road NE and Oklahoma Avenue NE, primarily caused by high left-turn volumes at this intersection. Significant queueing was observed on Nannie Helen Burroughs Avenue NE and the surrounding local streets (e.g. southbound Minnesota Avenue NE, southbound Kenilworth Avenue NE, and Deane Avenue NE).

Table 10: Existing (2019) Peak Hour (AM and PM) Delay and Levels of Service (LOS) at the Critical Intersections

Intersection	Traffic	Peak			North- bound		Sou bou		West- bound		East- bound	
	Control	Hour	Delay	LOS	Delay	LOS	Delay	LOS	Delay	LOS	Delay	LOS
Benning Road NE at Anacostia Avenue NE	Signalized	PM	127	F	44	D	59	Е	3	A	191	F
Benning Road NE at 34 <sup>th</sup> Street NE	Signalized	PM	107	F	19	В	45	D	26	С	166	F
Benning Road NE Ramp to DC-295 at 36 <sup>th</sup> Street NE	Unsignalized	PM	46	Е	7	A					59	F
Benning Road NE at Minnesota Avenue NE	Signalized	PM	213	F	139	F	39	D	82	E	387	F
Minnesota Avenue NE at Dix Street NE	Signalized	PM	58	E	87	F	12	В	98	F	38	D
Deane Avenue NE at Lee St NE	Unsignalized	AM	177	F			227	F	1	A	271	F
Deane Avenue NE at Kenilworth Terrace NE	Signalized	AM	113	F	34	С	228	F	3	A	59	Е

Nannie Helen Burroughs	Signalized	PM	63	Е	161	F			30	С	17	В
Avenue NE at												
Kenilworth Avenue NE												
and DC- 295 U-turns												
Nannie Helen Burroughs	Signalized	AM	75	E	44	D	212	F	62	E	22	C
Avenue NE at Minnesota												
Avenue NE												
Kenilworth Avenue NE	Unsignalized	AM	158	F			183	F			17	В
at Foote Street NE												

Table 11: Existing (2019) Peak Hour (AM and PM) Maximum Queue Length (feet) at the Critical Intersections

Intersection	Peak Hour	Northbound	Southbound	Westbound	Eastbound
Benning Road NE at East Capitol Street SE	AM	726	1059	776	279
Benning Road NE at East Capitol Street SE	PM	278	337	276	708
East Capitol Street SE at Texas Avenue SE	AM	699	516	-	-
East Capitol Street SE at Texas Avenue SE	PM	277	191	-	-
Benning Road NE and Minnesota Avenue NE	AM	454	537	411	554
Benning Road NE and Minnesota Avenue NE	PM	581	424	297	5057

Two unexpected lane closures were in place during the PM peak period: eastbound Benning Road NE at Minnesota Avenue NE and northbound Minnesota Avenue NE at Benning Road NE. As mentioned previously, one lane was closed on eastbound Benning Road NE just downstream of the Minnesota Avenue NE intersection, and a construction vehicle blocked the left-turn lane on northbound Minnesota Avenue NE until 5:00PM. These lane closures led to significant queueing west (through the Anacostia Avenue NE intersection) on Benning Road NE and south (through the Dix Street NE intersection) of Minnesota Avenue NE. Bottleneck conditions were also observed on Nannie Helen Burroughs Avenue NE and the surrounding local streets (e.g. southbound Minnesota Avenue NE, southbound Kenilworth Avenue NE, and Deane Avenue NE).

Detailed modeling results for the existing conditions for all project area intersections are provided in **Attachment A**.

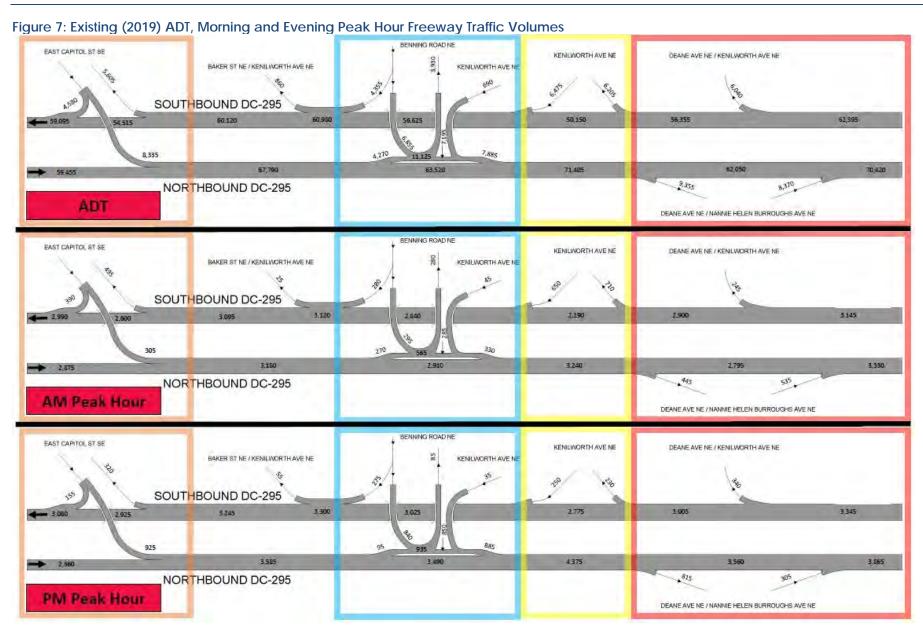


Figure 8: Existing (2019) Morning and Evening Peak Hour Intersection Traffic Volumes (Sheet 1 of 2)

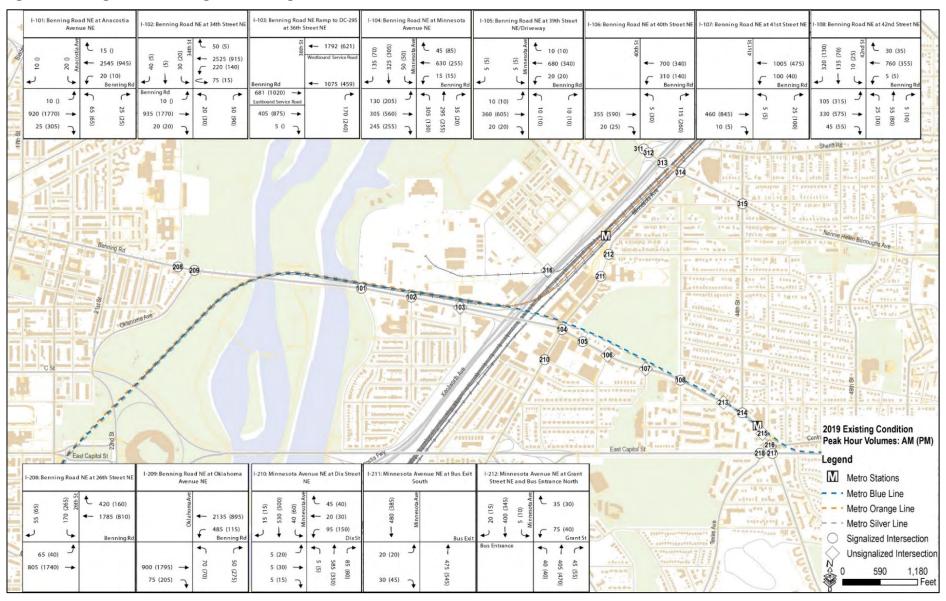
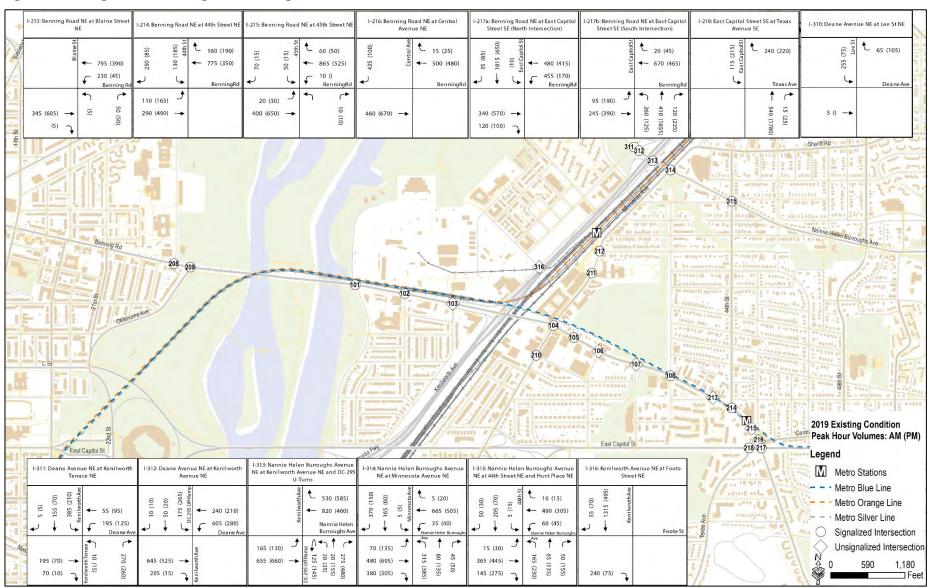


Figure 9: Existing (2019) Morning and Evening Peak Hour Intersection Traffic Volumes (Sheet 2 of 2)



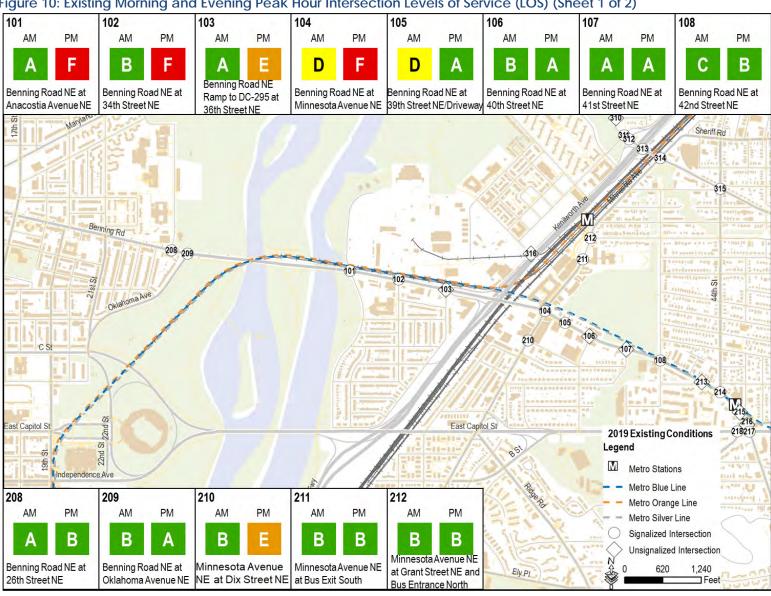


Figure 10: Existing Morning and Evening Peak Hour Intersection Levels of Service (LOS) (Sheet 1 of 2)

213 216 217 218 215 214 310 311 AM PM AM PM AM PM AM PM AM PM AM PM PM AM PM В В Benning Road NE at Benning Road NE Benning Road NE at Benning Road NE at Benning Road NE at East Capitol Street SE Deane Avenue NE at Deane Avenue NE at Blaine Street NE at 44th Street NE 45th Street NE East Capitol Street SE at Texas Avenue Se Central Avenue NE Lee St NE Kenilworth Terrace NE 2019 Existing Conditions Legend Metro Stations Independence Ave Metro Blue Line 312 314 315 313 316 Metro Orange Line PM AM PM AM PM AM PM AM AM PM Metro Silver Line Signalized Intersection Unsignalized Intersection Nannie Helen BurroughsNannie Helen BurroughsNannie Helen Burroughs Deane Avenue NE at Kenilworth Avenue NE Avenue NE at Kenilworth Avenue NE at 1,240 Avenue NE at 44th Ely PI Kenilworth Avenue NE at Foote Street NE Minnesota Avenue NE

Figure 11: Existing Morning and Evening Peak Hour Intersection Levels of Service (LOS) (Sheet 2 of 2)

# 3.2.3 Existing Traffic Safety Conditions

Detailed crashed data analysis was performed based on a three-year crash data (2016 – 2018) provided by DDOT. The data includes type of crashes (e.g., rear end, side swiped, etc.), crash severity (e.g., injuries involved), the crash location, and indicates whether any pedestrians were involved in the accident. The analysis indicated that Benning Road and Minnesota Avenue, Benning Road and East Capitol Street, and Minnesota Avenue and Nannie Helen Burroughs intersections are the three intersections with the highest crash rates. Detailed information regarding these calculations and analysis can be found in **Appendix A**, *Crash Data and Safety Analysis Technical Memorandum*.

# 4.0 Opening Year 2025 Conditions

It was assumed that the proposed action would be fully operational in 2025. Traffic and transit operations during the morning and evening peak hours were evaluated for the opening year 2025. A future No Build scenario was analyzed for the year 2025 to serve as a baseline comparison for the Build Alternatives. No Build refers to planned and/or programmed highway, transit, High-Occupancy Vehicle (HOV), and bicycle and pedestrian projects defined in the Financially Constrained Long-Range Plan (CLRP).

For the build scenarios, two streetcar alternatives were developed and tested. The first alternative assumed curbside operation, while the second alternative assumed median operation, with both alternatives terminating at the Benning Road Metrorail Station.

# 4.1 Opening Year 2025 No Build Travel Demand Forecasting

The two major inputs into the demand modelling process used in this analysis were land use data and transportation networks. For the No-Build scenario, both inputs were based on regionally accepted baseline conditions including the MWCOG Round 9.1 Cooperative Land Use Forecasts and the regional CLRP.

#### 4.1.1 2025 No-Build Land Use

For the opening year 2025 No-Build scenario, the regional land use was determined based on the MWCOG Round 9.1 Cooperative Land Use Forecasts for 2025. Regionally, the model shows a growth of 3.4 percent in number of households, 2.9 percent in population, and 11 percent in employment over the existing conditions (2019). **Table 12** highlights the total population and employment by sector within a half-mile of the Benning Road corridor for 2025.

Table 12: No Build Population and Employment Growth

	2019 Total	<b>Growth Factor</b>	Total Growth	2025 Total
Households	18,418	2.26%	13.54%	20,911
Population	46,878	2.02%	12.11%	52,554
Total Employment	6,786	1.90%	11.42%	7,561

## 4.1.2 2025 No Build Transportation Network

The transportation network for the 2025 No-Build scenario includes the changes proposed in the 2020 CLRP. The CLRP includes a range of planned improvements to the roadway and transit networks throughout the metropolitan region. Within the project area, the only one change to the roadway network identified in the CLRP is the removal of one of the three lanes in each direction along East Capitol Street between 40<sup>th</sup> Street and Southern Avenue to improve pedestrian safety<sup>1</sup>. As a result, East Capitol Street will operate with two through lanes in both directions between 40<sup>th</sup> Street and Southern Avenue.

#### 4.1.2.1 Metrorail

Regional Metrorail service improvements provide some changes to the services provided at the two stations within the project area. **Table 13** summarizes the peak and off-peak headways of the assumed Metrorail system in 2020.

Table 13: Peak and Off-Peak Headways of Metrorail Lines (2025)

Line	Terminus A	Terminus B	Peak Headway (minutes)	Off-Peak Headway (minutes)
Blue	Franconia/Springfield	Largo	7	12
Orange	Vienna	New Carrollton	6	12
Silver	Wiehle-Reston East	Largo Town Center	6	12

#### 4.1.2.2 Local Bus

Local bus service in the project area is provided primarily by Metrobus. **Table 14** summarizes the peak and off-peak headways for the local routes serving the project area. The table shows morning peak frequencies; evening peak frequencies are assumed to be directionally reversed.

# 4.2 Opening Year 2025 No-Build VISSIM Simulation Modeling

This section describes the development of the 2025 No-Build VISSIM model and provides an assessment of No-Build traffic conditions in the project area. To reflect the change proposed in the CLRP, the VISSIM model was updated at East Capitol Street and Benning Road intersection to accommodate the through-lane reduction on East Capitol Street.

<sup>&</sup>lt;sup>1</sup> <u>http://www.mwcog.org/clrp/projects/new/proposed\_2013.asp</u>

## 4.2.1 2025 No-Build Traffic Volume Development

The 2025 No-Build future year intersection traffic volumes were developed using a growth rate obtained from MWCOG Version 2.3.75 regional travel demand model. As mentioned previously, traffic volume growth rates for the links in the subarea were estimated by developing future year O-D matrices for AM and PM peak periods, assigning this demand to the Visum network, and comparing against the 2019 traffic assignment. **Figure 12**,

Figure 13, and Figure 14 show the projected peak hour traffic volumes for the opening year 2025.

Table 14: Local Bus Frequency (2025)

Route	Direction	Morning Peak Headway (minutes)	Off-Peak Headway (minutes)
96,97	Eastbound	20	60
90,97	Westbound	20	60
IIO	Westbound	30	30
U2	Southbound	30	30
U4	Clockwise	12	30
III II	Northbound	30	38
U5,U6	Southbound	14	14
110	Northbound	12	12
U8	Southbound	60	60
V/7 0 0	Eastbound	20	30
V7,8,9	Westbound	20	27
TA7.4	Southbound	30	60
W4	Northbound	12	60
V1 V2	Eastbound	30	-
X1,X3	Westbound	15	-
V2	Eastbound	6	8
X2	Westbound	7	9
V0	Eastbound	15	15
X9	Westbound	15	15

#### 4.2.2 2025 No-Build Conditions

Like the existing condition, extensive congestion persisted on southbound DC-295 over the entire simulation period in the morning peak due to multiple capacity constraints, including the downstream bottleneck at the Pennsylvania Boulevard SE interchange, lane drop at the East Capitol Street interchange, and merges from Benning Road NE and Kenilworth Avenue NE.

In the evening peak, extensive congestion was observed in northbound DC-295 over most of the simulation period like existing condition due to multiple capacity constraints, including the downstream bottleneck at the US 50 interchange, diverge to Nannie Helen Burroughs Avenue NE, and merges from Benning Road NE and Kenilworth Avenue NE. Southbound DC-295 travel time improved by nearly 1.5 minutes, likely due to increased metering of demand from eastbound Benning Road NE by the eastbound-to-northbound ramp, which suffers increased delay and queueing under 2025 volume.

## Figure 15 and

Figure 16 display No Build intersection LOS at the project area intersections during the morning and evening peak hours for opening year 2025. Table 15 provides delay and LOS by movement for the critical intersections for the morning and evening peak hours. Critical intersections are defined as those with intersection LOS E or worse.

Table 16 shows the associated maximum queue lengths by approach.

Substantial delays were observed at East Capitol Street, especially on the westbound Benning Road NE and southbound East Capitol Street approaches in the morning peak. In the meantime, Northeast Heights Development access points, such as westbound and westbound Dix Street NE were congested too.

As for the evening peak, operations were dramatically improved on eastbound Benning Road NE under 2025 No-Build scenario relative to existing conditions, since the latter was calibrated with a lane closure in place. Further improvements to signal timing and the realignment of 39th Street (which eliminates split phasing at this intersection) also contributed to smoother traffic flow. The addition of a northbound left turn at Texas Avenue SE contributed to a doubling in delay at East Capitol Street SE at Texas Avenue SE intersection.

Detailed results for the study area in the Opening Year 2025 No-Build conditions are provided in **Attachment B.** 

Table 15: 2025 No-Build Peak Hour (AM and PM) Delay and Levels of Service (LOS) at the Critical Intersections

Intersection	Traffic	Peak	Interse	ection	Northb	ound	Southb	ound	Westbe	ound	Eastl	oound
Intersection	Control	Hour	Delay	LOS	Delay	LOS	Delay	LOS	Delay	LOS	Delay	LOS
Benning Road NE at East Capitol Street SE	Signalized	AM	105	F	12	В	214	F	48	D	19	В
East Capitol Street SE at Texas Avenue SE	Signalized	PM	74	Е	82	F			8	A	114	F
Deane Avenue NE at Lee Street NE	Unsignalized	AM	287	F			398	F	1	A	321	F
Deane Avenue NE at Kenilworth Terrace NE	Signalized	AM	62	E	91	F	71	Е	4	A	61	Е
Deane Avenue NE at Kenilworth Avenue NE	Signalized	AM	55	Е			291	F	59	Е	8	A
Nannie Helen Burroughs Avenue NE at Kenilworth Avenue NE and DC-295 U- turns	Signalized	PM	68	E	152	F	175	F	24	С	30	С
Nannie Helen Burroughs Avenue NE at Minnesota Avenue NE	Signalized	AM	78	Е	249	F	80	Е	43	D	24	С
Kenilworth Avenue NE at Foote Street NE	Unsignalized	AM	163	F			189	D			17	В

<sup>\*</sup> Stop-controlled intersection, in which intersection LOS is expressed in terms of the average vehicle delay of the worst movement

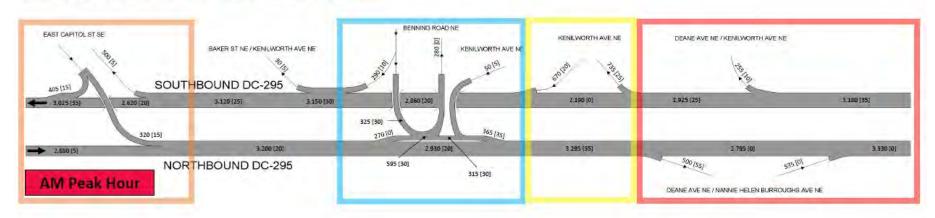
Table 16: 2025 No-Build Peak Hour (AM and PM) Maximum Queue Length (feet)

Intersection	Peak Hour	Northbound	Southbound	Westbound	Eastbound
Benning Road NE at East Capitol Street SE	AM	417	533	1775	220
Benning Road NE at East Capitol Street SE	PM	244	492	229	1332
East Capitol Street SE at Texas Avenue SE	AM	432	74	-	-
East Capitol Street SE at Texas Avenue SE	PM	207	253	-	-
Benning Road NE and Minnesota Avenue NE	AM	460	530	378	372
Benning Road NE and Minnesota Avenue NE	PM	285	447	284	560

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Figure 12: 2025 No-Build Morning and Evening Peak Hour Freeway Traffic Volumes

\* Numbers on Figures denote: Forecasted Volumes [Vehicle difference from Existing Conditions]



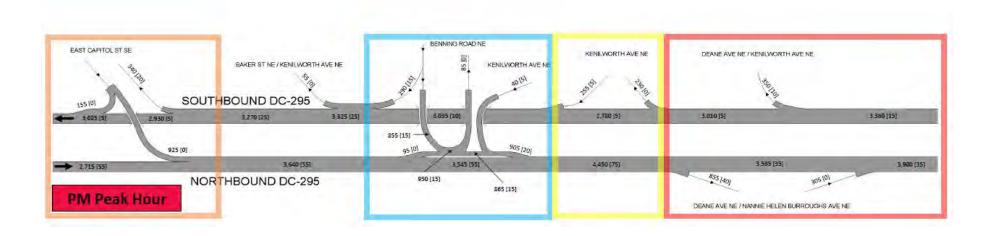


Figure 13: 2025 No-Build Morning and Evening Peak Hour Intersection Traffic Volumes (Sheet 1 of 2)

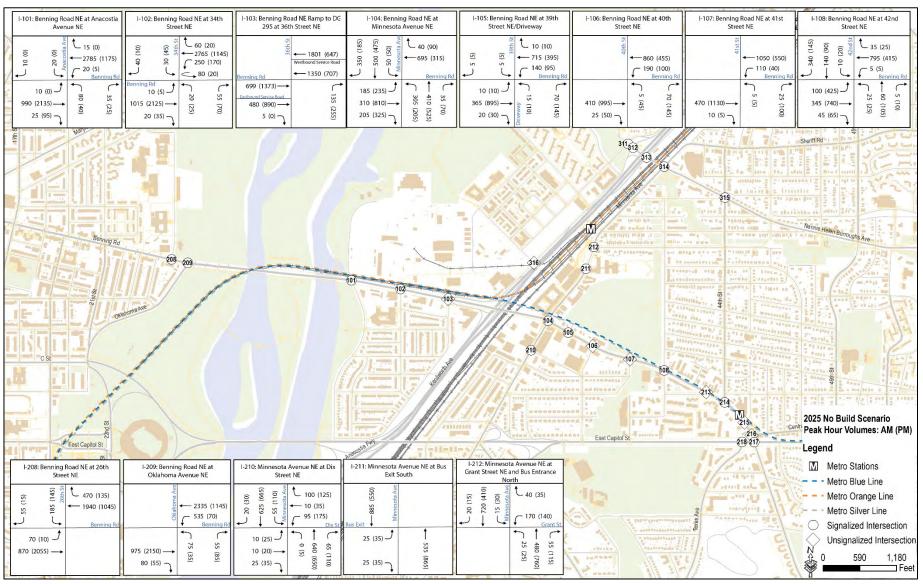
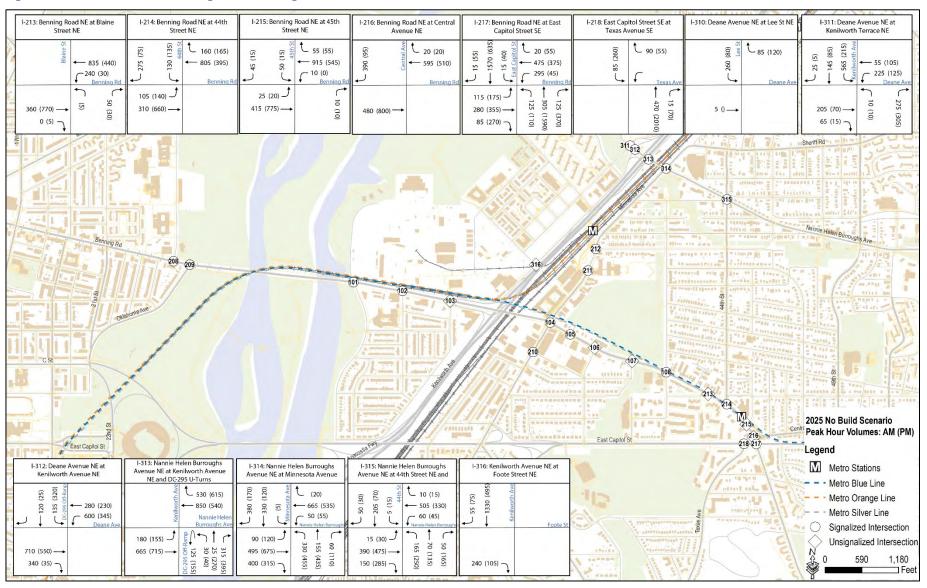
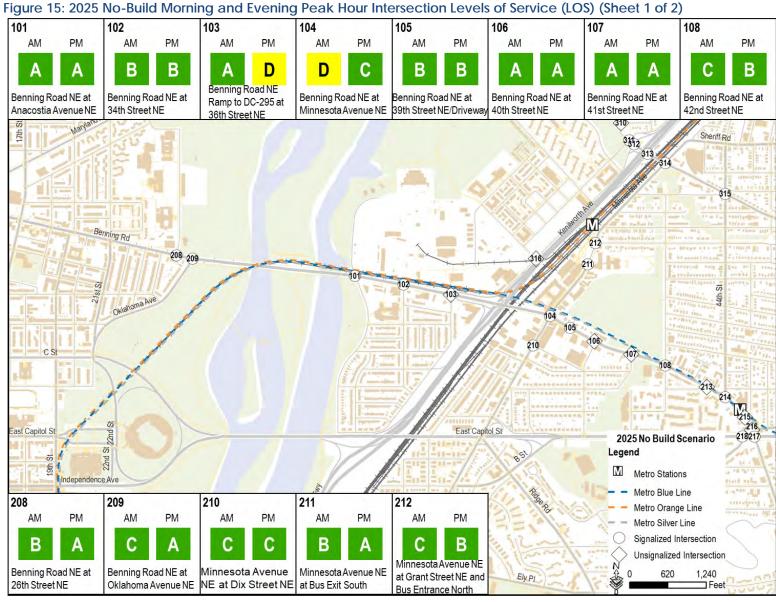
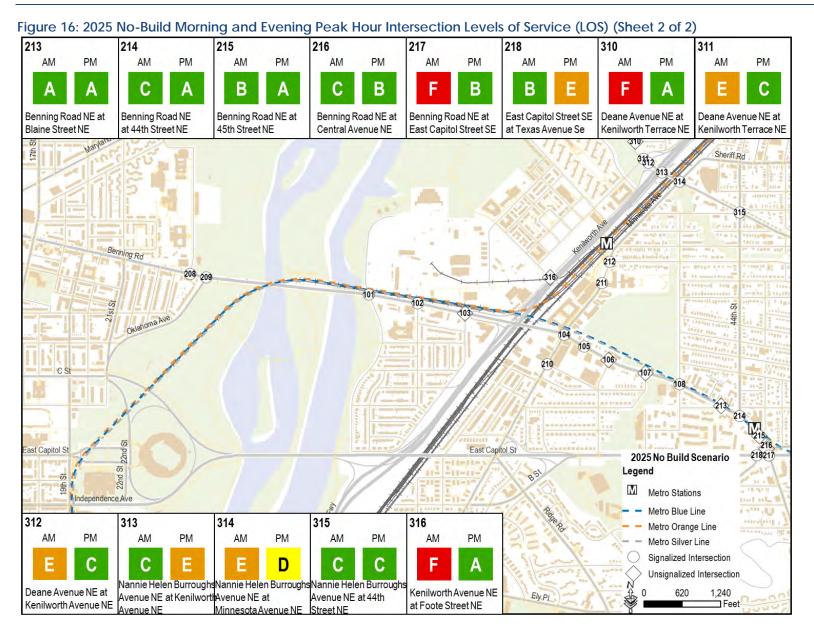


Figure 14: 2025 No-Build Morning and Evening Peak Hour Intersection Traffic Volumes (Sheet 2 of 2)







APPENDIX E-45

# 4.3 Opening Year 2025 Build Travel Demand Forecasting

A future build scenario, which included the proposed action, was analyzed for the year 2025 to test the alternatives developed as part of the Benning Road EA. This scenario combined regional baseline assumptions with a more fine-grained roadway system to predict travel patterns, transit usage and vehicular turning movements in the project area.

### 4.3.1 2025 Build Land Use

For the build scenario, the regional land use of the travel demand analysis area was the same as for the No-Build scenario, using the MWCOG Round 9.1 Cooperative Land Use Forecasts for 2025.

# 4.3.2 2025 Build Transportation Network

The transportation networks in the build scenario is based on the regional CLRP transportation networks which is the same as the No-Build scenario, except it includes the proposed action. The proposed frequency of service for the line is 10 minutes in both directions through the entire service day.

# 4.3.3 2025 Build Ridership Forecasting

Simplified Trips-on-Project Software (STOPS) (version 2.5) was used to forecast ridership for the proposed extension of streetcar transit service to the Benning Road Metrorail station. STOPS was developed by the Federal Transit Administration (FTA) and has been widely used in ridership analysis and forecasting for transit planning and development projects. STOPS estimates daily ridership patterns using typical planning data including current and projected regional land use and demographic data. It reports line-level and stop-level ridership, but it does not report peak versus off-peak trips separately. It is also able to model both local and regional transit networks, including the first phase of the DC Streetcar and the planned extension. STOPS provides an order of magnitude estimate of line-level ridership but is less precise at the stop-level.

For the STOPS planning-level analysis it is assumed that in 2025 the H Street Bridge construction will be completed, and the Streetcar stop at Union Station will be operational with the relevant federal and local certifications granted for revenue service. The actual timing of the H Street Bridge construction work and completion is unknown at this time and it is expected to impact Streetcar ridership and operations. The STOPS model also does not fully capture any impacts of the H Street Bridge construction (including the closure of the Union Station Streetcar stop) on transit ridership (Metrobus and DC Streetcar) and traffic patterns, and the time required for travel patterns to normalize after completion of the bridge. In addition, special markets and associated transit trips are not modeled. Special markets include those related to tourism, recreation, and sporting events.

STOPS may not accurately differentiate between overlapping services of the same mode, therefore bus routes along shared corridors should be looked at together. The STOPS analysis also does not

include any changes to bus routes or bus schedules with the introduction of the Benning Road Streetcar Extension.

**Table 17** summarizes the projected streetcar boardings at each stop under the No-Build (terminating at Oklahoma Avenue) and the build (includes extension to Benning Road Metrorail Station) scenarios.

Table 17: 2025 No-Build and Build Weekday Streetcar Ridership

		2025	
Stop	No-		
	Build	Build	Change
Union Station	1,240	1,510	+270
3 <sup>rd</sup> Street and H Street NE	210	590	+380
5th Street and H Street NE	150	210	+60
8th Street and H Street NE	250	350	+100
13th Street and H Street NE	390	460	+70
15th Street and Benning Road NE	390	700	+310
19th Street and Benning Road NE	480	720	+240
Oklahoma Avenue and Benning Road NE	600	530	-70
Kingman Island and Benning Road NE	-	10	+10
34th Street and Benning Road NE	-	440	+440
39th Street and Benning Road NE	-	400	+400
42nd Street and Benning Road NE	-	360	+360
Benning Road Metrorail Station	-	1,310	+1,310
Total	3,710	7,590	+3,880
Existing Stops	3,710	5,070	+1,360
Proposed Stops	-	2,520	+2,520

It is projected that in 2025, combined weekday boardings at the proposed stops along the Benning Road Extension would be approximately 2,500 riders. This is in addition to a projected 5,000 total riders boarding at stops along the existing line between Union Station and Oklahoma Avenue, 27% of which are riders projected to be generated by the addition of the extension.

**Table 18** provides weekday bus ridership under the No-Build and build scenarios for 2025 for key routes or lines that serve the study area. These numbers include ridership for the entire routes, not just ridership at stops within the study area. Compared to the 2025 No-Build scenario, the total daily weekday bus ridership in the study area (bus stops within a quarter mile of an existing or proposed Streetcar stop) would decrease by approximately 6 percent in the 2025 Build scenario—from 16,800 to 15,700 daily weekday riders. This decrease may be explained by the introduction of the Streetcar service, which offers faster service at a higher frequency, particularly during the offpeak periods because the Streetcar would operate with the same 10-minute headway during both the peak and off-peak, thereby reducing passenger waiting time between the Benning Road Metrorail Station and Union Station.

Table 18: 2025 No-Build and Build Weekday Bus Ridership (Boardings)

		2025	
Stop	No- Build	Build	Change
X Line (X1, X2, X3, X9)	17,490	16,480	-1,010
U4	1,900	1,880	-20
U5, U6	2,410	2,340	-70
U8	2,570	2,250	-320
96, 97	7,880	7,700	-180
W4	8,170	8,240	+70
Total	40,420	38,890	-1,530
Percent Difference Compared to No-Build		-3.8%	

# 4.4 Opening Year 2025 Build VISSIM Simulation Modeling

The build scenario assumes the same transportation network as in the No-Build, with the exception of two changes: (1) the introduction of the streetcar from 26<sup>th</sup> Street to Benning Road Metrorail Station; and (2) proposed lane changes at the intersection of Benning Road and Minnesota Avenue to improve traffic operations. Intersection improvements at Benning Road and East Capitol Street intersection to reduce vehicular delay are beyond the scope and therefore not considered in this analysis.

The proposed lane configuration at Benning Road and Minnesota Avenue intersection is shown in **Figure 17**. Visual observations and sensitivity tests showed potential capacity failure at this intersection, particularly for the southbound Minnesota Avenue intersection. Therefore, the proposed lane configuration changes are found to be more effective in 2045. This will be discussed in further detail in **Section 5**.

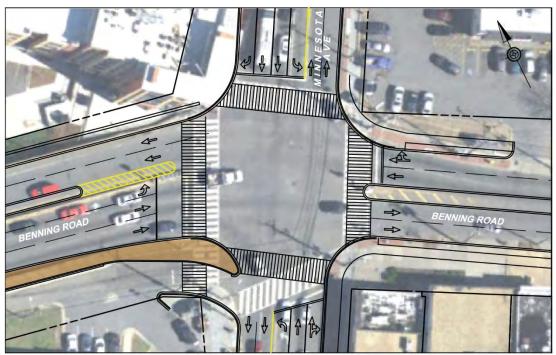


Figure 17: Proposed Lane Configuration at Benning Road and Minnesota Avenue Intersection

## The proposed improvements include:

- Providing one additional through lane (two in total) on the southbound approach of Minnesota Avenue with a 100-foot left turn pocket
- Modifying the southbound approach with a 200-foot right turn pocket
- Restricting the westbound left turn on Benning Road to increase through-lane capacity

The Build scenario also considered special transit-only signals at certain intersections where streetcar tracks transition to an adjacent lane. The intersections where transitions occur for curbside and median alignments are described below:

# Eastbound Curbside Streetcar Alignment

- Benning Road and Oklahoma Avenue intersection: Transition from the recently constructed median tracks to a track in the curb lane.
- Benning Road and 34th Street intersection: Transition from the curb lane onto the third lane to continue in curb lane on the eastbound structure over DC-295 and CSX Railroad.
- Benning Road and 45<sup>th</sup> Street intersection: Transition from the curb lane to the Benning Road Station terminus. This transition would require signalization of the intersection as it currently operates as an unsignalized intersection.

#### Westbound Curbside Streetcar Alignment

 Benning Road and 45<sup>th</sup> Street intersection: Transition from the Benning Road Station terminus to a track in the curb lane.

Benning Road and 36th Street intersection: Transition from the curb lane of the bridge to the curb
of Benning Road. Only westbound -ramp traffic from Kenilworth Avenue would be stopped to
allow the streetcar to make this transition.

### Eastbound Median Streetcar Alignment

 Benning Road and 45<sup>th</sup> Street intersection: Transition from the median tracks to the Benning Road Station terminus.

# Westbound Median Streetcar Alignment

- Benning Road and 45<sup>th</sup> Street intersection: Transition from the median tracks to the Benning Road Station terminus.
- Benning Road, west of Kingman Island stop: Transition from the fourth lane to the third lane to align track with the existing tail track extending from the Benning Road and Oklahoma Avenue intersection. This would require a new signalized intersection at Kingman Island to stop westbound traffic to allow this transition.

## 4.4.1 2025 Build Traffic Volume Development

Same forecasting methods for the 2025 No-Build scenario were used to develop the 2025 Build scenario. **Figure 18**, **Figure 19**, and

Figure 20 show the freeway and intersection traffic volume in 2025 Build scenario.

### 4.4.2 2025 Build Conditions

A comparison 2025 No-Build and Median Alignment Build revealed several findings:

In the morning peak, travel times were comparable between the No-Build and Streetcar Build scenario for both directions of Benning Road NE and DC-295. Southbound DC-295 was very congested over the entire AM analysis period in 2019 and the congestion did not increase much in 2025 due to little increase in forecasted demand due to capacity constraints. Unresolved bottlenecks downstream of the study area are expected to govern.

In the evening peak, Benning Road NE eastbound and westbound travel times increased by between 30 seconds and one (1) minute from the 2025 No-Build to 2025 Streetcar Build scenario. In the eastbound direction, this travel time increase was primarily observed west of the Benning Road and Minnesota Avenue intersection. In the westbound direction, this travel time increase was observed primarily east of the Benning Road and Minnesota Avenue intersection, but slightly impacted by the lane reductions on Benning Road NE to the west.

In the morning peak, the intersection of Benning Road NE and Oklahoma Avenue NE operated with more delay under the 2025 No-Build scenario than the 2025 Streetcar Build scenario. This difference was due to the changes in volume on westbound Benning Road NE. Under the Streetcar Build scenario, the westbound approach demand was approximately 170 vehicles fewer than in the No-Build, primarily due to the proposed lane reduction on Benning Road. The intersections of Benning Road NE with 45th Street NE and Central Avenue NE operated with similar delay under the Streetcar Build scenario and No-Build scenario, suggesting that the added signal for the EOL did not have significant impact in the near-term. The streetcar experienced approximately 40 seconds of delay at this intersection, and when present, blocked the leftmost eastbound through lane. Congestion and significant weaving patterns were observed west of the Benning Road NE and DC-295 Interchange under the Streetcar Build scenario, where a lane reduction began in the westbound direction. The impact of this weaving area was amplified by a bus stop on the rightmost lane just west of the Kenilworth Avenue NE and Benning Road NE merge. Queue lengths greater than 400 feet are observed under the 2025 Streetcar Build scenario at the signalized crosswalk across southbound Kenilworth Avenue at westbound Benning Road NE. The impact of increased pedestrian volumes on vehicular operations here should be monitored.

In the evening peak, at the intersections of Benning Road NE with Anacostia Avenue NE and 34th Street NE, the eastbound approach delay increased slightly from No-Build to Streetcar Build due to the lane reduction on Benning Road NE. Traffic operations at the intersection of Benning Road NE and 36th Street NE were worse under the No-Build than the Streetcar Build scenario due to queue spillback from the eastbound Benning Road NE on-ramp to northbound DC-295. The intersection of East Capitol Street and Texas Avenue SE operated at LOS E under both the 2025 No-Build and Streetcar Build scenarios. This decrease in intersection LOS relative to existing conditions can be explained by the road diet planned for East Capitol Street. While the eastbound demand was forecasted to decrease slightly with the reduction of a travel lane, the eastbound

approach experienced the largest increase in delay relative to existing conditions. At the intersection of Benning Road NE and Minnesota Avenue NE, intersection LOS was C under both the 2025 No-Build and Streetcar Build scenarios. The proposed design and signal timing at this intersection was effective in processing forecasted demand, and intersection delay only increased by five seconds under the Streetcar Build scenario. Intersections along Nannie Helen Burroughs Avenue NE experienced increased delay due to higher volumes under the 2025 No-Build and Streetcar Build scenarios relative to existing conditions. There was significant queue spillback at the intersections with the northbound DC-295 offramp and with Minnesota Avenue NE. In general, the operation of streetcar service, including transitions, results in typically very marginal increase in intersection delay.

The findings of the 2025 Curbside Build and Median Alignment Build traffic analysis are summarized below:

- Benning Road and East Capitol Street intersection operated with LOS F in the morning peak and evening peak hours under Curbside alignment but LOS B under Median alignment in the evening peak.
- During evening peak, intersection LOS at Benning Road at 44<sup>th</sup> Street was B under Median alignment but downgraded to E under curbside alignment and from LOS F to LOS D with the median alignment.

Figure 21 provides intersection LOS at the project area intersections during the morning and evening peak hours with the curbside alignments and

Figure 22 and Figure 23 show the operational results under median alignment. Table 19 provides delay and LOS by movement for the critical intersections for the 2025 Build curbside alignment. **Table 20** displays the associated queue lengths by movement.

#### Table 21 and

Table 22 provide delay and queuing results for the median alignment, respectively.

**Attachment B** provides the detailed results for all project area intersections for the 2025 Build Alternatives.

Table 19: 2025 Build Curbside Alignment Peak Hour (AM and PM) Delay and Levels of Service (LOS) at the Critical Intersections

Intersection	Traffic	Peak	Inters	ection	North	bound	South	bound	Westl	oound	Easth	ound
Intersection	Control	Hour	Delay	LOS	Delay	LOS	Delay	LOS	Delay	LOS	Delay	LOS
Benning Road and East Capitol Street	Signalized	AM	181	F	285	F	62	Е	127	F	386	F
Benning Road and 44 <sup>th</sup> Street	Signalized	PM	60	Е	18	В	99	F	35	С	-	-
Benning Road and 45 <sup>th</sup> Street	Unsignalize d	PM	55	D	3	A	99	F	28	С	34	С
Benning Road and East Capitol Street	Signalized	PM	166	F	287	F	69	Е	251	F	148	F

<sup>\*</sup> The curbside results are based on the original AECOM study.

Table 20: 2025 Build Curbside Alignment Peak Hour (AM and PM) Maximum Queue Length (feet) at the Critical Intersections

Intersection	Peak	Northbound			So	uthbou	nd	W	/estboun	ıd	E	astboun	d
Intersection	Hour	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Benning Road and East Capitol Street	AM	1,070	1,070	1,070	320	320	330	1,675	1,675	1,675	1,465	1,465	1,465
Benning Road and 44 <sup>th</sup> Street	PM	-	235	235	830	830	-	255	-	255	-	-	-
Benning Road and 45 <sup>th</sup> Street	PM	135	135	135	470	470	470	105	55	85	30	30	30
Benning Road and East Capitol Street	PM	1,015	1,015	1,015	365	365	375	1,030	1,030	1,030	1,465	1,465	1,465

<sup>\*</sup> The curbside results are based on the original AECOM study.

Table 21: 2025 Build Median Alignment Peak Hour (AM and PM) Delay and Levels of Service (LOS) at the Critical Intersections

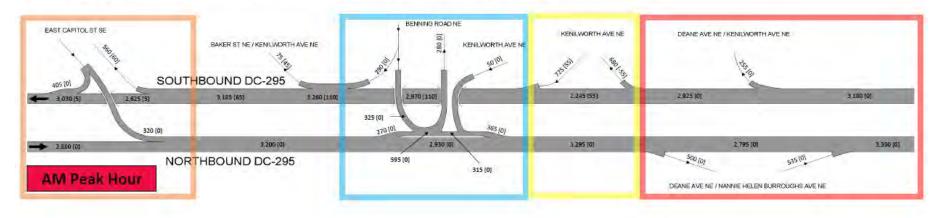
Intersection	Traffic Control	Peak	k Intersection		Northbound		Southbound		Westbound		Eastbound	
		Hour	Delay	LOS	Delay	LOS	Delay	LOS	Delay	LOS	Delay	LOS
Benning Road NE at East Capitol Street SE	Signalized	AM	106	F	12	В	210	F	50	D	19	В
East Capitol Street SE at Texas Avenue SE	Signalized	PM	75	E	87	F			6	A	113	F
Deane Avenue NE at Lee St NE	Unsignalized	AM	304	F			426	F	1	A	387	F
Deane Avenue NE at Kenilworth Terrace NE	Signalized	AM	93	F	154	F	107	F	4	A	63	Е
Deane Avenue NE at Kenilworth Avenue NE	Signalized	AM	58	E			291	F	66	E	9	A
Nannie Helen Burroughs Avenue NE at Kenilworth Avenue NE and DC- 295 U-turns	Signalized	PM	67	E	154	F			23	С	31	С
Nannie Helen Burroughs Avenue NE at Minnesota Avenue NE	Signalized	AM	93	F	349	F	100	F	43	D	24	С
Kenilworth Avenue NE at Foote Street NE	Unsignalized	AM	163	F			190	D			14	В

Table 22: 2025 Build Median Alignment Peak Hour (AM and PM) Maximum Queue Length (feet)

Intersection	Peak Hour	Northbound	Southbound	Westbound	Eastbound
Benning Road NE at East Capitol Street SE	AM	438	410	1781	232
Benning Road NE at East Capitol Street SE	PM	235	402	219	1335
East Capitol Street SE at Texas Avenue SE	AM	450	95	-	-
East Capitol Street SE at Texas Avenue SE	PM	232	156	-	-
Benning Road NE and Minnesota Avenue NE	AM	353	533	412	270
Benning Road NE and Minnesota Avenue NE	PM	442	445	285	673

# Figure 18: 2025 Build Morning and Evening Peak Hour Freeway Traffic Volumes

\* Numbers on Figures denote: Forecasted Volumes [Vehicle difference from 2025 No-Build without Streetcar and Interchange (Scenario 5)]



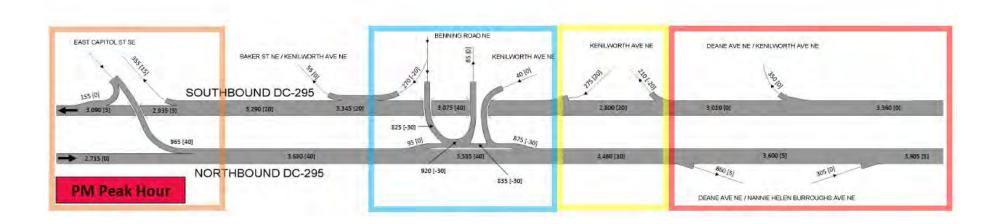


Figure 19: 2025 Build Morning and Evening Peak Hour Intersection Traffic Volumes (Sheet 1 of 2)

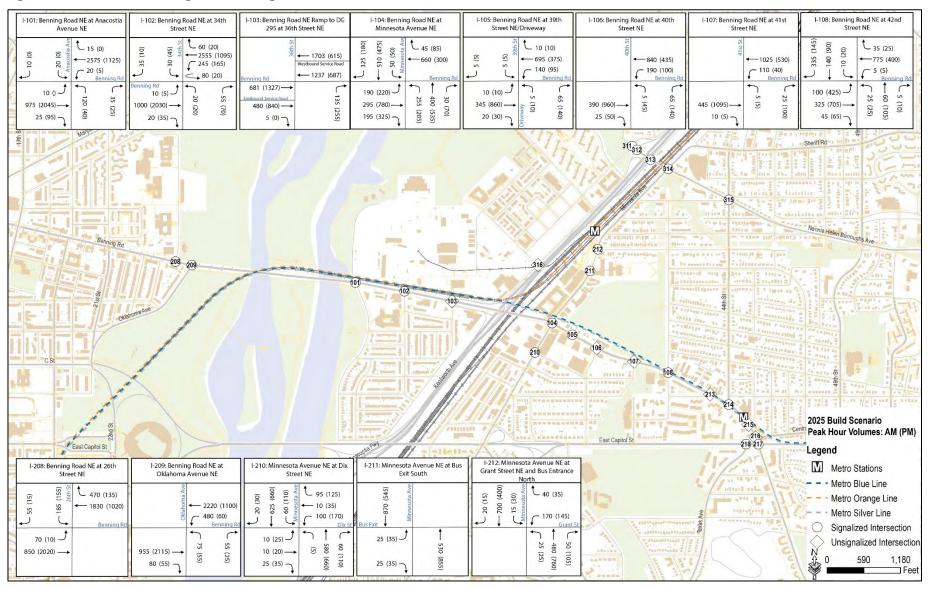
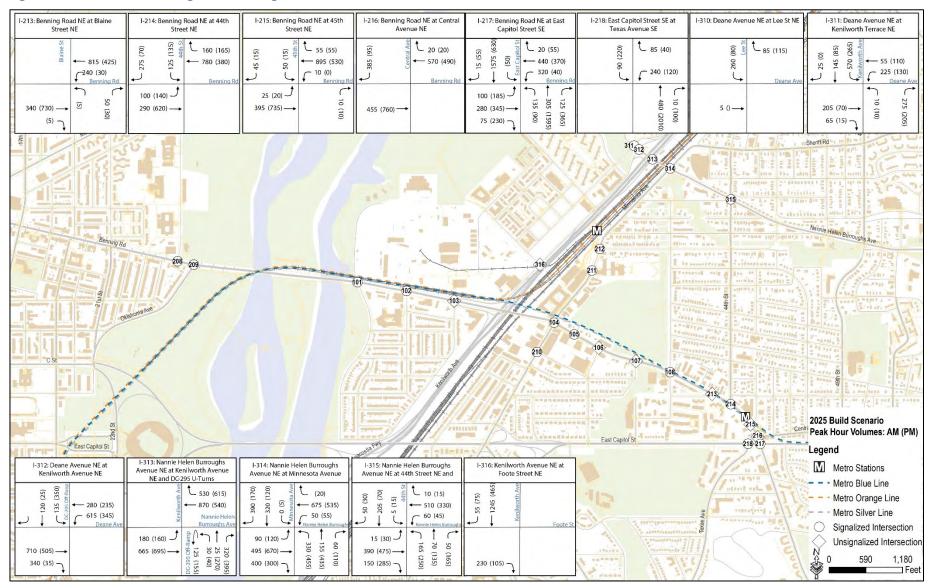


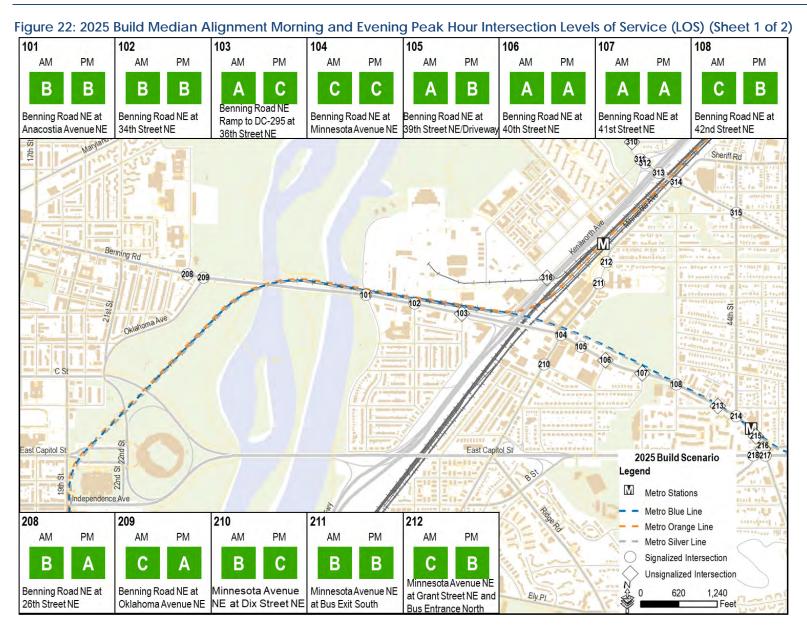
Figure 20: 2025 Build Morning and Evening Peak Hour Intersection Traffic Volumes (Sheet 2 of 2)

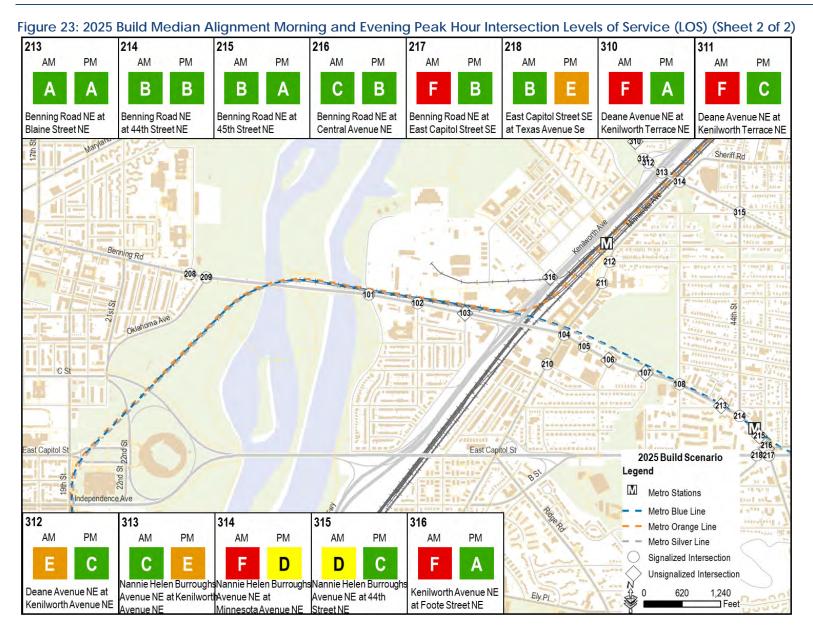


D Benning Rd and 26th St Benning Rd and Anacostia Ave Benning Rd and 34th St Benning Rd and Minnesota Ave Benning Rd and 42nd St Benning Rd and 44th St Benning Rd and Oklahoma Ave Langston Course Minnesota Ave Kingman (11) Legend Level of Service (LOS): Metrorail Station Metrorail Blue Line Benning Road Metrorail Orange Line Signalized Intersection EAST CAPITOL ST Unsignalized Intersection Fort Chaplin 1,200 Feet 10 11 12 13 14 15 PM PM Benning Rd and Benning Rd and Central Ave Benning Rd and Minnesota Ave and E Capitol St Gault PI Grant St Hayes St NHB Ave

Figure 21: 2025 Build Curbside Alignment Morning and Evening Peak Hour Intersection Levels of Service (LOS)

<sup>\*</sup> The curbside results are based on the original AECOM study.





## 4.4.3 2025 Build Streetcar Operations

To evaluate the operation of the streetcar alignments, average travel speeds were obtained from the VISSIM simulation model. VISSIM travel time segments for curbside alignment are defined from 20th Street to Benning Road Metrorail Station in the eastbound direction and from Benning Road Metrorail Station to 26th Street in the westbound direction. Travel time segments for median alignments are defined from 26th Street to Benning Road Metrorail Station in the eastbound direction and from Benning Road Metrorail Station to 26th Street in the westbound direction. **Table 23** provides average travel time and speed for the curbside and median streetcar alignment in the morning and evening peak hours.

Table 23: 2025 Average Travel Time and Speed for the Streetcar Alignments in the Morning and Evening Peak Hour

Direction	Peak Hour	Peak Hour Travel Time (min)							
Curbside Streetcar Alignment									
Eastbound	A N (	10.0	12.8						
Westbound (peak direction)	AM	10.9	10.3						
Eastbound (peak direction)	D) (	13.9	9.2						
Westbound	PM	9.6	11.7						
	Median Stre	eetcar Alignment							
Eastbound	AM	7.3	15.4						
Westbound (peak direction)	AlVI	7.3	15.4						
Eastbound (peak direction)	D) (	7.9	14.2						
Westbound	PM	7.1	15.8						

st The curbside results are based on the original AECOM study.

Results show that the curbside alignment operates with relatively slower speeds compared to the median alignment during the both peak hours. This can be attributed to the higher number of transitions for the curbside alignment, which increases intersection delay as no signal pre-emption was provided for the special streetcar signals to limit the disruption of general traffic. Another important finding is that the average streetcar speed is generally higher than 10mph, faster than the typical peak period bus speeds in downtown Washington DC2, as a result of off-board fare collection, level boarding, and relatively larger spacing between streetcar stops.

**Table 24** and **Table 25** show stop to stop travel times for both Build Alternatives in 2025 based on VISSIM results. Vehicle travel times along Benning Road for the same travel segments were also presented in **Table 26** and

**Table** 27 for comparison purposes.

For both morning and evening peak, travel times (excluding dwell time at the streetcar stops) for the streetcar vehicles were slightly higher than t/hose for vehicular traffic in both directions of

<sup>&</sup>lt;sup>2</sup> http://www.wmata.com/pdfs/planning/November2009\_AMSpeedMap.pdf

Benning Road NE. The biggest difference existed along the segment from 42<sup>nd</sup> Street NE to 45<sup>th</sup> Street NE due to long delays and eastbound queue impacts at the Streetcar End of Line (EOL).

Table 24: 2025 Station to Station VISSIM Travel Time Results for Curbside Streetcar Alignment

	Curbside Alignment						
Segment	Travel Time (min) – AM Peak Hour	Travel Time (min) – PM Peak Hour					
Eastbound Direction							
20th Street to Oklahoma Avenue	0.7	0.8					
Oklahoma Avenue to Kingman Island	2.0	2.1					
Kingman Island to 34th Street	1.0	1.3					
34th Street to Minnesota Avenue	3.3	3.6					
Minnesota Avenue to 42nd Street	1.0	1.3					
42nd Street to Benning Road Metrorail Station	2.0	4.9					
TOTAL	10.0	13.9					
Ţ	Westbound Direction						
Benning Road Metrorail Station to 42nd Street	2.0	1.6					
42nd Street to Minnesota Avenue	2.0	1.2					
Minnesota Avenue to 34th Street	2.3	3.0					
34th Street to Kingman Island	1.4	1.2					
Kingman Island to Oklahoma Avenue	2.7	2.2					
Oklahoma Avenue to 26th Street	0.5	0.4					
TOTAL	10.9	9.6					

<sup>\*</sup> The curbside results are based on the original AECOM study.

Table 25: 2025 Station to Station VISSIM Travel Time Results for Median Streetcar Alignment

	Median Alignment					
Segment	Travel Time (min) – AM Peak Hour	Travel Time (min) – PM Peak Hour				
	Eastbound Direction					
20th Street to Oklahoma Avenue	0.1	0.1				
Oklahoma Avenue to Langston Golf Course	1.2	1.0				
Langston Golf Course to 34th Street	1.6	1.8				
34th Street to 39th Street	1.7	2.0				
39th Street to 42nd Street	0.9	1.0				
42 <sup>nd</sup> Street to Benning Road Metrorail Station	2.0	2.0				
TOTAL	7.3	7.9				
	Westbound Direction					
Benning Road Metrorail Station to 42 <sup>nd</sup> Street	1.4	1.2				
42nd Street to 39th Street	0.9	0.9				
39th Street to 34th Street	2.1	2.5				
34th Street to Langston Golf Course	1.3	1.1				
Langston Golf Course to Oklahoma Avenue	1.0	1.0				

	Median Alignment					
Segment	Travel Time (min) – AM Peak Hour	Travel Time (min) – PM Peak Hour				
Oklahoma Avenue to 20th Street	0.5	0.4				
TOTAL	7.3	7.1				

Table 26: 2025 Corridor Vehicle Travel Times under Curbside Alignment

	Curbside Streetcar					
Segment	Travel Time (min) – AM Peak Hour	Travel Time (min) – PM Peak Hour				
Eastbound Direction						
20 <sup>th</sup> Street to Benning Road Metrorail Station	6.3	9.9				
Westbound Direction						
Benning Road Metrorail Station to 26 <sup>th</sup> Street	6.0	5.3				

<sup>\*</sup> The curbside results are based on the original AECOM study.

Table 27: 2025 Corridor Vehicle Travel Times under Median Alignment

	Median Streetcar					
Commont	Travel Time (min) – AM Peak	Travel Time (min) – PM Peak				
Segment	Hour	Hour				
Eastbound Direction						
26 <sup>th</sup> Street to Benning Road Metrorail Station	5.2	5.8				
Westbound Direction						
Benning Road Metrorail Station to 26 <sup>th</sup> Street	6.0	5.5				

[This space left intentionally blank.]

# 5.0 Design Year 2045 Conditions

This section describes transit and general traffic conditions in the project area for the design year 2040. Like the opening year analysis, curbside and median alignments were tested as part of the Build scenario and their impact on general traffic is evaluated.

# 5.1 Design Year 2045 No-Build Travel Demand Forecasting

For the 2045 No Build scenario, both inputs were based on regionally accepted baseline conditions for 2045, including the MWCOG Round 9.1 Cooperative Land Use Forecasts and the regional CLRP.

#### 5.1.1 2045 No-Build Land Use

The regional land use was determined based on the MWCOG Round 9.1 Cooperative Land Use Forecasts for 2045. Regionally, this represents growth of approximately 30 percent in population and 59 percent in employment over the existing conditions. **Table 28** shows the total population and employment by sector in the project area for 2045.

Table 28: 2045 No-Build Population and Employment Growth

	2019 Total	Growth Factor	Growth between 2019-2045	2045 Total
Households	18,418	1.97%	51.21%	27,850
Population	46,878	1.85%	48.17%	69,457
Total Employment	6,786	4.87%	126.66%	15,381

### 5.1.2 2045 No-Build Transportation Network

As discussed in the 2025 No Build Transportation Network Section (**Section 4.1.2**), the only change to the roadway network in the project area is the removal one of the three lanes in each direction along East Capitol Street from 40<sup>th</sup> Street to Southern Avenue to improve pedestrian safety. As a result, East Capitol Street will operate with two through lanes in both directions between 40<sup>th</sup> Street and Southern Avenue.

#### 5.1.2.1 Metrorail

Regional Metrorail service improvements provide some changes to the services provided at the two stations within the project area. **Table 29** summarizes the peak and off-peak headways of the assumed Metrorail system in 2045.

Table 29: Peak and Off-Peak Headways of Metrorail Lines (2045)

Line	Terminus A	us A Terminus B Headway (minutes		Off-Peak Headway (minutes)
Blue	Franconia/Springfield	Largo	7	12
Orange	Vienna	New Carrollton	6	12
Silver	VA 772	Armory	7	12

#### 5.1.2.2 Local Bus

Local bus service in the project area is provided primarily by Metrobus. **Table 30** summarizes the peak and off-peak headways for the local routes serving the project area. The table shows morning peak frequencies; evening peak frequencies are assumed to be directionally reversed.

Table 30: Local Bus Frequency (2045)

Route	Direction	Morning Peak Headway (minutes)	Off-Peak Headway (minutes)
06.07	Eastbound	20	60
96,97	Westbound	20	60
TIO	Westbound	20	30
U2	Southbound	30	30
U4	Clockwise	12	30
IIE IIC	Northbound	30	38
U5,U6	Southbound	14	14
TIO	Northbound	12	12
U8	Southbound	60	60
W7.0.0	Eastbound	20	30
V7,8,9	Westbound	20	27
TA7.4	Southbound	30	60
W4	Northbound	12	60
V1 V2	Eastbound	30	-
X1,X3	Westbound	15	-
V2	Eastbound	6	8
X2	Westbound	7	9
1/0	Eastbound	10	15
Х9	Westbound	10	15

# 5.2 Design Year 2045 No Build VISSIM Simulation Modeling

This section describes the development of the 2045 No Build VISSIM model and provides the summary of the traffic analysis. Like the 2025 VISSIM models, the number of through lanes on East Capitol Street is reduced from three lanes to two lanes in both directions to accommodate the changes that are described in CLRP.

### 5.2.1 2045 No Build Traffic Volume Development

Traffic volume growth rates for the links in the subarea were estimated by developing 2045 O-D matrices for AM and PM peak periods, assigning this demand to the VISUM network, and comparing against the 2019 traffic assignment. The 2045 O-D matrices for AM and PM peak periods were developed from the corrected 2019 O-D matrices using three methods:

- Pass-through trips were grown for each cordon zone O-D pair using the applicable growth from the MWCOG model (2019 to 2045). Adjustments previously made to the 2019 MWCOG trip tables based on StreetLight data were also made to the 2045 trip tables prior to determining growth rates;
- For zones with projects in the development pipeline, development-based trip growth was used.
   The developments taken into consideration includes Hill East, Northeast Heights and Parkside; and
- For zones without projects in the development pipeline, uniformly applied annual growth rates of 0.92% and 0.84% for the AM and PM peak periods, respectively, were used. These growth rates were developed from the total trips for these zones in the MWCOG model.

Figure 24, Figure 19, and

**Figure** 26 shows the projected peak hour traffic volumes in freeway and intersections for the year 2045.

#### 5.2.2 2045 No Build Conditions

In general, the trends in traffic operations observed throughout the network are consistent with those of 2025 No-Build but the increases in delay at the intersection and corridor level were more substantial under the 2045 scenario due to higher traffic volumes.

In the morning peak, queue spillback from the westbound left turn at Benning Road NE/Oklahoma Avenue NE intersection was present over an extensive period during the morning peak, causing turbulence to westbound traffic flow on Benning Road. Difficulty for buses to maneuver to the stop on westbound Benning Road NE at 34th Street NE was observed in the model due to large traffic flows from westbound Kenilworth Service Road. Congestion and queueing existed on westbound Benning Road NE between Minnesota Avenue NE and 42nd Street NE. Delay on this segment decreased slightly under the 2045 No-Build scenario due to the realignment of 39th Street NE, which eliminated split phasing at this intersection. Congestion and queueing were present on southbound 42nd Street NE and 44th Street NE due to large turning volumes and lack of dedicated turn lanes. Higher delay and larger queuing impacts existed on Nannie Helen Burroughs Avenue NE and the associated minor streets in comparison to Existing Conditions due to increased demand and challenging intersection geometry and signal phasing. Delays were especially high the at Kenilworth Avenue NE and Kenilworth Terrance NE intersections with Deane Avenue NE due to large traffic flows from southbound Kenilworth Avenue NE to eastbound Deane Avenue NE and to southbound DC-295.

In the evening peak, the intersection of the northbound DC-295 off-ramp with Nannie Helen Burroughs Avenue NE operated with nearly 100 additional seconds of delay under the 2045 No-Build scenario, increasing the potential for queue spillback onto the freeway. This deterioration in LOS was caused by an increase in demand, especially for the northbound right turn movement. Adjacent intersections along Nannie Helen Burroughs Avenue NE were also adversely impacted.

To accommodate the future year volumes, split timing and offsets for traffic signals were optimized at critical intersections while maintaining the existing cycle length. Figure 27 and

Figure 28 display intersection LOS at the project area intersections for the morning and evening peak hours. Table 31 provides delay and LOS by movement for the critical intersections for the 2045 No-Build for the morning and evening peak hours.

Table 32 shows the maximum delay in associated intersections. Detailed operational results for all project area intersections for the 2045 No-Build conditions are provided in Attachment C.

Table 31: 2045 No-Build Peak Hour (AM and PM) Delay and Levels of Service (LOS) at the Critical Intersections

Intersection	Traffic	Peak	Interse	ection	Northl	ound	South	ound	Westb	ound	Eastbo	ound
Intersection	Control	Hour	Delay	LOS	Delay	LOS	Delay	LOS	Delay	LOS	Delay	LOS
Benning Road NE Ramp to DC-295 at 36th Street NE	Unsignalized	PM	140	F	280	F					104	F
Benning Road NE at Oklahoma Avenue NE	Signalized	AM	56	E	39	D			75	E	13	В
Benning Road NE at East Capitol Street SE	Signalized	AM	104	F	17	В	215	F	53	D	19	В
East Capitol Street SE at Texas Avenue SE	Signalized	PM	73	E	80	E			8	A	112	F
Deane Avenue NE at Lee Street NE	Unsignalized	AM	387	F			516	F	1	A	394	F
Deane Avenue NE at Kenilworth Terrace NE	Signalized	AM	179	F	268	F	258	F	5	A	57	E
Deane Avenue NE at Kenilworth Avenue NE	Signalized	AM	62	E			314	F	66	E	8	A
Nannie Helen Burroughs Avenue NE at Kenilworth Avenue NE and DC- 295 U-turns	Signalized	PM	154	F	339	F			45	D	33	С
Nannie Helen Burroughs Avenue NE at Minnesota Avenue NE	Signalized	AM	155	F	364	F	366	F	49	D	29	С
Nannie Helen Burroughs Avenue NE at Minnesota Avenue NE	Signalized	PM	176	F	182	F	130	F	512	F	31	С
Kenilworth Avenue NE at Foote Street NE	Unsignalized	AM	192	F			226	F			17	В

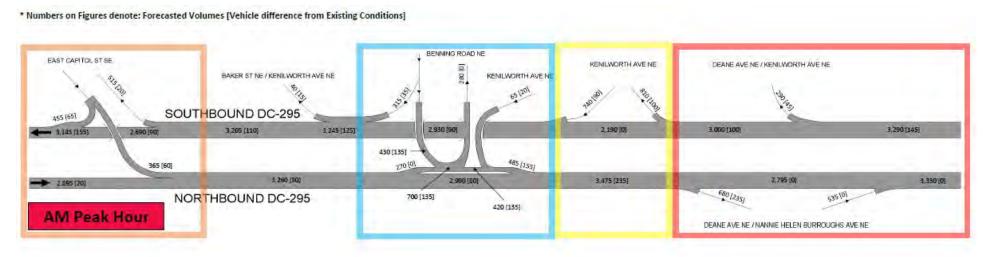
Nannie Helen	Signalized	PM	177	F	360	F	81	F	404	F	23	С
Burroughs												
Avenue NE at												
44th Street NE and												
Hunt Place NE												

Table 32: 2045 No-Build Peak Hour (AM and PM) Maximum Queue Length (feet)

Intersection	Peak Hour	Northbound	Southbound	Westbound	Eastbound
Benning Road NE at East Capitol Street SE	AM	555	903	1776	247
Benning Road NE at East Capitol Street SE	PM	292	522	240	1335
East Capitol Street SE at Texas Avenue SE	AM	540	86	-	-
East Capitol Street SE at Texas Avenue SE	PM	204	278	-	-
Benning Road NE and Minnesota Avenue NE	AM	478	532	389	382
Benning Road NE and Minnesota Avenue NE	PM	373	454	313	645

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Figure 24: 2045 No-Build Morning and Evening Peak Hour Freeway Traffic Volumes



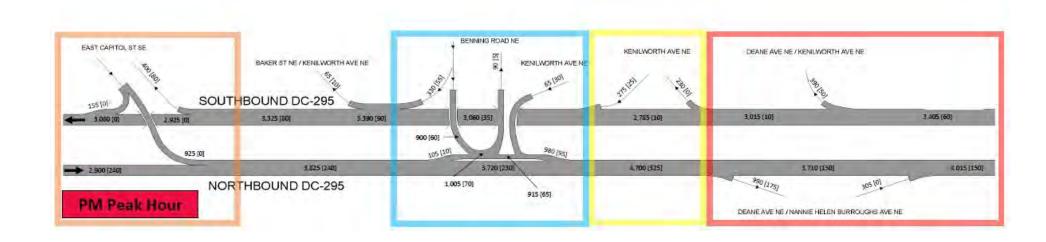


Figure 25: 2045 No-Build Morning and Evening Peak Hour Intersection Traffic Volumes (Sheet 1 of 2)

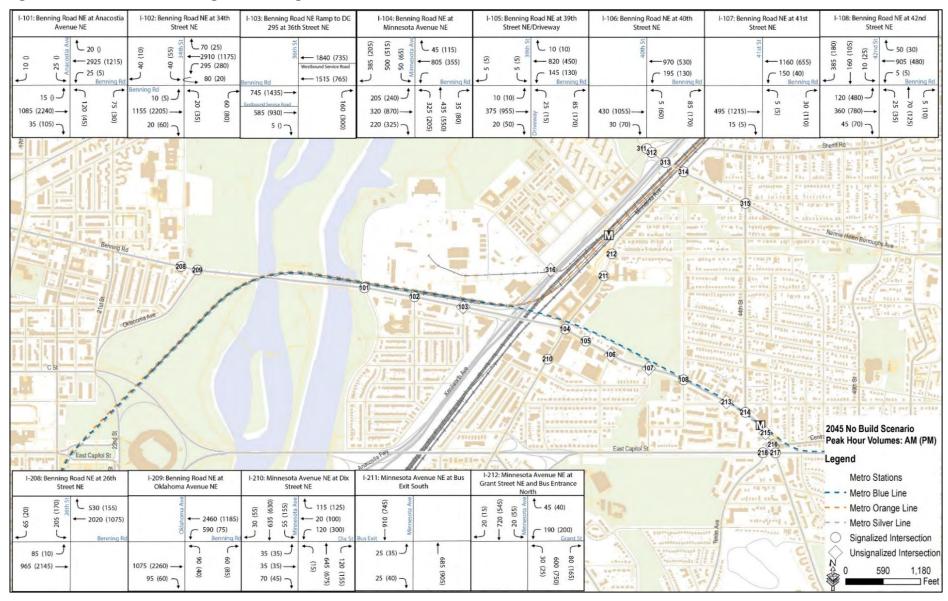


Figure 26: 2045 No-Build Morning and Evening Peak Hour Intersection Traffic Volumes (Sheet 2 of 2)

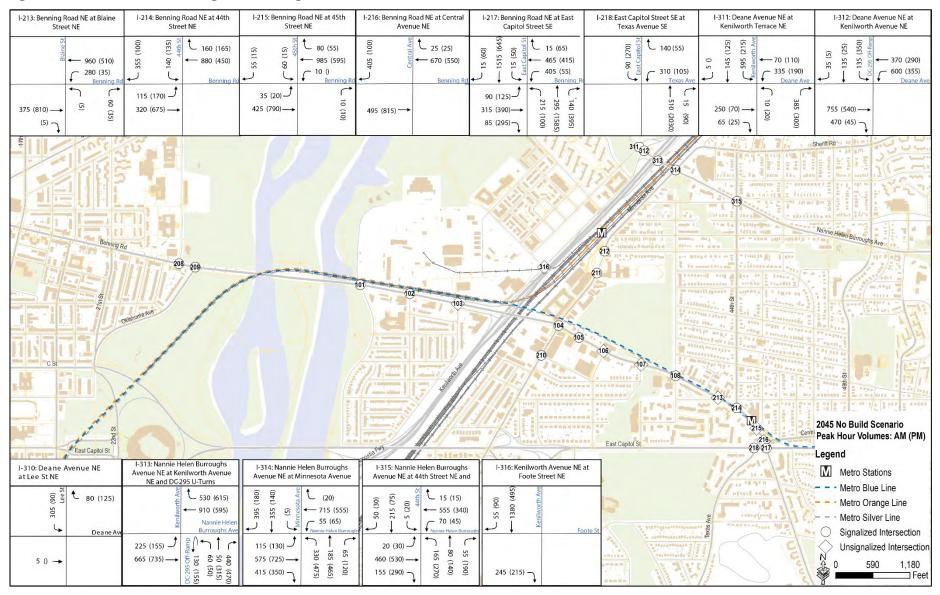


Figure 27: 2045 No-Build Morning and Evening Peak Hour Intersection Levels of Service (LOS) (Sheet 1 of 2)

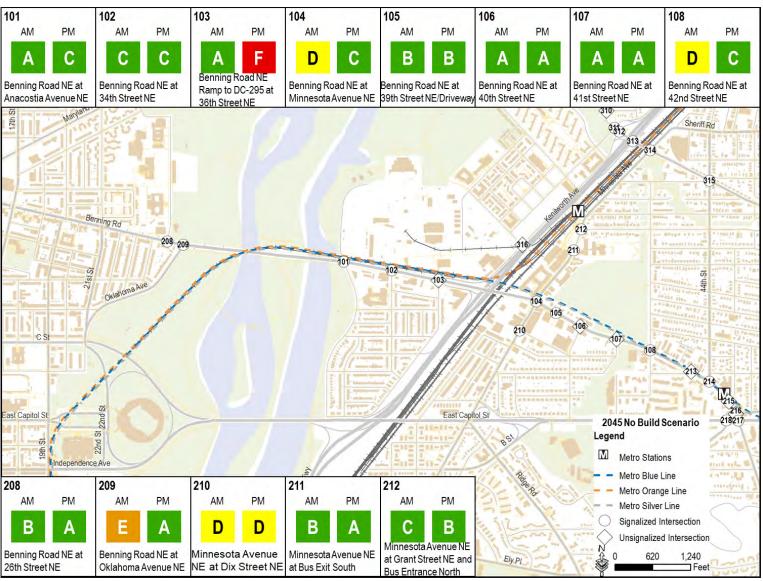
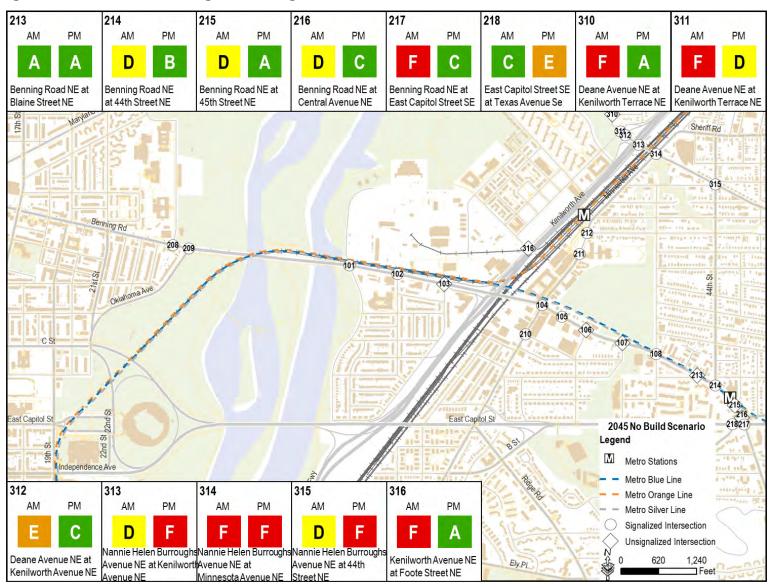


Figure 28: 2045 No-Build Morning and Evening Peak Hour Intersection Levels of Service (LOS) (Sheet 2 of 2)



# 5.3 Design Year 2045 Build Travel Demand Forecasting

The 2045 build travel demand model combined regional baseline assumptions for 2045 with the proposed action.

#### 5.3.1 2045 Build Land Use

For the build scenario, the regional land use of the travel demand analysis area was the same as for the No-Build scenario, using the MWCOG Round 9.1 Cooperative Land Use Forecasts for 2045.

### 5.3.2 2045 Build Transportation Network

The transportation networks in the build scenario are the same as for the No-Build scenario except for the addition of the proposed action.

### 5.3.3 2045 Build Ridership Forecasting

**Table 33** summarizes the projected streetcar boardings at each stop under the No-Build and the build scenarios in 2045. Same techniques as described in section 4.3.3 was used to forecast the 2045 ridership.

Table 33: 2045 No-Build and Build Weekday Streetcar Ridership

		2045	
Stop	No-Build	Build	Change
Union Station	2,210	2,840	+630
3 <sup>rd</sup> Street and H Street NE	380	1,190	+810
5th Street and H Street NE	270	390	+120
8 <sup>th</sup> Street and H Street NE	470	640	+170
13 <sup>th</sup> Street and H Street NE	690	800	+110
15th Street and Benning Road NE	680	1,110	+430
19th Street and Benning Road NE	930	1,410	+480
Oklahoma Avenue and Benning Road NE	1,340	1,430	+90
Kingman Island and Benning Road NE	-	20	+20
34th Street and Benning Road NE	-	770	+770
39th Street and Benning Road NE	-	1,010	+1,010
42 <sup>nd</sup> Street and Benning Road NE	-	750	+750
Benning Road Metrorail Station	-	2,390	+2,390
Total	6,970	14,750	+7,780
Existing Stops	6,970	9,810	+2,840
Proposed Stops	-	4,940	+4,940

By 2045, estimated weekday boardings at proposed stops along the Benning Road Extension would be approximately 4,900 riders. This is in addition to a projected 9,800 riders boarding at

stops along the existing line, 29% of which is projected to be generated by the addition of the extension.

**Table 34** provides the weekday bus ridership under the No-Build and build scenarios for 2045 for key routes or lines that serve the study area.

Table 34: 2045 No-Build and Build Route Level Daily Bus Ridership

	2045						
Stop	No- Build						
		Build	Change				
X Line (X1, X2, X3, X9)	21,630	19,580	-2,050				
U4	2,160	2,020	-140				
U5, U6	3,390	3,200	-190				
U8	3,430	2,880	-550				
96, 97	9,320	9,040	-280				
W4	13,330	13,430	+100				
Total	53,260	50,150	-3,110				
Percent Difference Compared to No-Build	-	-5.8%					

Like the results of opening year analysis, the introduction of the streetcar reduces passenger waiting time and provides faster services. By the year of 2045, the total daily weekday bus ridership in the study area was projected to decrease from 23,000 to 20,800 (nine percent decrease).

## 5.4 Design Year 2045 Build VISSIM Simulation Modeling

The build scenario for design year 2045 assumes the same transportation network as in the 2025 build scenario (Section 4.3). The changes compared to the 2045 No-Build model are summarized below:

- Proposed lane configuration changes at Benning Road and Minnesota Avenue intersection
   (Figure 12)
- Extension of the streetcar to Benning Road Metrorail Station

The same transitions and special transit-only signals described for the 2025 scenario (**Section 4.3**) are considered in the 2045 traffic analysis.

## 5.4.1 2045 Build Traffic Volume Development

Same forecasting methods were used to develop the 2045 build scenario. (Figure 23,

Figure 24, and

Figure 25).

## 5.4.2 2045 Build Conditions

Figure 26,

Figure 33,

Figure 34, and

Figure **29** provide 2045 build intersection LOS at the project area intersections during the morning and evening peak hours with the curbside and median alignment, respectively. Note that, like the 2025 traffic analysis, the traffic control type at Benning Road and 45<sup>th</sup> Street intersection was changed from unsignalized to signalized control to accommodate the transition of the streetcar at Benning Road Metrorail Station.

Overall, the impact of Streetcar is similar as of 2025. In the morning peak, travel times were comparable between the No-Build and Streetcar Build scenario for both directions of Benning Road NE and DC-295.

In the meantime, the intersection of Benning Road NE and Oklahoma Avenue NE operated with more delay under the 2045 No-Build scenario than the 2045 Streetcar Build scenario. This difference was due to the changes in volume on westbound Benning Road NE. Under the Streetcar Build scenario, the westbound approach demand is 200 vehicles fewer than in the No-Build, primarily due to the proposed lane reduction on Benning Road. The intersections of Benning Road NE with 44th Street NE and 45th Street NE operated with more delay under the Streetcar Build scenario than the No-Build scenario. This difference may primarily be attributed to the added signal for the EOL. The streetcar experienced nearly 3 minutes of delay at this intersection, and when present, blocked the leftmost eastbound through lane. Congestion and significant weaving patterns were observed west of the Benning Road NE and DC-295 Interchange under the Streetcar Build scenario, where a lane reduction begins in the westbound direction. The impact of this weaving area was amplified by a bus stop on the rightmost lane just west of the Kenilworth Avenue NE and Benning Road NE merge. The intersection of Benning Road NE and East Capitol Street operated with more delay under the Streetcar Build scenario than the No-Build scenario. The increase in delay occurred on the approaches of southbound East Capitol Street (true westbound direction) and westbound Benning Road NE, and was attributable to the slight increases in demand heading to southbound East Capitol Street and the blockage of traffic by the permissive southbound left turn to eastbound Benning Road NE.

In the evening peak, at the intersections of Benning Road NE with Anacostia Avenue NE and 34<sup>th</sup> Street NE, the eastbound approach delay increased slightly from No-Build to Streetcar Build due to queue spillback from the on-ramp from eastbound Benning Road NE to northbound DC-295 and the lane reduction on Benning Road NE. Traffic operations at the intersection of Benning Road NE and 36<sup>th</sup> Street NE were worse under the No-Build than the Streetcar Build scenario due to severe queue spillback from the eastbound Benning Road NE on-ramp to northbound DC-295. The intersection of Minnesota Avenue NE and Dix Street NE operated at LOS D and LOS F on the westbound approach. At the intersection of Benning Road NE and Minnesota Avenue NE, intersection LOS is C under both the 2045 No-Build and Streetcar Build scenarios. The proposed design and signal timing at this intersection is effective in processing forecasted demand, and intersection delay only increases by seven seconds under the Streetcar Build scenario.

Table 35 and Table 37 provide delay and LOS by movement for the critical intersections for the 2045 Build curbside and median alignments respectively. Table 36 and

Table 38 provide queueing results for the curbside and median alignments. Attachment C provides detailed operational results for all project area intersections for both the curbside and median alignments.

Table 35: 2045 Build Curbside Alignment Peak Hour (AM and PM) Delay and Levels of Service (LOS) at the Critical Intersections

Intersection	Traffic	Peak	Intersection		Northbound		Southbound		Westbound		Eastbound	
Intersection	Control	Hour	Delay	LOS	Delay	LOS	Delay	LOS	Delay	LOS	Delay	LOS
Benning Road and Minnesota Avenue	Signalized	AM	73	E	63	Е	105	F	70	E	51	D
Benning Road and East Capitol Street	Signalized	AM	189	F	256	F	75	Е	198	F	287	F
Benning Road and East Capitol Street	Signalized	PM	214	F	384	F	54	D	424	F	158	F
Minnesota Avenue and NHB Avenue	Signalized	PM	66	Е	68	Е	32	С	27	С	93	F

<sup>\*</sup> The curbside results are based on the original AECOM study.

Table 36: 2045 Build Curbside Alignment Peak Hour (AM and PM) Maximum Queue Length (feet) by Movement at the Critical Intersections

Intersection Peak		Northbound			Southbound			Westbound			Eastbound		
intersection	Hour	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Benning Road and Minnesota Avenue	AM	545	265	265	1345	1345	1345	-	745	745	480	325	325
Benning Road and East Capitol Street	AM	1,075	1,075	1,075	350	350	360	1,675	1,675	1,675	1,465	1,465	1,465
Benning Road and East Capitol Street	PM	1,070	1,070	1,070	365	365	375	1,675	1,675	1,675	1,465	1,465	1,465
Minnesota Avenue and NHB Avenue	PM	810	810	810	1	190	210	205	205	225	950	950	950

<sup>\*</sup> The curbside results are based on the original AECOM study.

Table 37: 2045 Build Median Alignment Peak Hour (AM and PM) Delay and Levels of Service (LOS) at the Critical Intersections

Intersection	Traffic Control	Peak	Interse	ction	Northbound		Southbound		Westbound		Eastbound	
Intersection	Traffic Control	Hour	Delay	LOS	Delay	LOS	Delay	LOS	Delay	LOS	Delay	LOS
Benning Road NE	Unsignalized	PM	53	F	19	C					64	F
Ramp to DC-295 at												
36th Street NE												
Benning Road NE	Unsignalized	AM	36	E			41	E	6	Α	83	F
at Central Avenue												
NE												
Benning Road NE	Signalized	AM	119	F	19	В	209	F	110	F	19	В
at East Capitol												
Street SE												
East Capitol Street	Signalized	PM	74	E	87	F			6	Α	111	F
SE at Texas Avenue												
SE												
Deane Avenue NE	Unsignalized	AM	382	F			515	F	1	Α	297	F
at Lee Street NE												
Deane Avenue NE	Signalized	AM	195	F	297	F	281	F	5	A	59	E
at Kenilworth												
Terrace NE												

Intersection Traffic Control		Peak	Interse	ction	Northb	ound	Southb	ound	Westbound		Eastbound	
Intersection	Traffic Control	Hour	Delay	LOS	Delay	LOS	Delay	LOS	Delay	LOS	Delay	LOS
Deane Avenue NE	Signalized	AM	61	E			294	F	69	E	8	A
at Kenilworth												
Avenue NE												
Nannie Helen	Signalized	PM	125	F	287	F			27	С	35	D
Burroughs Avenue												
NE at Kenilworth												
Avenue NE and												
DC- 295 U-turns												
Nannie Helen	Signalized	AM	171	F	355	F	458	F	60	E	31	C
Burroughs Avenue												
NE at Minnesota												
Avenue NE												
Nannie Helen	Signalized	PM	84	F	52	D	112	F	216	F	29	C
Burroughs Avenue												
NE at Minnesota												
Avenue NE												
Nannie Helen	Signalized	AM	56	E	66	E	32	C	105	F	16	В
Burroughs Avenue												
NE at 44th Street NE												
and Hunt Place NE												
Kenilworth Avenue	Unsignalized	AM	191	F			225	F			15	В
NE at Foote Street												
NE												

Table 38: 2045 Build Median Alignment Peak Hour (AM and PM) Maximum Queue Length (feet) by Approach

Intersection	Peak Hour	Northbound	Southbound	Westbound	Eastbound
Benning Road NE at East Capitol Street SE	AM	862	913	1784	320
Benning Road NE at East Capitol Street SE	PM	271	459	220	1334
East Capitol Street SE at Texas Avenue SE	AM	541	79	-	-
East Capitol Street SE at Texas Avenue SE	PM	232	238	-	-
Benning Road NE and Minnesota Avenue NE	AM	403	549	416	304
Benning Road NE and Minnesota Avenue NE	PM	482	482	330	777

Figure 29: 2045 Build Morning and Evening Peak Hour Freeway Traffic Volumes

\* Numbers on Figures denote: Forecasted Volumes [% Difference from Scenario 2 (2045 No-Build)] BENNING ROAD NE EAST CAPITOL STISE KENILWORTH AVE NE DEANE AVE NE / KENILWORTH AVE NE BAKER STINE / KENILWORTH AVE NE KENILWORTH AVE N SOUTHBOUND DC-295 455 (0.0%) 3,265 [1.9%] 1,250 [2.7%] 3,000 [0.0%] 3,290 [0.0%] 2.690 (0.0%) 3,365 (3,6%) 3.050 [4.0%] 3,145 (0.0%) 430 [0.0%] 485 [0.0%] 365 [0.0%] 3,260 [0.0%] 2.990 10.0%1 3,475 [0.0%] 2,795 [0.0%] 3,330 (0.0%) NORTHBOUND DC-295 700 [0.0%] 420 [0.0%] AM Peak Hour DEANE AVE NE / NANNIE HELEN BURROUGHS AVE NE

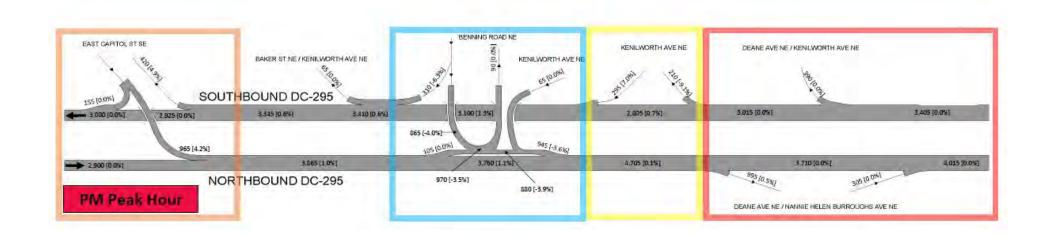


Figure 30: 2045 Build Morning and Evening Peak Hour Intersection Traffic Volumes (Sheet 1 of 2)

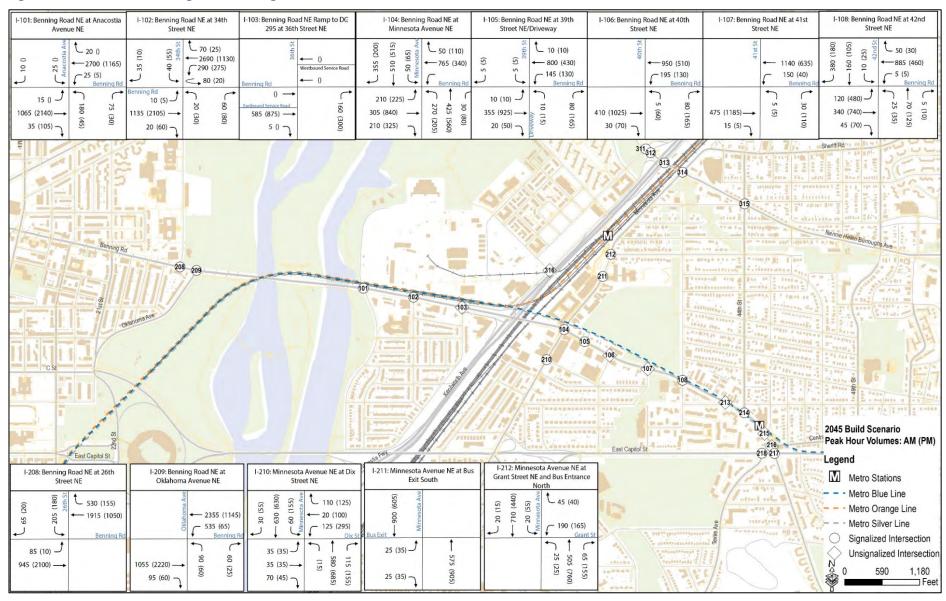
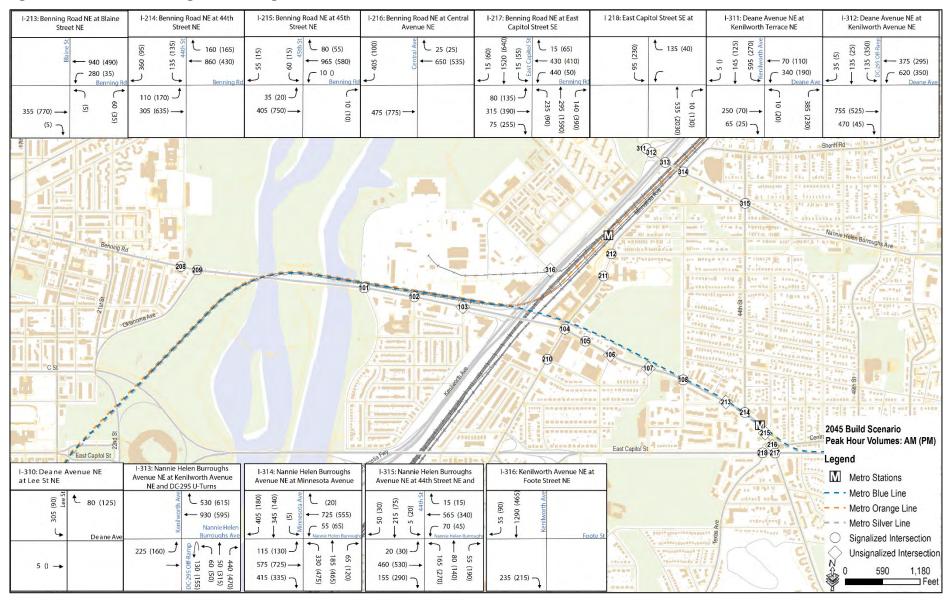
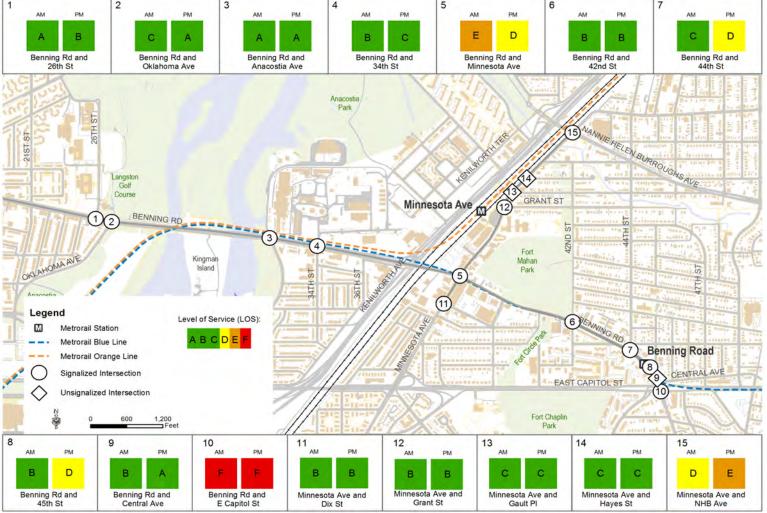


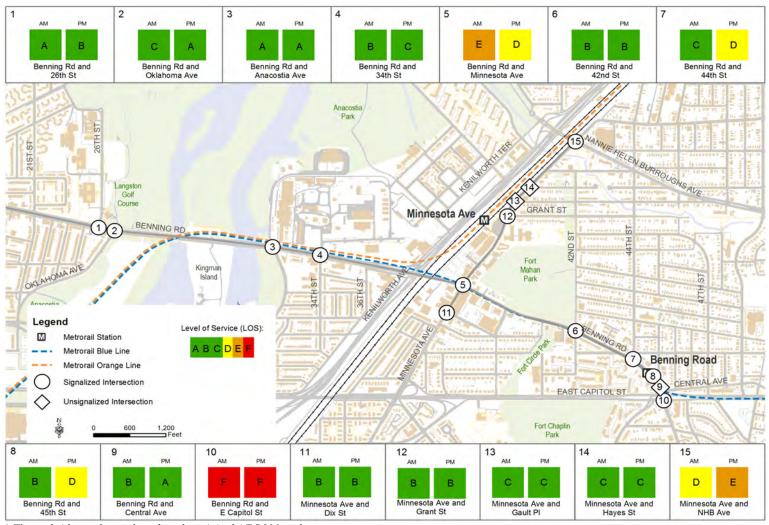
Figure 31: 2045 Build Morning and Evening Peak Hour Intersection Traffic Volumes (Sheet 2 of 2)





<sup>\*</sup> The curbside results are based on the original AECOM study.

Figure 33: 2045 Build Curbside Alignment Morning and Evening Peak Hour Intersection Levels of Service (LOS) (Sheet 2 of 2)



<sup>\*</sup> The curbside results are based on the original AECOM study.

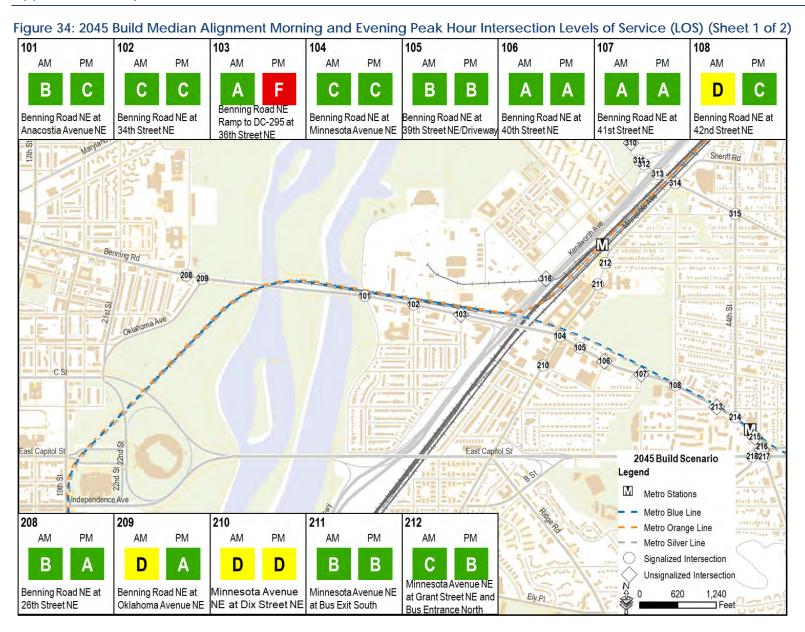
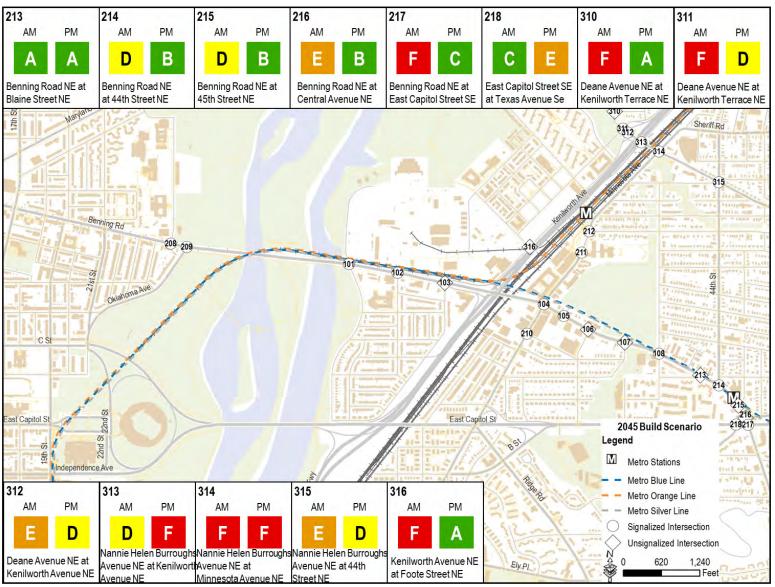


Figure 35: 2045 Build Median Alignment Morning and Evening Peak Hour Intersection Levels of Service (LOS) (Sheet 2 of 2)



#### 5.4.3 2045 Build Streetcar Operations

Streetcar operation was evaluated based on the average travel speeds which were obtained from the VISSIM simulation model. VISSIM travel time segments were defined from 20<sup>th</sup> Street to Benning Road Metrorail Station in the eastbound direction and from Benning Road Metrorail Station to 26<sup>th</sup> Street in the westbound direction. Travel time segments for median alignments are defined from 26<sup>th</sup> Street to Benning Road Metrorail Station in the eastbound direction and from Benning Road Metrorail Station to 26<sup>th</sup> Street in the westbound direction. **Table 39** provides average travel time and speed for the curbside and median streetcar alignments in 2045 during the morning and evening peak hours.

Table 39: 2045 Average Travel Time and Speed for the Streetcar Alignments in the Morning and Evening Peak Hour

Direction	Peak Hour	Travel Time (min)	Speed (mph)			
	Curbside St	reetcar Alignment				
Eastbound	A N 4	10.3	12.5			
Westbound (peak direction)	AM	11.5	9.7			
Eastbound (peak direction)	PM	13.2	9.7			
Westbound	PIVI	9.8	11.4			
	Median Str	eetcar Alignment				
Eastbound	AM	8.1	13.8			
Westbound (peak direction)	Alvi	7.3	15.3			
Eastbound (peak direction)	PM	8.5	13.1			
Westbound	L IAI	7.1	15.7			

<sup>\*</sup> The curbside results are based on the original AECOM study.

Similar findings, as reported in the 2025 Build section, were obtained from the analysis. Travel times (excluding dwell time at the streetcar stops) for the streetcar vehicles were slightly higher than those for vehicular traffic in both directions of Benning Road NE. The biggest difference existed along the segment from 42nd Street NE to 45th Street NE due to long delays and eastbound queue impacts at the streetcar EOL. The curbside alignment operates with relatively slower speeds compared to the median alignment during the both peak hours due to the higher number of transitions for the curbside alignment. Moreover, results suggest that with the increase in background traffic in 2040, streetcar travel times would generally increase with the exception of eastbound travel times during the evening peak hour, where this can be attributed to the 2045 MWCOG projections. MWCOG model projected lower traffic volumes in 2045 on East Capitol Street compared to 2025 projections due to the through lane reduction on East Capitol Street. As a result, more green time could be allocated to Benning Road approach at Benning Road and East Capitol Street intersection in 2045 (due to lower East Capitol Street volumes), limiting congestion and queue spillback on Benning Road approach, thereby improving travel times.

**Table** 40 and **Table** 41 show stop to stop travel times for streetcar alternatives in 2045 based on the VISSIM output. Vehicle travel times obtained from VISSIM along Benning Road for the same travel segments were also included in **Table** 42 and **Table** 43 or comparison purposes.

Table 40: 2045 Station to Station VISSIM Travel Time Results for Curbside Streetcar Alignment

	Curbsid	e Streetcar				
Segment	Travel Time (min) – AM Peak Hour	Travel Time (min) – PM Peak Hour				
East	bound Direction					
20 <sup>th</sup> Street to Oklahoma Avenue	0.7	0.8				
Oklahoma Avenue to Kingman Island	2.0	2.1				
Kingman Island to 34th Street	1.0	1.3				
34th Street to 39th Street	3.3	3.8				
39th Street to 42nd Street	1.0	1.2				
42 <sup>nd</sup> Street to Benning Road Metrorail Station	2.2	3.9				
TOTAL	10.3	13.2				
West	bound Direction					
Benning Road Metrorail Station to 42 <sup>nd</sup> Street	2.1	1.7				
42 <sup>nd</sup> Street to 39 <sup>th</sup> Street	2.3	1.3				
39th Street to 34th Street	2.4	3.0				
34th Street to Kingman Island	1.4	1.2				
Kingman Island to Oklahoma Avenue	2.8	2.2				
Oklahoma Avenue to 26th Street	0.5	0.4				
TOTAL	11.5	9.8				

<sup>\*</sup> The curbside results are based on the original AECOM study.

Table 41: 2045 Station to Station VISSIM Travel Time Results for Median Streetcar Alignment

	Median Al	ignment
Coomant	Travel Time (min) – AM 🔠	ravel Time (min) – PM Peak
Segment	Peak Hour	Hour
Eastbou	nd Direction	
20th Street to Oklahoma Avenue	0.1	0.1
Oklahoma Avenue to Langston Golf Course	1.2	1.1
Langston Golf Course to 34th Street	1.7	2.0
34th Street to 39th Street	1.6	2.1
39th Street to 42nd Street	0.9	1.1
42 <sup>nd</sup> Street to Benning Road Metrorail Station	2.7	2.3
TOTAL	8.1	8.5
Westboo	and Direction	
Benning Road Metrorail Station to 42nd Street	1.4	1.2
42 <sup>nd</sup> Street to 39 <sup>th</sup> Street	0.9	1.0
39th Street to 34th Street	2.2	2.5
34th Street to Langston Golf Course	1.3	1.1
Langston Golf Course to Oklahoma Avenue	1.1	1.0
Oklahoma Avenue to 20th Street	0.5	0.4
TOTAL	7.3	7.1

Table 42: 2045 Corridor Vehicle Travel Times under Curbside Alignment

	Curbside Streetcar										
Segment	Travel Time (min) – AM Peak Hour	Travel Time (min) – PM Peak Hour									
Eastbound Direction											
20th Street to Benning Road Metrorail Station	6.8	9.3									
Westbound Direction											
Benning Road Metrorail Station to 26th Street	6.4	5.3									

<sup>\*</sup> The curbside results are based on the original AECOM study.

Table 43: 2045 Corridor Vehicle Travel Times under Median Alignment

	Median Streetcar										
Segment	Travel Time (min) – AM Peak Hour	Travel Time (min) – PM Peak Hour									
Eastbound Direction											
26th Street to Benning Road Metrorail Station	5.3	6.6									
Westbound Direction											
Benning Road Metrorail Station to 26th Street	6.2	5.6									

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# 6.0 Attachment A - 2019 Existing Model Calibration

# 6.1.1.1 Volume Calibration (Freeways)

#### 6.1.1.1.1 AM Peak Hour (7:45 AM - 8:45 AM)

		Subtotal	Total	Percent	Target	Target Met
Segments (n = 29)	Within ± 100 vph for < 700 vph	14				
	Within ± 15% for ≥ 700 vph to < 2,700 vph	3	29	100%	85%	Yes
	Within ± 400 vph for ≥ 2,700 vph	12				

\* MOEs of Throughput, Speed, and Density represent an average of 10 simulation runs.

Facility	Segment	Type	Balanced Count (vph)	VISSIM Throughput (vph)	Difference (vph)	Difference (%)	Average Speed (mph)	Average Density (vpmpl)
	Mainline south of EB Capitol Street On-Ramp	Basic	2,875	2,874	-1	0%	49	29.5
	Between EB Capitol Street On-Ramp and WB Benning Road Off- Ramp	Merge/Diverge	3,180	3,178	-2	0%	51	20.5
NB DC-295	Between WB Benning Road Off-Ramp and EB Benning Road/Kenilworth Avenue On-Ramp	Basic	2,910	2,905	-5	0%	51	18.9
NB DC-293	Between EB Benning Road/Kenilworth Avenue On-Ramp and Nannie Helen Burroughs Avenue Off-Ramp	Merge/Diverge	3,240	3,224	-16	0%	50	20.2
	Between Nannie Helen Burroughs Avenue Off-Ramp and Kenilworth Avenue On-Ramp	Basic	2,795	2,792	-3	0%	52	18.0
	Mainline north of Nannie Helen Burroughs Avenue	Merge/Basic	3,330	3,328	-2	0%	43	21.0
	Mainline North of Deane Avenue Off-Ramp	Merge/Diverge	3,145	3,120	-25	-1%	8	114.9
	Between Deane Avenue Off-Ramp and Kenilworth Avenue Off-Ramp	Basic/Diverge	2,900	2,876	-24	-1%	7	133.1
	Between Kenilworth Avenue Off-Ramp and Kenilworth Avenue On-Ramp	Basic	2,190	2,150	-40	-2%	4	165.4
CD DC 205	Between Kenilworth Avenue On-Ramp and EB Benning Road On-Ramp	Merge/Basic	2,840	2,782	-58	-2%	6	143.2
SB DC-295	Between EB Benning Road On-Ramp and Baker Street Off-Ramp	Weave	3,120	3,026	-94	-3%	8	100.8
	Between Baker Street Off-Ramp and WB Capitol Street Off-Ramp	Diverge	3,095	3,013	-82	-3%	11	90.5
	Between WB Capitol Street Off-Ramp and EB Capitol Street On- Ramp	Basic	2,600	2,535	-65	-3%	12	102.7
	Mainline south of EB Capitol Street On-Ramp	Merge/Basic	2,990	2,909	-81	-3%	15	89.9

# 6.1.1.2 AM Calibration Summary

Interchange	Ramp		Balanced Count (vph)	VISSIM Throughput (vph)	Difference (vph)	Difference (%)	Average Speed (mph)	Average Density (vpmpl)
	EB Capitol Street to SB DC-295	Ramp	390	390	0	0%	26	15.7
Capitol Street	EB Capitol Street to NB DC-295	Ramp	305	305	-1	0%	33	9.6
	SB DC-295 to WB Capitol Street	Ramp	495	489	-6	-1%	40	12.6
Baker Street	SB DC-295 to WB Baker Street	Ramp	25	21	-4	-15%	20	1.1
	EB Benning Road to SB DC-295	Ramp	280	275	-5	-2%	36	7.6
	EB Benning Road to NB DC-295 or WB Benning Road (U-turns)	Ramp	295	287	-8	-3%	24	22.7
Danning Dand	NB DC-295 to WB Benning Road	Ramp	270	274	4	1%	27	23.9
Benning Road	NB DC-295/EB Benning Road (U-turns) to WB Benning Road	Ramp	280	280	0	0%	2	136.9
	SB Kenilworth Avenue NE to NB DC-295	Ramp	45	44	-1	-2%	23	1.9
	EB Benning Road/SB Kenilworth Avenue NE to NB DC-295	Ramp	330	320	-11	-3%	39	8.2
	SB Kenilworth Avenue to SB DC-295 (South)	Ramp	650	640	-10	-1%	5	123.1
Kenilworth Avenue	SB DC-295 to SB Kenilworth Avenue	Ramp	710	723	13	2%	18	47.8
	NB Kenilworth Avenue to NB DC-295	Ramp	535	537	2	0%	23	22.6
Nannie Helen	NB DC-295 to Nannie Helen Burroughs Avenue/Kenilworth Avenue	Ramp	445	432	-13	-3%	47	9.1
Burroughs Avenue	SB DC-295 to Deane Avenue/Kenilworth Terrace/Kenilworth Avenue	Ramp	245	232	-13	-5%	39	6.1

# 6.1.1.3 Volume Calibration (Intersections)

# 6.1.1.3.1 AM Peak Hour (7:45 AM - 8:45 AM)

	Volume Criteria	Subtotal	Total	Percent	Target	Target Met
	Within ± 100 vph for < 700 vph	61			85%	
Approaches (n = 77)	Within ± 15% for ≥ 700 vph to < 2,700 vph	24	86	99%		Yes
	Within ± 400 vph for ≥ 2,700 vph	1				



Approach	es:
LOS	Count
A-D	68
E	10
F	10

			g	5															
#	Intersection	Approach	Movement	(v	ed Count ph)	Through	SIM put (vph)		ice (vph)	Differer	ice (%)	(sec	e Delay (veh)	Ler (fe	e Queue ngth eet)	(fe	ue Length eet)		os*
		NB	RT	25	89	24	90	-1	1	-3%	1%	6.6	39.4	15	19	145	149	D A	D
			LT	19		20		2		9%		48.9		7		68		D	
		SB	RT	9	28	8	28	-1	1	-10%	3%	50.2	49.3	7	7	68	68	D	D
	Benning Road		LT	10		10		0		0%		33.1		6		139		С	
101	NE at Anacostia	EB	TH	918	953	907	941	-11	-11	-1%	-1%	2.6	3.0	6	6	139	139	Α	Α
	Avenue NE		RT	25		25		-1		-2%		4.7		0		40		Α	
			LT TH	21		23		2		12%		13.5		16		354		В	
		WB	RT	2,546 13	2,580	2,655 11	2,690	109 -2	110	4% -16%	4%	2.5 8.1	2.6	16 10	16	354 376	376	A .	Α
		Intersection			649		749		00	-10%	/o	3	9	10		3/6		Α	A
			LT	18		19		1		4%		38.7		7		106		D	
		NB	RT	52 70	56	76	4	6	8%	8%	11.3	18.3	6	7	115	115	В	В	
		SB	LT RT	30 40	70	28 42	70	-2 2	0	-5% 4%	0%	38.7 36.5	37.4	15 15	15	133 133	133	D	D
	Benning Road NE at 34th Street NE		LT	8		10		2		20%		17.5		0	_	29		В	
		EB	TH	933	962	918 949	-15	-15 -13	-2%	-1%	11.4	11.4	23	29	190	213	В	В	
102			RT	21		21		0		1%		8.9		29		213		Α	
			LT TH	219 2,523	-	224 2,622		5 99		2% 4%		31.3 14.8		58 102		488 586		C B	
		WB	RT	49	2,791	45	2,949	-4	158	-8%	6%	17.4	16.4	105	105	593	593	В	В
			U	76		59		-17		-23%		33.1		58		488		С	
		Intersection	1	3,893		3,985		92		2%		15.9		•					В
	Benning Road	NB	RT	171	171	169	169	-2	-2	-1%	-1%	2.2	2.2	0	0	44	44	Α	Α
	NE Ramp to DC-		TH	404		395		-8		-2%		1.6		2		138		Α	
103	295 at 36th Street	EB	RT	6	410		5 400		-1 -1		-15%		1.6	1	2	119	138	Α	Α
	NE -	Intersection			81		70		11	-15%		1.2	7			110			A
		intersection	LT	307	01	302	70	-5		-2%	70	35.1	· <i>'</i>	83	ı	454		D	
		NB	TH	296	637	308	645	12	7	4%	1%	27.8	30.9	83	83	454	454	C	С
			RT	34		35		1		3%		23.2		83		454		С	
			LT TH	50		61		11		21%		48.2		15		243		ט	_
	Benning Road	SB	RT	323 137	510	334 152	547	11 15	37	3% 11%	7%	43.8 32.6	41.2	115 115	115	537 537	537	C	D
104	NE at Minnesota		LT	129		133		4		3%		123.8		177		554		F	
	Avenue NE	EB	TH	306	681	279	643	-27	-38	-9%	-6%	56.6	68.3	177	177	554	554	Е	E
	<u> </u>		RT	245		231		-15		-6%		50.5		177		554		D	
		WB	LT TH	16 629	692	11 706	777	-5 76	84	-32% 12%	12%	86.7 71.6	72.6	221 221	221	411	411	E	Е
		***	RT	47	092	60	,,,	13	04	28%	1278	81.4	72.0	221	221	411	411	F	
		Intersection	1	2,	521	2,0	611	9	90	49	6	54	1.7						D

#	Intersection	Approach	Movement		ed Count ph)		SIM put (vph)	Differer	nce (vph)	Differe	nce (%)		ge Delay :/veh)	Ler	e Queue ngth eet)		eue Length eet)	LC	os*
		NB	RT	10	20	9	20	-1	0	-13%	-2%	18.6	52.8	3	5	76	76	F B	D
		SB	LT RT	5	10	5	8	0 -5	-2	-2% -100%	-20%	55.0	44.0	1 0	1	29 0	29	E	D
	Benning Road		LT	10		11		0		3%		2.6		10		175		A	
105	NE at 39th Street NE/Driveway	EB	TH RT	359 21	390	340 23	374	-19 3	-16	-5% 13%	-4%	5.1 4.9	5.0	10 10	10	175 175	175	A	Α
	NED THE WAY	WB	LT TH	20 678	708	17 762	782	-3 84	74	-14% 12%	10%	64.8 61.1	61.1	251 251	251	756 756	756	E	E
			RT	10		3	_	-7		-74%		38.2		251	251	756	756	D	
		Intersection		1,	127		183		56	5'	%		3.1	_					D
		NB	LT RT	114	121	6 112	118	-1 -2	-4	-15% -2%	-3%	33.2 8.9	10.2	7	7	114 115	115	D A	В
106	Benning Road NE at 40th Street	EB	TH RT	354 19	373	334 18	352	-21 -1	-21	-6% -4%	-6%	0.7 0.8	0.7	0 3	3	127 124	127	A A	А
100	NE NE	WB	LT TH	311 699	1,010	311 775	1,086	0 75	75	0% 11%	7%	19.5 24.3	22.9	94 94	94	423 423	423	C	С
		Intersection			505		555		51	3'	%		6.9	34		720		_	В
		1	LT	7		6	1	-1	1	-10%		16.5		1		49	F.	С	
	Benning Road NE at 41st Street NE	NB	RT TH	24 459	31	21 436	27	-3 -24	-4	-12% -5%	-12%	7.0 0.4	9.1	1 0	1	51 39	51	A	А
107		EB	RT	9	468	9	444	0	-24	-2%	-5%	0.8	0.4	0	0	15	39	A	Α
		WB	LT TH	99 1,003	1,102	91 1,084	1,175	-9 81	72	-9% 8%	7%	9.5 9.2	9.2	39 39	39	291 291	291	A	А
		Intersection	1	1,0	601	1,6			45	3%		6.8				, <u>, , , , , , , , , , , , , , , , , , </u>		,	A
		NB	LT	26		17		-10		-38%		23.1		7		98		С	
			TH RT	57 4	87	47 20	84	-10 16	-4	-18% 410%	-4%	20.9 9.7	18.6	7 6	7	98 112	112	C A	В
		SB	LT	12		9		-3		-22%		56.2		148		522		E	
			TH	136	466	134	448	-2	-18	-1% -4%	-4%	54.4	49.1	148	159	522	539	D	D
108	Benning Road		RT LT	318 106		304 108		-14 2		-4% 2%		46.6 73.4		159 56		539 279		D E	
108	NE at 42nd Street NE	EB	TH	331	483	311	459		-25	-6% -14%	15.2 28.4		68	279	304	В	С		
	NL		RT	46	.00	40	.00	-6			9.7		68		304	1 00.	Α	, i	
			LT	7		6		-1		-10%		16.8		38		329		В	
		WB	TH RT	758 31	796	852 32	890	94	94	12% 3%	12%	12.9 12.8	12.9	38 39	39	329 348	348	B B	В
		Intersection			833		380		47	3/6	%		5.6	39		340		В	c .
		SB	LT	170	225	162	214	-8	-11	-5%	-5%	51.9	48.8	66	72	392	407	D	D
	Benning Road		RT LT	55 65		52 67		-3 2		-5% 3%	0,0	39.0 36.9	10.0	72 6		407 114		D	
208	NE at 26 <sup>th</sup> Street	EB	TH	803	868	813	880	10	12	1%	1%	6.5	8.8	10	10	149	149	A	Α
	NE	WB	TH RT	1,784 419	2,204	1,896 409	2,304	111 -11	101	6% -3%	5%	4.7 10.5	5.7	64 68	68	295 320	320	A B	А
		Intersection			297		399		02	3'	%		9.3				•		A
		NB	LT RT	68 52	120	67 51	118	-1 -1	-2	-1% -3%	-2%	56.3 3.9	33.7	21	21	147 45	147	E	С
	Benning Road	EB	TH	900	973	906	977	6	4	1%	0%	7.5	7.7	29	29	240	254	A	А
209	NE at Oklahoma Avenue NE		RT LT	73 483		71 490		-2 7		-3% 1%		9.8 49.3		27 128		254 704	-	A D	
		WB	TH	2,136	2,619	2,236	2,726	100	107	5%	4%	16.6	22.4	32	128	479	704	В	С
		Intersection	l .	3,	712	3,8	321	1	80	3'	%	1	9.0						В

#	Intersection	Approach	Movement		ed Count ph)		SIM put (vph)	Differer	nce (vph)	Differer	nce (%)		e Delay /veh)	Ler	e Queue ngth eet)		ue Length eet)	LOS*
			LI	5		5		0		-2%		9.9		18		192		A
		NB	TH	586	677	594	687	7	10	1%	2%	8.7	8.5	18	18	192	239	A
			RT	86		89		3		3%		7.2		14		239		· · · · · · · · · · · · · · · · · · ·
			LT TH	41 528		34		-7		-17% 0%		14.7 7.6		15		215 215		<u>В</u>
	Minnesota	SB	RT	15	584	527 14	574	-2 -1	-10	-9%	-2%	5.8	8.0	15 18	18	238	238	A A
040			LT	4		3		-1 -1		-9% -17%		45.0		2		41		D
210	Avenue NE at Dix	EB	TH	3	10	4	10	1	0	23%	-2%	37.2	40.7	2	2	41	41	D D
	Street NE	ЕВ	RT	3	10	3	10	0	U	-7%	-270	40.4	40.7	2	2	41	41	D
	 		LT	93		94		1		1%		48.0		40		237		D
		WB	TH	18	158	16	159	-2	1	-11%	0%	49.7	43.6	40	40	237	243	D D
		***	RT	47	100	49	100	1	'	3%		33.1		40	70	243	240	С
		Intersection	1	1,	430	1,4	431		1	00	%	12	2.4					В
		NB	TH	473	473	500	500	27	27	6%	6%	19.7	19.7	42	42	518	518	в в
	Minnesota	SB	TH	480	480	497	497	16	16	3%	3%	4.2	4.2	8	8	93	93	A A
211	Avenue NE at	EB	LT	19	50	11	57	-8	7	-43%	14%	55.6	28.6	18	18	160	161	E C
	Bus Exit South		RT	31		46		15		50%		22.3		16	10	161	101	С
		Intersection			003		054		51	59	%		2.9					В
			LT	38		16		-22		-58%		16.4		2		71		В
		NB	TH	407	489	428	486	22	-3	5%	-1%	13.1	13.0	36	36	356	356	В В
	Minnesota		RT	44		42		-3		-7%		10.9		36		356		В
	Avenue NE at		LT	/		8		1		10%		23.5		16		127		<u> </u>
212	Grant Street NE	SB	TH	402	430	438	446	36	15	9%	4%	10.6	10.9	16	17	127	154	В В
	and Bus		RT	21		0		-21		-100%		- 45.0		17		154		-
	Entrance North	WB	LT RT	75 34	109	73 36	109	-2 2	0	-3% 7%	0%	15.2 6.6	12.3	6 5	6	111 125	125	B B
	-	Intersection			027		040		12	170	0/_		2.0	5		125		A
			LT	1	021	1	1	0		-20%	70	41.2	2.0	3		63		E
		NB	RT	52	53	52	53	0	0	1%	0%	15.9	16.2	4	4	68	68	C C
	Benning Road		TH	345		336		-9		-3%		36.6		33		150		E
213	NE at Blaine	EB	RT	2	347	3	338	0	-9	17%	-3%	13.3	30.4	44	44	196	196	В
	Street NE	wn	LT	228	4.000	236	4 400	8	-00	3%	400/	8.9	0.0	16	40	279	070	A
		WB	TH	795	1,023	886	1,122	91	99	11%	10%	1.7	3.2	12	16	248	279	A
		Intersection			423	1,	514	0)	90	69	%		1.1					В
		SB	LT	130	378	129	370	-2	-8	-1%	-2%	76.0	65.1	161	165	576	583	E E
		36	RT	248	3/6	241	370	-7	-0	-3%	-2/0	59.2	05.1	165	105	583	363	E
	Benning Road	ЕВ	LT	109	397	101	383	-8	-14	-7%	-3%	121.7	57.1	129	129	357	357	F
214	NE at 44th Street		TH	288		282	000	-6		-2%		34.0	· · · · ·	129		357		C
	NE	WB	TH RT	775 159	934	877 148	1,026	102 -11	91	13% -7%	10%	15.2 11.7	14.7	53 25	53	327 371	371	B B
		Intersection			709		779		69	-7% 4°			1.3	20	<u> </u>	3/1		
		intersection	LT	48	108	42	119	-7	פנ	-14%	/0		ŧ.J	283		537		F
		SB	RT	69	117	52	93	-17	-24	-14%	-20%	406.4 364.4	383.1	286	286	541	541	F F
	Benning Road		LT	19	<del>                                     </del>	19	1	0		-1%		112.0		167		389		F
215	NE at 45th Street	EB	TH	399	418	385	404	-14	-14	-3%	-3%	115.4	115.2	167	167	389	389	F F
213	NE at 45th Street		TH	864		972		108		13%		1.1		323		594		A
	175	WB	RT	62	926	66	1,037	4	112	6%	12%	3.4	1.3	9	323	237	594	A A
	l t	Intersection	1	1,	460	1,	535	7	74	59	%	54	1.4					D
		WD	TH	499	540	538		39	-00	8%		0.5	0.4	0		82	- 00	Α .
	Benning Road	WB	RT	14	513	13	551	-2	38	-11%	7%	0.0	0.4	0	0	57	82	A
216	NE at Central	EB	TH	459	459	425	425	-34	-34	-7%	-7%	104.1	104.1	85	85	197	197	F F
	Avenue NE	SB	RT	437	437	493	493	56	56	13%	13%	12.0	12.0	36	36	393	393	ВВ
		Intersection	1	1,	410	1,4	470	6	60	49	%	34	1.4					D

#	Intersection	Approach	Movement		ed Count ph)		SIM put (vph)	Differen	nce (vph)	Differe	nce (%)		e Delay /veh)	Ler	e Queue igth et)		ue Length eet)	LO	os*
			LT	453		418		-35		-8%		131.9		294		742		H	
		NB	TH	218	693	247	686	29	-6	13%	-1%	58.9	103.1	294	294	742	747	Е	F
			RT	22		21		-1		-3%		50.9		294		747		D	
			LT	97		87		-10		-10%		31.2		0		2		С	
		SB	TH	243	461	223	421	-20	-40	-8%	-9%	27.1	22.2	3	117	62	168	С	С
	Benning Road		RT	121		112		-10		-8%		5.4		117		168		Α	
217	NE at East		LT	262		273		11		4%		47.5		104		231		D	
	Capitol Street SE	EB	TH	409	789	371	765	-38	-24	-9%	-3%	4.4	19.4	104	104	231	244	Α	В
			RT	118		122		4		3%		2.5		37		244		Α	
		WB	TH	1,817	4 054	1,799	4 000	-18	-22	-1%	40/	51.4	54.4	262	000	860	870	D	D
		WB	RT	34	1,851	30	1,829	-4	-22	-11%	-1%	51.0	51.4	269	269	870	870	D	U
		Intersection			793	3,7			92		!%	5	1.0					Î	D
		NB	RT	242	242	137	137	-106	-106	-44%	-44%	658.4	658.4	649	649	701	701	F	F
		EB	TH	542	556	617	630	74	73	14%	13%	47.4		72	72	288	298	D	i i
	East Capitol	LD	RT	14	550	13	000	-1	73	-8%	1370	22.1	40.9	69	12	298	230	С	υ
218	Street SE at	Ļ	U	4		14		10	1	245%		64.1	0.4	39		300		E	
	Texas Avenue SE	WB	LT	117	2,391	111	2,370	-6	-21	-5%	-1%	50.0	3.4	40	40	299	300	D	Α
			TH	2,270	-	2,246		-25		-1%		0.8		9		200		Α	
		Intersection		3	190	3 1	136		54	-2	!%	40	0.7						D
			LT	256	100	239	100	-17	1	-6%	. 70	226.9	J.1	291		704		F	
		SB	RT	2	258	0	239	-2	-19	-100%	-7%	-	∠∠0.∀	291	291	704	704	-	F 1
			LT	2		3		1		35%		295.0		5		35		F	
310	Deane Avenue	EB	TH	3	5	2	4	-1	-1	-48%	-14%	227.7	271.0	5	5	35	35	F	F
0.0	NE at Lee St NE		TH	2		3		1		31%		2.0		0		11		А	
		WB	RT	65	67	68	70	3	3	4%	5%	0.9	0.9	0	0	11	11	Α	Α
		Intersection		3	29	3	13	-	16	-5	i%	17	7.0					F	F
		NB	LT	8	280	9	280	1	0	6%	0%	51.5		35	35	321	321	D	
		ND	RT	272	200	272	200	0	U	0%	0 /0	33.5	34.1	27	33	302	321	С	U
		_	LT	387		394		7		2%		232.4		779		1,507		F	
	Deane Avenue	SB	TH	153	547	158	552	5	5	3%	1%	217.4	228.1	805	805	1,535	1,535	F	F
311	NE at Kenilworth		RT	7		0		-7		-100%		-		0		0		-	
	Terrace NE	EB	TH	193	261	186	250	-8	-12	-4%	-5%	66.1	50.5	171	197	286	316	E	Е
	_		RT LT	68 194		64 188		-4 -6		-6% -3%		36.2 3.7	00.0	197		316 66		D A	
		WB	TH	55	249	61	249	6	0	11%	0%	2.6	3.4	3	3	66	66	A	Α
	-	Intersection			337		330		-7		%		3.4	<u> </u>		00			F
			LT	174	331	137	550	-37	1	-21%	70	284.8		355		550		F	
		SB	TH	50	224	38	175	-13	-49	-25%	-22%	286.1	285.1	355	355	550	550	F	F
	Deane Avenue		TH	645		653		8		1%		10.2		79		174		В	
312	NE at Kenilworth	EB	RT	207	852	199	852	-8	0	-4%	0%	4.9	9.0	85	85	181	181	А	Α
	Avenue NE	14/5	LT	604	0.40	594	0.40	-10		-2%	00/	11.0	40.0	31	0.4	283	000	В	
		WB	TH	238	842	249	843	11	1	5%	0%	17.4	12.9	31	31	283	283	В	В
		Intersection			918		370		49		1%		6.5						D
			U	127		125		-3		-2%		19.6		67		371		В	
	Nannie Helen	NB	LT	20	445	19	432	-1	-13	-5%	-3%	51.0	10.4	121	121	439	439	D	В
	Burroughs	ND	TH	21		22		0		1%		50.7		0		0		D	
	Avenue NE at		RT	276	276	267	267	-9	-9	-3%	-3%	62.6	62.6	29	29	459	459	Е	Е
313	Kenilworth	ЕВ	LT	165	819	161	819	-5	-1	-3%	0%	38.4	(1) m	44	44	276	276	D	
	Avenue NE and	25	TH	654	019	658	019	4	-1	1%	0 /8	17.5	∠1.6	44	***	276	210	В	U
	DC-295 U-turns	WB	TH	822	1,354	825	1,364	3	10	0%	1%	32.4	29.6	186	208	354	380	С	С
	DG-293 U-turns		RT	532		538		6		1%		25.2		208	200	380	300	С	
	1	Intersection		1 2	394	26	314	-2	280	-10	<b>1</b> 0/_	3(	0.3					-	С

# Benning Road and Bridges Transportation Improvements Final Environmental Assessment Appendix E – Transportation Technical Memorandum

#	Intersection	Approach	Movement		ed Count ph)	_	SIM put (vph)	Differen	ice (vph)	Differe	nce (%)	Averag		Lei	e Queue ngth eet)	(fe	ue Length eet)		os*
		NB	TH	78	436	80	447	2	11	3%	3%	24.4	44.3	9	110	91	469	C	D
			RT	44		48		4		8%		4.4		6		95		Α	
	Nannie Helen	SB	TH	166	500	170	504	4		3%		73.9		623	044	979	4.000	Е	
	Burroughs	28	RT	372	538	364	534	-9	-4	-2%	-1%	276.5	211.9	644	644	1,000	1,000	F	F
314	Avenue NE at		LT	70		71		1		1%		42.1		102		341		D	
314		EB	TH	478	928	476	923	-3	-5	-1%	-1%	25.3	22.2	102	108	341	350	С	С
	Minnesota		RT	381		377		-4		-1%		14.7		108		350		В	
	Avenue NE		LT	36		31		-5		-14%		72.0		18		185		Е	
		WB	TH	666	669	678	719	12	50	2%	7%	61.2	61.7	153	154	413	416	E	E
			RT	3		10		7		230%		62.4		154		416		E	
		Intersection			571		523		51	2'	%	75	5.4	0.4	1	000			
			LT TH	132		132		0		0% -1%		67.3 47.7		64	ł	328 328	4 1	E	
		NB		53	227	53	223	0	-4		-2%		56.3	64	64		330	D	E
			RT	42		38		-4		-9%		30.1		64		330		С	
			LT	6		6		-1		-8%		42.9		88		429		D	,
		SB	TH	205	359	203	354	-2	-6	-1%	-2%	39.6	39.0	88	101	429	462	D	υ
		35	SBR - NHB EB	52	333	50	334	-3	-0	-5%	-2/0	32.5	39.0	101	101	462	402	С	U
	Nannie Helen		SBR - Hunt	96		96		0		0%		40.7		88		429		D	
	Burroughs		LT	15		14		-1		-5%		16.7		24		271		В	
	Avenue NE at	EB	TH	364	526	378	525	13	-1	4%	0%	10.2	10.9	24	24	271	271	В	В
315	44th Street NE	25	EBR - Hunt Street	4	020	4	020	0		-3%	0 /0	16.9	10.5	24		271		В	, ,
	and Hunt Place		EBR - 44th Street NBL- NHB EB	142		129		-13		-9% -8%		12.2		24		271		В	
	NE		NBT - 44th SB	30 11		28 10		-2 -1		-8%		44.9 46.0		12 12		87 87		D	_ '
		Hunt Street	NBR - NHB WB	9	50	9	47	0	-3	-3%	-6%	42.8	45.0	12	12	87	87	D	D
			WBL - Hunt Street	52		52		0		0%		37.7		73		506	-	D	
			WBL - 44th Street	5		5		0		-4%		40.1	27.9	73	ł	506	1	D	С
		WB	TH	490	559	493	560	3	1	1%	0%	26.9	21.5	73	81	506	520	C	Ü
			RT	12		11		-1		-12%		18.3		81		520	1	В	
	<u> </u>	Intersection			721	1,7	708	-	13		%	29	).1						С
			TH	1,317		1,308		-10		-1%		184.5		0		0		F	_
	Kenilworth	SB	RT	56	1,373	56	1,364	0	-9	1%	-1%	145.6	182.9	0	0	0	0	F	D
316	Avenue NE at	EB	RT	240	240	239	239	-1	-1	0%	0%	16.6	16.6	27	27	188	188	В	В
	Foote Street NE	Intersection	1	1,6	313	1,6	603		10	-1	%	15	8.1						-

<sup>\*</sup>Simulated level of service is approximated based on delay but is not equivalent to that produced using Highway Capacity Manual methodology.

<sup>\*</sup>The intersections are numbered based on their inclusion in both the Streetcar and IMR projects (100s), the Streetcar project (200s), and the IMR project (300s).

#### 6.1.1.4 Travel Time Calibration

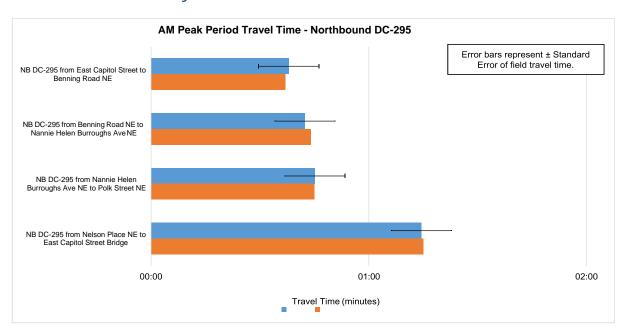
# 6.1.1.4.1 AM Peak Period (7:00 AM - 9:00 AM)

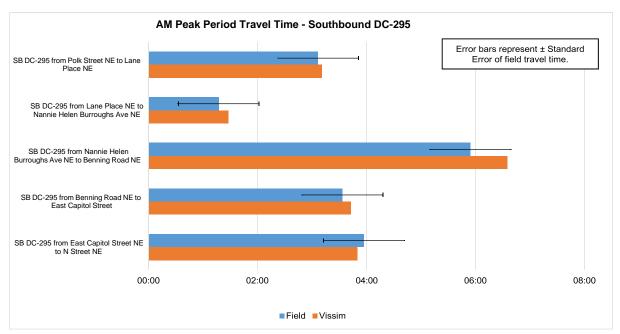
	Travel Time Criteria	Total	Percent	Target	Target Met
NB 295	Within ± 15% for average travel time on freeways	1	100%	85%	Yes
SB 295	Within ± 15% for average travel time on freeways	1	100%	85%	Yes
EB Benning	Within ± 15% for average travel time on arterials	1	100%	85%	Yes
WB Benning	Within ± 15% for average travel time on arterials	1	100%	85%	Yes
Total (n = 4)	Within ± 15% for average travel time	4	100%	85%	Yes

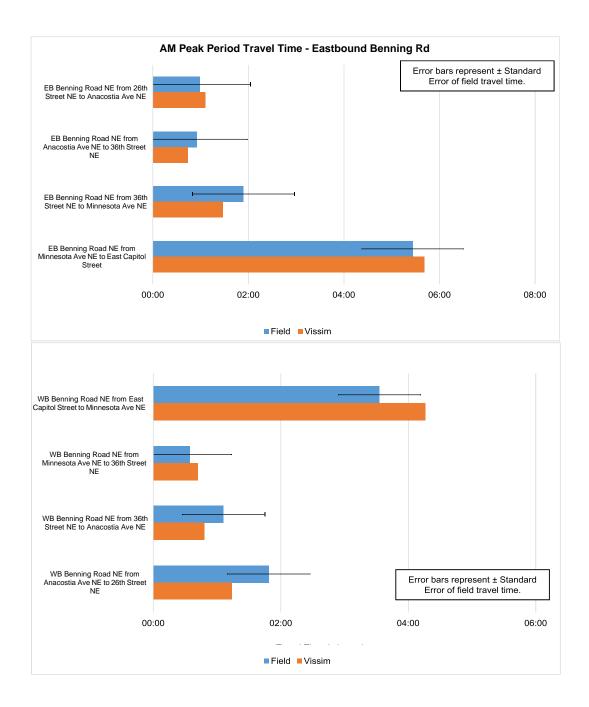
\* MOE of Vissim Travel Time represents the average from 10 simulation runs.

			Peak	Period Trave	el Time	
Segment ID	Route	Field (MM:SS)	VISSIM (MM:SS)	Difference (MM:SS)	Difference (%)	VISSIM STDEV (MM:SS)
5	NB DC-295 from East Capitol Street to Benning Road NE	00:38	00:37	-00:01	-3%	00:00
6	NB DC-295 from Benning Road NE to Nannie Helen Burroughs Ave NE	00:42	00:44	00:02	4%	00:00
7	NB DC-295 from Nannie Helen Burroughs Ave NE to Polk Street NE	00:45	00:45	-00:00	0%	00:00
18	NB DC-295 from Nelson Place NE to East Capitol Street Bridge	01:15	01:15	00:00	1%	00:01
Total	NB DC-295	03:20	03:21	00:01	0%	
8	SB DC-295 from Polk Street NE to Lane Place NE	03:07	03:11	00:04	2%	00:24
9	SB DC-295 from Lane Place NE to Nannie Helen Burroughs Ave NE	01:17	01:28	00:11	14%	00:12
10	SB DC-295 from Nannie Helen Burroughs Ave NE to Benning Road NE	05:54	06:35	00:41	11%	00:43
11	SB DC-295 from Benning Road NE to East Capitol Street	03:33	03:43	00:10	5%	00:20
12	SB DC-295 from East Capitol Street NE to N Street NE	03:57	03:50	-00:07	-3%	00:05
Total	SB DC-295	17:49	18:47	00:58	5%	
14	WB Benning Road NE from East Capitol Street to Minnesota Ave NE	03:33	04:16	00:43	20%	01:37
15	WB Benning Road NE from Minnesota Ave NE to 36th Street NE	00:35	00:42	00:07	21%	00:00
16	WB Benning Road NE from 36th Street NE to Anacostia Ave NE	01:06	00:48	-00:18	-27%	00:02
17	WB Benning Road NE from Anacostia Ave NE to 26th Street NE	01:49	01:14	-00:35	-32%	00:07
Total	WB Benning Road NE	07:03	07:00	-00:03	-1%	
1	EB Benning Road NE from 26th Street NE to Anacostia Ave NE	00:59	01:06	00:07	12%	00:02
2	EB Benning Road NE from Anacostia Ave NE to 36th Street NE	00:55	00:44	-00:11	-20%	00:02
3	EB Benning Road NE from 36th Street NE to Minnesota Ave NE	01:54	01:28	-00:26	-23%	00:31
4	EB Benning Road NE from Minnesota Ave NE to East Capitol Street	05:26	05:41	00:15	4%	02:41
Total	EB Benning Road NE	09:14	08:59	-00:15	-3%	

#### 6.1.1.5 AM Calibration Summary







#### 6.1.1.6 Queue Length Calibration

#### 6.1.1.6.1 AM Peak Period (7:00 AM - 9:00 AM)

	Queue Criteria	Total	Percent	Target	Target Met
Approaches (n = 14)	Modeled queues qualitatively reflect the impacts of observed queues (e.g., spillback from ramp intersections, turn bay, or downstream intersection)	11	92%	85%	Yes

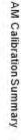
#### 6.1.1.6.2 Freeway Ramps

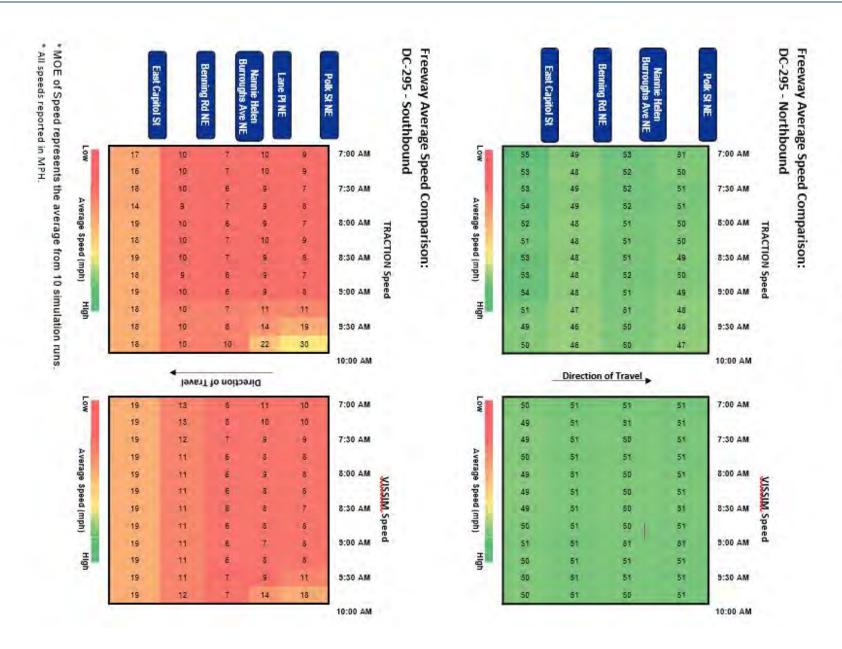
Interchange	Location	Observed Max Queue (feet)	VISSIM Max Queue (feet)	Max Queue Difference (feet)	Max Queue Difference (%)	Field Conditions Represented (Yes/No)	Vissim Max Queue Standard Deviation (feet)	Field-Observed Queue Description	Queue Calibration Justification
	Eastbound Benning Road NE to Southbound DC-295	250	0	-250	-100%	N	0	Slowdown from Google Maps typical traffic conditions estimated to extend 650 feet	The Vissim model displayed consistently reduced speeds on this ramp; however, the reduced speeds did not result in a queue being measured in Vissim.
	Eastbound Benning Road NE/ Southbound Kenilworth Avenue NE to Northbound DC-295	,	7	-	-	Y	8	Slowdown from Google Maps typical traffic conditions estimated to extend 0 feet	
	Southbound Kenilworth Avenue NE to Northbound DC-295	,	18	-	-	Y	13	Queue from Google Maps typical traffic conditions estimated to be 0 feet	
Benning Road	Northbound DC-295 to Westbound Benning Road NE	-	380	-	-	Y	129	Queue from Google Maps typical traffic conditions estimated to be 200 feet	Traffic was observed to occupy two lanes' space approaching the stop sign after the weaving area, which results in shorter queue length than VISSIM output; however, such yield behavior is difficult to model and therefore was not coded. The resultant queue impact does not affect operations at other locations because average queue is much shorter.
	Northbound DC-295/Southbound Kenilworth Avenue NE to Westbound Benning Road NE	-	0	-	-	Y	0	Slowdown from Google Maps typical traffic conditions estimated to extend 800 feet	The westbound service road merges with Benning Road without needing to stop; Google data represents a slowdown due to traffic friction, not queuing.

# 6.1.1.7 PM Calibration Summary

#### 6.1.1.7.1 Arterial Queue Counters

Interchange	Location	Observed Max Queue (feet)	VISSIM Max Queue (feet)	Max Queue Difference (feet)	Max Queue Difference (%)	Field Conditions Represented (Yes/No)	Vissim Max Queue Standard Deviation (feet)	Field-Observed Queue Description	Queue Calibration Justification
	Southbound Benning Road NE at East Capitol Street	550	1,059	509	93%	Y	429	Queue spillback from East Capital Street intersection was observed to impact 44th and 45th Street intersections	The queue spillback impact was reasonably modeled
Benning Road and	Northbound Benning Road NE at East Capitol Street	475+	726	-	-		281	Slowdown from Google Maps typical traffic conditions estimated to extend 1900 feet	The queue spillback impact was reasonably modeled; field measured queue data was limited to adjacent driveways (e.g., A Street SE, B Street SE)
East Capitol Street Intersection	Westbound East Capitol Street at Benning Road NE	750+	776	-	-		171	Slowdown from Google Maps typical traffic conditions estimated to extend 550 feet	The queue spillback impact was reasonably modeled
	Eastbound East Capitol Street at Benning Road NE	200	279	79	40%	Y	55	Queue spillback from Benning Road intersection was observed to fill the short block between Texas Avenue and Benning Road all the time	The queue spillback impact was reasonably modeled; field measured queue data was limited to Texas Avenue
Benning Road and Oklahoma Avenue Intersection	Westbound Benning Road at Oklahoma Avenue	300+	704	-	-	Y	235	Queue spillback from the intersection extends out of the turn lane frequently and as far as Anacostia Avenue intersection; Slowdown from Google Maps typical traffic conditions estimated to extend 2600 feet	The queue spillback impact was reasonably modeled; field measured queue data was limited.
	Eastbound Benning Road NE at Minnesota Avenue NE	300+	554	-			112	Slowdown from Google Maps typical traffic conditions estimated to extend 725 feet	The queue spillback impact was reasonably modeled
Benning Road and	Westbound Benning Road NE at Minnesota Avenue NE	300+	411	-	-	Y	47	Queue spillback from Minnesota Avenue intersection was observed to extend east of 39th Street	The queue spillback impact was reasonably modeled; field measured queue data was limited to adjacent intersections
Minnesota Avenue	Northbound Minnesota Avenue NE at Benning Road NE	475	454	-21	-5%	Y	66	Slowdown from Google Maps typical traffic conditions estimated to extend 700 feet	The queue spillback impact was reasonably modeled
	Southbound Minnesota Avenue NE at Benning Road NE	400	537	137	34%	Y	101	Slowdown from Google Maps typical traffic conditions estimated to extend 1000 feet	The queue spillback impact was reasonably modeled





# 6.1.1.8 Volume Calibration (Freeways)

# 6.1.1.8.1 PM Peak Period (4:30 PM - 5:30 PM)

		Subtotal	Total	Percent	Target	Target Met
Comments	Within ± 100 vph for < 700 vph	11				
Segments	Within ± 15% for ≥ 700 vph to < 2,700 vph	5	29	100%	85%	Yes
(n = 29)	Within $\pm 400$ vph for $\geq 2,700$ vph	13				

\* MOEs of Throughput, Speed, and Density represent an average of 10 simulation runs.

Facility	Segment	Туре	Balanced Count (vph)	VISSIM Throughput (vph)	Difference (vph)	Difference (%)	Average Speed (mph)	Average Density (vpmpl)
	Mainline south of EB E Capitol Street On-Ramp	Basic	2,660	2,895	235	9%	16	91.9
	Between EB E Capitol Street On-Ramp and WB Benning Road NE Off-Ramp	Merge/Diverge	3,585	3,836	251	7%	14	93.5
NB DC-295	Between WB Benning Road Off-Ramp and EB Benning Road/Kenilworth Avenue NE On-Ramp	Basic	3,490	3,737	247	7%	12	106.4
NB DC-295	Between EB Benning Road NE/Kenilworth Avenue NE On-Ramp and Nannie Helen Burroughs Avenue NE Off-Ramp	Merge/Diverge	4,375	4,572	197	4%	19	75.2
	Between Nannie Helen Burroughs Avenue NE Off-Ramp and Kenilworth Avenue NE On-Ramp	Basic	3,560	3,773	213	6%	12	102.0
	Mainline north of Nannie Helen Burroughs Avenue	Merge/Basic	3,865	4,057	192	5%	13	86.1
	Mainline North of Deane Avenue NE Off-Ramp	Merge/Diverge	3,345	3,466	121	4%	51	20.0
	Between Deane Avenue NE Off-Ramp and Kenilworth Avenue NE Off-Ramp	Basic/Diverge	3,005	3,122	117	4%	51	19.5
	Between Kenilworth Avenue NE Off-Ramp and Kenilworth Avenue NE On-Ramp	Basic	2,775	2,887	112	4%	52	18.5
SB DC-295	Between Kenilworth Avenue NE On-Ramp and EB Benning Road NE On-Ramp	Merge/Basic	3,025	3,110	85	3%	50	20.0
3B DC-293	Between EB Benning Road NE On-Ramp and Baker Street Off-Ramp	Weave	3,300	3,368	68	2%	36	34.4
	Between Baker Street Off-Ramp and WB E Capitol Street Off-Ramp	Diverge	3,245	3,330	85	3%	21	70.5
	Between WB E Capitol Street Off-Ramp and EB E Capitol Street On-Ramp	Basic	2,925	3,008	83	3%	20	79.4
	Mainline south of EB E Capitol Street On-Ramp	Merge/Basic	3,080	3,170	90	3%	25	68.9

Interchange	Ramp		Balanced Count (vph)	VISSIM Throughput (vph)	Difference (vph)	Difference (%)	Average Speed (mph)	Average Density (vpmpl)
	EB E Capitol Street to SB DC-295	Ramp	155	156	1	0%	29	5.5
Capitol Street	EB E Capitol Street to NB DC-295	Ramp	925	920	-5	0%	27	35.0
	SB DC-295 to WB E Capitol Street	Ramp	320	330	10	3%	40	8.5
Baker Street	SB DC-295 to WB Baker Street	Ramp	55	56	1	1%	20	2.8
	EB Benning Road NE to SB DC-295	Ramp	275	266	-9	-3%	36	7.6
	EB Benning Road NE to NB DC-295 or WB Benning Road NE (U-turns)	Ramp	840	798	-42	-5%	15	64.3
Danning Band	NB DC-295 to WB Benning Road NE	Ramp	95	97	2	2%	30	3.6
Benning Road	NB DC-295/EB Benning Road NE (U-turns) to WB Benning Road NE	Ramp	85	97	12	14%	13	8.7
	SB Kenilworth Avenue NE to NB DC-295	Ramp	35	32	-3	-8%	18	2.2
	EB Benning Road NE/SB Kenilworth Avenue NE to NB DC-295	Ramp	885	833	-52	-6%	10	88.1
	SB Kenilworth Avenue NE to SB DC-295 (South)	Ramp	250	229	-21	-8%	25	8.9
Kenilworth Avenue	SB DC-295 to SB Kenilworth Avenue NE	Ramp	230	234	4	2%	39	6.0
	NB Kenilworth Avenue NE to NB DC-295	Ramp	305	289	-16	-5%	20	16.0
Nannie Helen	NB DC-295 to Nannie Helen Burroughs Avenue NE/Kenilworth Avenue NE	Ramp	815	805	-10	-1%	39	22.1
Burroughs Avenue	SB DC-295 to Deane Avenue NE/Kenilworth Terrace NE/Kenilworth Avenue NE	Ramp	340	342	2	1%	41	8.1

# 6.1.1.9 Volume Calibration (Intersections)

# 6.1.1.9.1 PM Peak Period (4:30 PM - 5:30 PM)

	Volume Criteria	Subtotal	Total	Percent	Target	Target Met
	Within ± 100 vph for < 700 vph	64				
Approaches (n = 77)	Within ± 15% for ≥ 700 vph to < 2,700 vph	21	85	98%	85%	Yes
(11 = 77)	Within ± 400 vph for ≥ 2,700 vph	0				



Approach	es:
LOS	Count
A-D	71
E	6
F	10

#	Intersection	Approach	Movement	(vr	d Count oh)	Through	SIM put (vph)	Differen	ce (vph)	Differe	nce (%)	(sec	je Delay /veh)	Lei (fe	je Queue ngth eet)	(fe	eue Length eet)	-	os*
		NB	RT	23	86	55 19	73	-8 -4	-12	-13% -17%	-14%	8.8	44.0	17	17	120	124	E A	D
	Ī		LT	1	_	2	_	1		59%		82.2		2		33		F	_
	Benning Road	SB	RT	2	3	4	6	2	2	85%	76%	47.5	58.8	2	2	33	33	D	Е
101	NE at Anacostia		LT	1		1		0		-10%		205.8		851		1,892		F	
	Avenue NE	EB	TH RT	1,768 304	2,073	1,610 271	1,881	-158 -33	-191	-9% -11%	-9%	205.1 106.6	190.9	851 869	869	1,892 1,956	1,956	F	F
			LT	304 9		7		-33		-11%		20.4		869 5		1,956		C	
		WB	TH	944	953	910	918	-34	-35	-4%	-4%	2.5	2.6	5	5	89	89	A	Α
		Intersect		3,1	15	,	378	-2	37	-8	3%		6.9				•		F
			LT TH	30		30 0		-1 -2		-2% -100%		36.3	40.0	12 12		168 168		D	
		NB	RT	88	120	91	121	3	1	4%	1%	13.8	19.3	12	12	178	178	- В	В
I	1		LT	19		9		-10		-52%		52.5		2		48		D	
	1	SB	TH	4	30	1	14	-3	-16	-73%	-54%	30.2	45.4	2	2	48	48	С	D
102	Benning Road		RT TH	7 1,772		4 1,576		-4 -197		-50% -11%		31.9 166.5		2 741		48 861		C	
102	NE at 34th Street NE	EB	RT	20	1,792	17	1,592	-3	-200	-17%	-11%	76.2	0.001	763	763	883	883	E	г
	"-		LT	141		136		-5		-3%		117.9		102		351		F	
		WB	TH RT	917 6	1,064	885 4	1,036	-32 -2	-28	-4% -34%	-3%	10.1 9.0	25.5	40 40	102	302 309	351	B A	С
			U	16		11		-6		-34%		129.3		102	1	351		F	
	1	Intersect	ion	3,0	006	2,7	752	-2	54	-8	3%	10	6.5		!		,		F
	Danning Dani	NB	RT	241	241	240	240	-1	-1	0%	0%	6.8	6.8	3	3	129	129	Α	А
	Benning Road NE Ramp to DC-		TH	874		758		-116		-13%		58.7	58.6	824		998		F	F
103	295 at 36th	EB	RT	2	876	2	760	0	-116	0%	-13%	34.5		743	824	851	998	D	
	Street NE	Intersect		1,1	17	1,0	000	-1	17		1%		6.2	. 40	L	301		_	E
		iiilei sect	LT	132		137		5		3%	. ,3	147.3		349	T T	581		F	
	1	NB	TH	257	411	314	469	57	58	22%	14%	135.5	138.8	349	349	581	581	F	F
I	1		RT	22		18		-4		-16%		131.9 52.9		349		581		F	
	1	SB	LT TH	52 306	429	52 287	416	-19	-13	0% -6%	-3%	52.9 42.6	38.8	12 88	88	187 424	424	D	D
	Benning Road	ЭВ	RT	71	429	78	410	7	-13	9%	-5%	15.8	55.0	88	- 00	424	424	В	_
104	NE at Minnesota		LT	203		185		-19		-9%		600.8		1,611		1,676		F	
	Avenue NE	EB	TH RT	561 256	1,020	470 221	875	-91 -35	-144	-16% -14%	-14%	346.8 293.7	386.9	1,611 1,611	1,611	1,676 1,676	1,676	F	F
	<del> </del>		LT	14		11		-3		-14%		130.7		103		297	+	F	
		WB	TH	256	357	243	349	-13	-8	-5%	-2%	82.7	82.0	103	103	297	297	F	F
		Intersect	RT	87 2.2	16	95	109	8 -1	07	10%	5%	74.6	2.7	103		297		Е	
		intersect	IUII	2,2	.10	Ζ,	103	-1	U1	-5	70	21	2.1						

#	Intersection	Approach	Movement	Balanced Count (vph)	VISSIM Throughput (vph)	Difference (vph)	Difference (%)	Average Delay (sec/veh)	Average Queue Length (feet)	Max Queue Length (feet)	LOS*
		NB	RT	9 19	9 20	0 1	10%	21.3 49.4	2 4	75 75	E D
		SB	LT RT	5 9	3 8	-1 -5 -1	-33% -100% -13%	51.8	1 0 1	28 28	D C
105	Benning Road NE at 39th Street NE/Driveway	ЕВ	LT TH RT	10 604 20 634	9 516 539	-1 -88 -6	-8% -15% -31%	6.8 8.2 8.1	25 25 25 25	401 401 401	A A A
		WB	LT TH RT	19 342 9	15 336 9	-4 -6 -1	-22% -2% -10%	8.6 2.9 4.4	3 3 3	85 85 85 85	A A A
		Intersect		1,032	926	-107	-10%	7.4			À
		NB	LT RT	31 261 292	33 277 310	2 17 18	5% 6% 6%	19.9 16.6	29 29 29	206 206	C C
106	Benning Road NE at 40th Street	ЕВ	TH RT LT	592 25 617	506 22 528	-86 -3 -11	-15% -12% -14%	0.7 1.4 0.7	3 3	128 123 125	A A
	NE	WB	TH	339 477	127 326 453	-14 -25	-8% -4% -5%	1.5 2.6	5 5 5	125 125 125	A A
		Intersect		1,386	1,290	-96	-7%	5.3			A
		NB	LT RT	98 102	87 89	-3 -11 -13	-63% -11% -13%	11.3 8.4 8.4	3 3	63 64 64	A A
107	Benning Road NE at 41st Street	EB	TH RT	847 6 853	778 6 784	-69 0 -69	-8% -3%	0.5	0 0	63 5 63	A A
	NE	WB	LT TH	41 473 515	44 451 495	3 -23 -20	7% -5% -4%	7.3 0.8	4 4	106 106	A A
		Intersect	ion	1,469	1,367	-102	-7%	1.4			A
		NB	LT TH RT	32 79 12	38 96 14	6 17 2 25	18% 22% 21% 19%	31.3 30.5 19.3	21 21 22 22	157 157 171	C C
	Benning Road	SB	LT TH RT	70 221	20 65 114	-3 -5 -14	-15% -8% -11%	30.5 29.4 21.5	20 20 20 22 22	179 179 179 196	C C
108	NE at 42nd Street NE	ЕВ	LT TH RT	316 574 945 55	297 515 864	-19 -58 -4	-6% -10% -6%	20.9 12.1 14.9	51 51 62	311 311 336	B A
		WB	LT TH RT	4 355 35 35	3 342 32 378	-1 -12 -3	-26% -3% -7%	13.1 8.2 5.7	11 11 10	149 149 168	В А А
		Intersect		1,682	1,588	-94	-6%	15.5	·	·	В
		SB	LT RT	264 64 328	259 62 321	<del>-4</del> -2 -6	-2% -3%	67.5 57.6 65.6	143 153 153	552 567 567	E E
208	Benning Road NE at 26th Street	EB	LT TH	40 1,741 1,781	1,831 1,872	90 91	3% 5% 5%	19.1 16.6 16.6	1 68 68	100 439 439	B B
	NE	WB	TH RT	811 158 968	782 148 930	-29 -10 -39	-4% -6%	4.3	13 11 13	178 184 184	A A
		Intersect		3,077	3,123	46	1%	18.0	40	454	В
		NB	LT RT	71 276 347	238 306	-3 -37 -41	-5% -14% -12%	49.3 9.5 18.3	18 7 18	154 134 154	A B
209	Benning Road NE at Oklahoma	ЕВ	TH RT	1,797 207 2,004	1,882 214 2,095	85 7 91	5% 3% 5%	5.0 6.1 5.1	37 35 37	272 285 285	A A
	Avenue NE	WB	LT TH	113 896 1,009	113 861 974	-35 -35	1% -4% -3%	37.8 4.6 8.4	10 10	121 104 121	D A
		Intersect	ion	3,360	3,376	16	0%	7.3			А

#	Intersection	Approach	Movement	Balanced (vph)		VISS Throughp		Differen	ce (vph)	Difference (%)		ge Delay :/veh)	Ler (fe	e Queue ngth eet)	(fe	ue Length eet)		os*
		NB	TH RT	350 79	434	454 95	556	1 104 16	122	30% 21% 28%	73.9 79.9 120.6	86.8	193 193 197	197	815 815 823	823	E E F	F
	Minnesota	SB	LT TH RT	58 502 15	576	53 455 11	519	-5 -48 -4	-57	-9% -9% -29%	24.7 10.1 7.2	11.6	20 20 25	25	250 250 273	273	B A	В
210	Avenue NE at Dix Street NE	ЕВ	LT TH RT	19 30 17	66	17 29 17	63	-2 -1 0	-4	-11% -4% -2%	48.0 34.3 35.8	38.4	11 11 11	11	105 105 105	105	C D	D
		WB	LT TH RT	150 29 41	220	161 28 42	231	11 -1 1	11	7% -4% 5%	98.4 99.8 94.6	97.8	141 141 144	144	543 543 549	549	F F	F
		Intersect		1,297		1,3		7	1	6%	5	7.9						Ē
		NB	TH	546	546	593	593	47	47	9% 9%	21.9	21.9	71	71	609	609	C	С
	Minnesota	SB	TH	385 20	385	383	383	-2	-2	-1% -1%	3.0	3.0	5 19	5	78 152	78	A D	Α
211	Avenue NE at Bus Exit South	ЕВ	LT RT	43	63	19 33	51	-1 -10	-12	-7% -24% -19%	42.4 25.6	31.7	19	19	152 141	152	С	С
		Intersect		994		1,0:	27		3	3%		5.3						3
	Minnesota	NB	LT TH RT	40 470 55	565	16 517 49	582	-24 47 -7	17	-60% 10% -12%	20.0 17.0 16.6	17.0	59 59	59	78 375 375	375	B B	В
212	Avenue NE at Grant Street NE and Bus	SB	LT TH RT	11 345 14	370	10 361 0	372	-1 17 -14	2	-6% 5% -100%	25.4 11.9	12.3	15 15 15	15	135 135 162	162	С В -	В
	Entrance North	WB	LT RT	39 29	68	34 26	60	-5 -3	-8	-12% -11% -12%	16.5 5.2	11.7	3 2	3	64 97	97	B A	В
		Intersect	tion LT	1,003	3	1,0	14	1	1	1%		5.0	2	ı	E4	1	В	В
		NB	RT	3 51	54	52	53	-2 1	-1	-70% 2% -2%	9.1	9.2	2	2	54 60	60	A	Α
213	Benning Road NE at Blaine	ЕВ	TH RT LT	603 5 44	608	543 6 40	549	-60 1 -4	-59	-10% 14% -8%	1.6 1.9 6.3	1.6	0	0	3 34 84	34	A A A	Α
	Street NE	WB	TH	390	434	376	416	-15	-18	-4%	0.4	0.9	0	1	53	84	A	Α
		Intersect	tion LT	1,097	′	1,0 176	18	-10	79	-7% -5%	36.5	1.7	50		275		D	A
		SB	RT LT	84	270	78	254	-6 -16	-16	-7% -6%	25.5	33.1	50 41	50	282	282	С	С
214	Benning Road NE at 44th Street	EB	TH	165 491	656	149 445	595	-45	-61	-10% -9% -9%	26.2 13.2	16.5	41	41	319 319	319	C B	В
	NE	WB	TH RT	351 192	543	337 187	524	-14 -5	-19	-4% -3%	11.0 11.5	11.2	24 19	24	249 286	286	B B	В
		Intersect		1,468	3	1,3	73		96	-7%		7.5						В
		SB	LT RT	14 14	28	8 7	15	-5 -7	-12	-40% -49% -45%	19.2 7.8	14.0	0	1	38 42	42	B A	В
215	Benning Road NE at 45th Street	ЕВ	LT TH	28 648	676	26 599	624	-2 -50	-52	-9% -8%	11.4 8.6	8.7	16 16	16	246 246	246	B A	Α
	NE	WB	TH RT	527 51	578	518 53	571	-9 2	-8	-2% -1%	1.0 3.0	1.2	1 6	6	138 224	224	A	Α
		Intersect		1,283	3	1,2	10		72	-6%		5.2						A
	Benning Road	WB	TH RT	481 23	504	471 20	491	-10 -2	-13	-2% -11% -3%	0.4	0.4	0	0	63 79	79	A	Α
216	NE at Central	EB	TH	671	671	607	607	-64	-64	-10% -10%	49.9	49.9	100	100	322	322	E	E
	Avenue NE	SB Intersect	RT	100	100	100	100 os	-1 -7	-1 '8	-1% -1% -6%	3.9	3.9 5.8	3	3	67	67	А	D A
		intersect		1,275	,	1,13	<i>3</i> 0	-/	U	-0 /0		J.U						,

#	Intersection	Approach	Movement	Balance			SIM put (vph)	Differen	ce (vph)	Differe	nce (%)		je Delay /veh)	Ler	e Queue ngth eet)		ue Length eet)	LC	os*
			LT	172		161		-11		-6%		65.0		80		300		Е	
		NB	TH	291	507	302	505	10	-2	4%	0%	64.8	63.7	80	86	300	313	E	E
	<u> </u>		RT	44		42		-2		-4%		51.1		86		313		D	
			LT TH	192		169		-24		-12%		1.3		0		0		A	
	Benning Road	SB	RT	380 100	672	351 87	607	-29 -13	-65	-8%	-10%	0.6	0.8	0 85	85	12 166	166	A	Α
217	NE at East		LT	126		109		-13		-13% -13%		47.8		34		203		A D	
	Capitol Street SE	ЕВ	TH	1,653	2,002	1,687	0.040	34	40	2%	2%	2.2	4.7	34	36	203	220	A	Α
	SE	ЕВ	RT	223	2,002	246	2,042	23	40	10%	2%	3.2	4.7	36	36	220	220	A	A
	<u>-</u>		TH	648		644		-5		-1%		39.8	38.5	68		285		D	D
		WB	RT	87	735	80	724	-7	-11	-8%	-2%	28.5	30.3	81	81	315	315	C	U
	<del> </del>	Intersect		3,9	16		378		39		%		8.1	0.		0.0			3
	+	EB	RT	218	218	218	218	0	0	0%	0%	70.5	70.5	70	70	320	320	Е	Е
		ND	TH	1,779	4 000	1,812	4.000	33	20	2%	00/	47.2	47.0	234	040	728	740	D	_
	East Capitol	NB	RT	23	1,802	21	1,833	-2	32	-7%	2%	51.4	47.3	243	243	740	740	D	D
218	Street SE at		U	5		18		13		254%		32.2		35		224	]	С	
1 2.0	Texas Avenue	SB	LT	213	920	205	906	-8	-14	-4%	-2%	28.4	7.4	35	35	224	224	С	Α
	Se		TH	702		683		-19		-3%		0.4		4		119		Α	
	<u>-</u>	Intersect	tion	2,9	40	2.9	957	1	7	1	%	36	6.8				ļ.		D
			LT LT	73		68	1	-5		-7%		1.5		0		21	ı	А	
		SB	RT	1	74	0	68	-1	-6	-100%	-8%	-	1.5	0	0	21	21	-	Α
	I F		LT	0		0		0	_	-		-		-		-		-	
310	Deane Avenue	EB	TH	2	2	0	0	-2	-2	-100%	-100%	-	-	0	0	0	0	-	-
	NE at Lee St NE	WB	TH	1	107	5	105	4	-2	386%	-2%	0.9	1.0	0	0	11	11	А	Α
	_		RT	106		101		-6		-5%		1.0	1.0	0		11	''	A	^
		Intersect		18	13		73		10		5%	10.4	.2	.=		011		,	λ
		NB	LT RT	13 262	275	10 229	239	-3 -33	-36	-23% -13%	-13%	49.1 17.3	18.6	17 13	17	214 194	214	D B	В
			LT	208		171		-38		-13%		47.3		41		216		D	
		SB	TH	70	281	73	243	3	-38	4%	-13%	41.0	45.4	51	51	244	244	D	D
	Deane Avenue	02	RT	3	201	0	243	-3	-50	-100%	-1370	-	40.4	0	31	0	244	-	J
311	NE at Kenilworth		TH	68	70	63		-5	_	-7%	00/	46.2	44.0	14	4-	90	400	D	_
	Terrace NE	EB	RT	10	78	9	72	-1	-6	-11%	-8%	28.3	44.0	15	15	120	120	С	D
I	Γ	WB	LT	125	219	121	215	-4	-4	-4%	-2%	1.9	1.2	1	1	21	21	Α	Α
			TH	94		94		0		0%		0.4		1	<u> </u>	21		A	
-		Intersect	LT LT	265	<b>13</b>	270	69		34		0%		4.6	52		217		D	
		SB	TH	265 19	284	270	293	5 4	9	2% 19%	3%	53.3 53.2	53.3	52 52	52	217	217	D	D
	Deane Avenue		TH	524		452		-72	-	-14%		9.9		32		169		A	
312	NE at Kenilworth	EB	RT	14	538	10	463	-4	-76	-28%	-14%	3.2	9.8	36	36	176	176	A	Α
•	Avenue NE	MD	LT	282	400	275	400	-7	_	-2%	40/	15.1	45.4	20	- 00	226	200	В	
I		WB	TH	211	493	215	490	4	-3	2%	-1%	15.9	15.4	20	20	226	226	В	В
		Intersect		1,3	15		246		70		5%		2.2						)
			U	147		137		-11		-7%		121.9		1,038		2,816		F	
	Nannie Helen	NB	LT	36	337	32	327	-4	-10	-10%	-3%	140.9	134.3	1,024	1,038	2,859	2,859	F	F
	Burroughs		TH	153	470	158	457	5		3%	40/	143.8	401	0	4.000	22	0.050	F	
	Avenue NE at		RT LT	478	478	457	457	-21	-21	-4%	-4%	161.1	161.1	1,026	1,026	2,859	2,859	F	
313	Kenilworth	EB	TH	131 660	791	121 612	733	-10 -48	-58	-8% -7%	-7%	10.4 17.9	16.6	15 15	15	233	233	В	В
	Avenue NE and		TH	459		612 458						17.9 29.4		15 127		347		В	
	DC-295 U-turns	WB	RT	584	1,043	553	1,011	-1 -31	-32	0% -5%	-3%	29.4	29.3	149	149	347	376	C	С
		Intersect		2,6	10		528	-51	21		5%		3.0	140	·	0.0			E
		intersect		2,0	70	Ζ,	<i>7</i> 20	-1			,,,,	0.	0.0						

#	Intersection	Approach	Movement	(vph) Throughput (vph)			e Delay /veh)	Ler	e Queue ngth eet)		ue Length et)	LC	os*						
		NB	LI TH RT	383 135 52	570	406 150 63	619	24 15 11	49	6% 11% 21%	9%	43.7 27.3 5.5	35.8	121 18 16	121	5/5 164 168	575	C A	D
	Nannie Helen Burroughs	SB	TH RT	59 152	211	56 136	193	-3 -16	-18	-5% -10%	-9%	50.2 28.5	34.8	24 31	31	153 174	174	D C	С
314	Avenue NE at Minnesota	ЕВ	LT TH RT	137 695 305	1,137	130 659 282	1,070	-7 -36 -24	-67	-5% -5% -8%	-6%	47.0 30.3 24.8	30.9	139 139 146	146	344 344 354	354	C	С
	Avenue NE	WB	LT TH RT	41 507 19	526	50 466 19	535	9 -42 0	8	22% -8% -2%	2%	75.6 59.8 59.0	61.2	25 112 113	113	162 437 439	439	E E	E
		Intersect			144		416	-2	28	-1	%		9.2						D
		NB	NBU-onto Hunt LT TH RT	0 142 84 94	320	0 113 69 74	257	-29 -15 -20	-64	- -20% -18% -21%	-20%	40.0 36.4 20.2	33.3	35 35 33	35	254 254 257	257	D D	С
	Nannie Helen	SB	LT TH SBR - NHB EB SBR - Hunt	13 69 32 28	142	14 65 29 27	135	1 -5 -3 -1	-8	5% -7% -8% -3%	-5%	37.2 34.8 24.2 34.2	32.6	24 24 25 24	25	180 180 196 180	196	C C	С
315	Burroughs Avenue NE at 44th Street NE and Hunt Place	ЕВ	LT TH EBR - Hunt Street EBR - 44th Street	31 446 2 273	752	28 430 0 263	721	-3 -16 -2 -10	-31	-8% -4% -100% -4%	-4%	12.5 10.7 - 17.7	13.3	39 39 39 39	39	415 415 415 415	415	B B - B	В
	NE NE	Hunt Street	NBL- NHB EB NBT - 44th SB NBR - NHB WB	87 51 58	196	93 56 68	218	6 5 10	22	7% 9% 17%	11%	43.6 44.4 42.8	43.6	55 55 55	55	276 276 276	276	D D D	D
		WB	WBL - Hunt Street WBL - 44th Street TH RT	38 4 307 16	365	38 4 287 16	345	0 0 -20	-20	0% -10% -7% 2%	-6%	72.1 72.6 32.1 18.2	36.2	48 48 48 56	56	263 263 263 278	278	E E C B	D
		Intersect			775		674	-1	01		%		5.6		,			(	C
216	Kenilworth 316 Avenue NE at Foote Street NE	SB	TH RT	495 71	566	464 64	527	-32 -7	-39	-6% -10%	-7%	2.2	2.2	0	0	0 5	5	A A	А
310		EB Intersect	RT ion	76 6	76 42	52 5	52 79	-24 -6	-24 63	-31% -10	-31% 0%	5.6 2	5.6 .5	2	2	57	57	A	A A

<sup>\*</sup>Simulated level of service is approximated based on delay but is not equivalent to that produced using Highway Capacity Manual methodology.

\*The intersections are numbered based on their inclusion in both the Streetcar and IMR projects (100s), the Streetcar project (200s), and the IMR project (300s)

#### 6.1.1.10 Travel Time Calibration

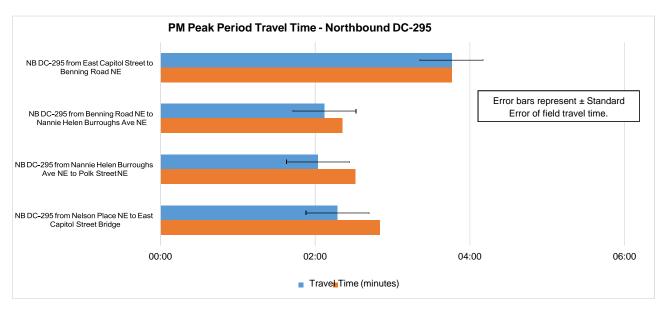
# 6.1.1.10.1 PM Peak Period (4:30 PM - 6:30 PM)

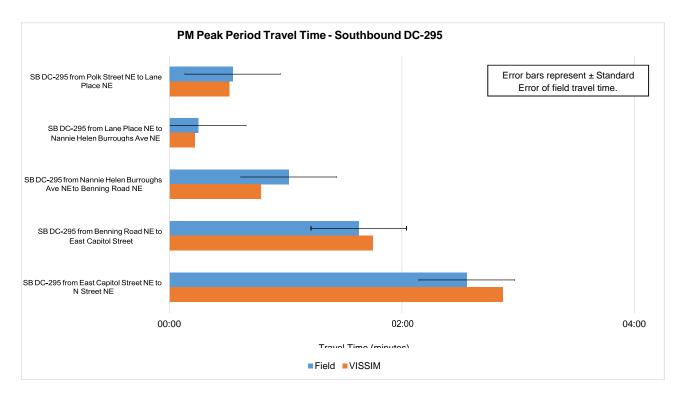
	Travel Time Criteria	Total	Percent	Target	Target Met
NB 295 (n = 1)	Within ± 15% for average travel time on freeways	1	100%	85%	Yes
SB 295 (n = 1)	Within ± 15% for average travel time on freeways	1	100%	85%	Yes
EB Benning (n = 1)	Within ± 15% for average travel time on arterials	1	100%	85%	Yes
WB Benning (n = 1)	Within ± 15% for average travel time on arterials	1	100%	85%	Yes
Total (n = 4)	Within ± 15% for average travel time	4	100%	85%	Yes

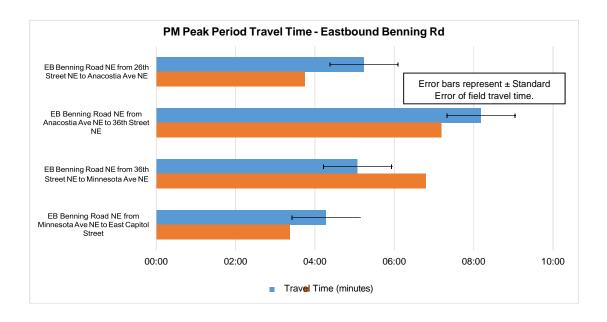
\* MOE of Vissim Travel Time represents the average from 10 simulation runs.

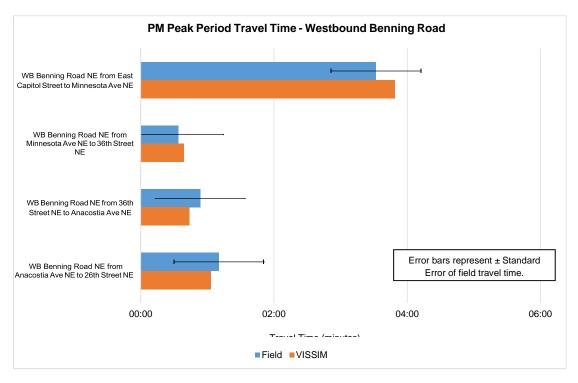
			Peak	Period Trave	l Time	
Segment ID	Route	Field (MM:SS)	VISSIM (MM:SS)	Difference (MM:SS)	Difference (%)	VISSIM STDEV (MM:SS)
18	NB DC-295 from East Capitol Street to Benning Road NE	03:46	03:46	00:00	0%	00:51
5	NB DC-295 from Benning Road NE to Nannie Helen Burroughs Ave NE	02:07	02:21	00:14	11%	00:14
6	NB DC-295 from Nannie Helen Burroughs Ave NE to Polk Street NE	02:02	02:31	00:29	24%	00:09
7	NB DC-295 from Nelson Place NE to East Capitol Street Bridge	02:17	02:50	00:33	24%	80:00
Total	NB DC-295	10:13	11:28	01:15	12%	
8	SB DC-295 from Polk Street NE to Lane Place NE	00:33	00:31	-00:02	-5%	00:00
9	SB DC-295 from Lane Place NE to Nannie Helen Burroughs Ave NE	00:15	00:13	-00:02	-13%	00:00
10	SB DC-295 from Nannie Helen Burroughs Ave NE to Benning Road NE	01:02	00:47	-00:15	-24%	00:07
11	SB DC-295 from Benning Road NE to East Capitol Street	01:38	01:45	00:07	7%	00:38
12	SB DC-295 from East Capitol Street NE to N Street NE	02:33	02:52	00:19	12%	00:14
Total	SB DC-295	06:01	06:08	00:07	2%	
14	WB Benning Road NE from East Capitol Street to Minnesota Ave NE	03:32	03:49	00:17	8%	00:21
15	WB Benning Road NE from Minnesota Ave NE to 36th Street NE	00:34	00:39	00:05	15%	00:01
16	WB Benning Road NE from 36th Street NE to Anacostia Ave NE	00:53	00:44	-00:10	-18%	00:01
17	WB Benning Road NE from Anacostia Ave NE to 26th Street NE	01:10	01:03	-00:07	-10%	00:01
Total	WB Benning Road NE	06:09	06:15	00:05	1%	
1	EB Benning Road NE from 26th Street NE to Anacostia Ave NE	05:14	03:45	-01:29	-28%	00:42
2	EB Benning Road NE from Anacostia Ave NE to 36th Street NE	08:11	07:11	-01:00	-12%	03:15
3	EB Benning Road NE from 36th Street NE to Minnesota Ave NE	05:05	06:48	01:43	34%	00:59
4	EB Benning Road NE from Minnesota Ave NE to East Capitol Street	04:17	03:22	-00:55	-21%	00:31
Total	EB Benning Road NE	22:46	21:06	-01:40	-7%	

#### 6.1.1.11 PM Calibration Summary









# 6.1.1.12 Queue Length Calibration

#### 6.1.1.12.1 PM Peak Period (4:30 PM - 5:30 PM)

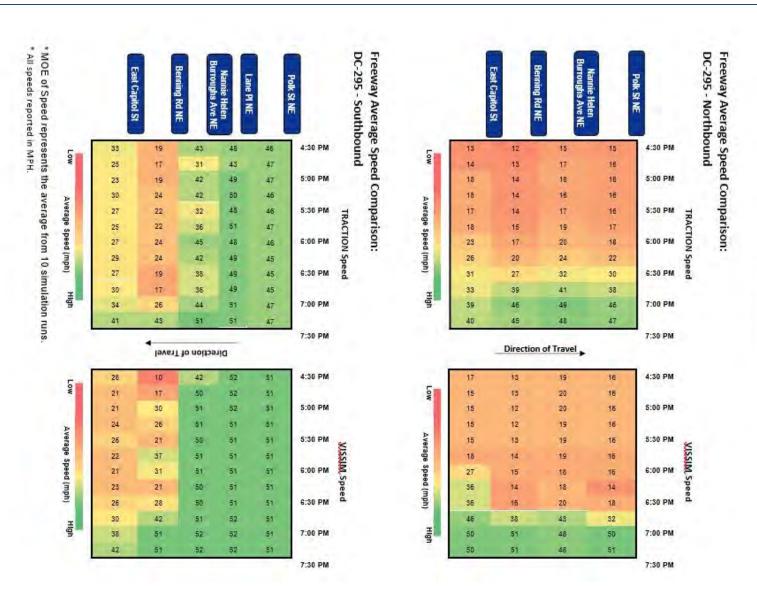
	Queue Criteria	Total	Percent	Target	Target Met
Approaches (n = 14)	Modeled queues qualitatively reflect the impacts of observed queues (e.g., spillback from ramp intersections, turn bay, or downstream intersection)	13	93%	85%	Yes

# 6.1.1.12.2 Freeway Ramps

Interchange	Location	Observed Max Queue (feet)	VISSIM Max Queue (feet)	Max Queue Difference (feet)	Max Queue Difference (%)	Field Conditions Represented (Yes/No)	Vissim Max Queue Standard Deviation (feet)	Field-Observed Queue Description	Queue Calibration Justification
	Eastbound Benning Road NE to Southbound DC-295	75	291	216	288%	Y	46		The queue spillback impact was reasonably modeled.
	Eastbound Benning Road NE/Southbound Kenilworth Avenue NE to Northbound DC- 295	-	4,603	-	-	Y	1142	Slowdown from Google Maps typical traffic conditions estimated to spill back to Benning Road (> 1500 feet)	Vissim queues extend back to Benning Road, as estimated by Google Maps typical traffic conditions; queue length output is likely overestimated due to queue spillback on eastbound Benning Road NE from Minnesota Avenue NE.
Benning Road NE	Southbound Kenilworth Avenue NE to Northbound DC-295	-	49	-	-	Y	15	Slowdown from Google Mapstypical traffic conditions estimated to extend 100 feet	The queue spillback impact was reasonably modeled.
	Northbound DC-295 to Westbound Benning Road NE	-	79	-	-	N	62	Slowdown from Google Maps typical traffic conditions estimated to extend 0 feet	Queue length output is not reflective of observed conditions in the simulation and may be overestimated due to queue spillback from other movements; measured throughput at this location exceeds volume targets.
	Northbound DC- 295/Southbound Kenilworth Avenue NE to Westbound Benning Road NE	-	0	-	-	Y	0	Slowdown from Google Maps typical traffic conditions estimated to extend 0 feet	

#### 6.1.1.12.3 Arterial Queue Counters

Intersection	m Max Queue Lengtl	Observed Max Queue (feet)	VISSIM Max Queue (feet)	Max Queue Difference (feet)	Max Queue Difference (%)	Field Conditions Represented (Yes/No)	Vissim Max Queue Standard Deviation (feet)	Field-Observed Queue Description	Queue Calibration Justification
	Eastbound Benning Road NE at E Capitol Street	400	337	-63	-16%	Y	123	Slowdown from Google Mapstypical traffic conditions estimated to extend 800 feet	The queue spillback impact was reasonably modeled.
	Westbound Benning Road NE at E Capitol Street	475	278	-197	-42%	Y	46	Slowdown from Google Mapstypical traffic conditions estimated to extend 200 feet	The queue spillback impact was reasonably modeled.
Benning Road NE at E Capitol Street	Southbound E Capitol Street at Benning Road NE	375	276	-99	-26%	Y	35	Slowdown from Google Maps typical traffic conditions estimated to extend 0 feet	The queue spillback impact was reasonably modeled.
	Northbound E Capitol Street at Benning Road NE	150	708	558	372%	Y	71	Slowdown from Google Mapstypical traffic conditions estimated to extend 2000 feet	The queue spillback impact was reasonably modeled; queue length at this location is highly variable, as shown by the difference in queue lengths reported from field measurements and Google Maps typical traffic conditions.
Benning Road NE at Oklahoma Avenue NE	Westbound Benning Road NE at Oklahoma Avenue NE	125	121	-4	-3%		17	Slowdown from Google Maps typical traffic conditions estimated to extend 0 feet	The queue spillback impact was reasonably modeled.
	Eastbound Benning Road NE at Minnesota Avenue NE	300+	5057	-	-	Y	889	Slowdown from Google Mapstypical traffic conditions estimated to extend 4200 feet	The queue spillback impact was reasonably modeled and aligns with the length of slowdown estimated by Google Maps typical traffic conditions; slightly longer queue lengths are attributable to the lane closure present during data collection.
Benning Road NE at Minnesota Avenue NE	Westbound Benning Road NE at Minnesota Avenue NE	300	297	-3	-1%	Y	30	Slowdown from Google Mapstypical traffic conditions estimated to extend 600 feet	The queue spillback impact was reasonably modeled.
INE	Northbound Minnesota Avenue NE at Benning Road NE	325+	581	-	-	Y	9	Slowdown from Google Mapstypical traffic conditions estimated to extend 700 feet	The queue spillback impact was reasonably modeled.
	Southbound Minnesota Avenue NE at Benning Road NE	425	424	-1	0%	Y	40	Slowdown from Google Mapstypical traffic conditions estimated to extend 1000 feet	The queue spillback impact was reasonably modeled.



PM Calibration Summary

# 7.0 Attachment B - Opening Year 2025 Results

#### 7.1.1.1 Freeway and Ramp MOEs Comparison

#### 7.1.1.1.1 AM Peak Hour (7:45 AM - 8:45 AM); PM Peak Hour (4:30 PM - 5:30 PM)

				2025 NB AM			2025 Build AM	Л		2025 NB PM		2	2025 Build PM	
Facility	Segment	Туре	VISSIM Throughput (vph)	Average Speed (mph)	Average Density (vpmpl)									
	Mainline south of EB Capitol Street On-Ramp	Basic	2,895	49	30	2,895	49	30	2,942	17	89	2,900	16	90
	Between EB Capitol Street On-Ramp and WB Benning Road Off-Ramp	Merge/Diverge	3,260	51	21	3,260	51	21	3,846	14	91	3,865	14	91
NB DC-295	Between WB Benning Road Off-Ramp and EB Benning Road/Kenilworth Avenue On-Ramp	Basic	2,990	51	19	2,990	51	19	3,751	12	105	3,760	12	104
	Between EB Benning Road/Kenilworth Avenue On-Ramp and Nannie Helen Burroughs Avenue Off-Ramp	Merge/Diverge	3,475	50	21	3,475	50	21	4,635	20	74	4,705	20	73
	Between Nannie Helen Burroughs Avenue Off-Ramp and Kenilworth Avenue On-Ramp	Basic	2,795	52	18	2,795	52	18	3,769	12	103	3,710	12	102
	Mainline north of Nannie Helen Burroughs Avenue	Merge/Basic	3,330	43	21	3,330	43	21	4,060	13	86	4,015	13	86
	Mainline North of Deane Avenue Off-Ramp	Merge/Diverge	3,290	8	116	3,290	8	115	3,445	51	20	3,405	51	20
	Between Deane Avenue Off-Ramp and Kenilworth Avenue Off-Ramp	Basic/Diverge	3,000	7	134	3,000	7	135	3,100	51	19	3,015	51	19
	Between Kenilworth Avenue Off-Ramp and Kenilworth Avenue On- Ramp	Basic	2,190	4	166	2,250	4	164	2,866	52	18	2,805	52	18
SB DC-295	Between Kenilworth Avenue On-Ramp and EB Benning Road On-Ramp	Merge/Basic	2,930	6	143	3,050	7	139	3,107	51	19	3,100	51	20
	Between EB Benning Road On-Ramp and Baker Street Off-Ramp	Weave	3,245	8	100	3,365	8	99	3,368	51	17	3,410	51	17
	Between Baker Street Off-Ramp and WB Capitol Street Off-Ramp	Diverge	3,205	11	91	3,265	12	90	3,326	49	23	3,345	48	24
	Between WB Capitol Street Off-Ramp and EB Capitol Street On-Ramp	Basic	2,690	12	103	2,690	12	103	2,972	44	35	2,925	41	39
	Mainline south of EB Capitol Street On-Ramp	Merge/Basic	3,145	15	90	3,145	15	90	3,103	33	50	3,080	31	52

				2025 NB AM			2025 Build AN	1		2025 NB PM		2025 Build PM			
Facility	Segment	Туре	VISSIM Throughput (vph)	Average Speed (mph)	Average Density (vpmpl)										
	EB Capitol Street to SB DC-295	Ramp	402	25	17	402	25	17	155	29	5	161	29	6	
East Capitol Street	EB Capitol Street to NB DC-295	Ramp	325	32	10	325	32	10	915	28	34	954	27	37	
	SB DC-295 to WB Capitol Street	Ramp	492	40	13	544	40	14	355	43	9	362	43	9	
Baker Street	SB DC-295 to WB Baker Street	Ramp	28	20	1	74	20	4	52	20	3	55	20	3	
	EB Benning Road to SB DC-295	Ramp	283	36	8	283	36	8	273	36	8	251	36	7	
	EB Benning Road to NB DC-295	Ramp	302	25	17	316	28	12	848	12	83	816	13	76	
	NB DC-295 to WB Benning Road	Ramp	274	31	12	273	32	9	95	29	4	102	30	4	
Benning Road	NB DC-295/EB Benning Road (U-turns) to WB Benning Road	Ramp	274	3	118	288	4	77	94	13	8	103	13	9	
	SB Kenilworth Avenue NE to NB DC-295	Ramp	49	23	2	50	23	2	35	17	3	40	16	4	
	EB Benning Road/SB Kenilworth Avenue NE to NB DC-295	Ramp	350	38	9	352	39	9	885	10	95	864	11	91	
	SB Kenilworth Avenue to SB DC-295 (South)	Ramp	644	5	130	681	5	140	243	25	9	267	25	10	
Kenilworth Avenue	SB DC-295 to SB Kenilworth Avenue	Ramp	714	17	51	661	16	49	235	39	6	212	39	5	
	NB Kenilworth Avenue to NB DC-295	Ramp	533	23	22	537	24	22	296	17	20	289	17	20	
	NB DC-295 to Nannie Helen Burroughs Avenue/Kenilworth Avenue	Ramp	488	47	10	492	47	10	882	41	21	895	41	23	
Burroughs Avenue	SB DC-295 to Deane Avenue/Kenilworth Terrace/Kenilworth Avenue	Ramp	246	35	9	247	35	10	342	40	9	346	40	9	

# 7.1.1.2 Intersection MOE Comparison

# 7.1.1.2.1 AM Peak Hour (7:45 AM - 8:45 AM); PM Peak Hour (4:30 PM - 5:30 PM)

						2025 N	IB AM					2025 B	uild AM					2025 NE	B PM					2025 Buil	ld PM		
No.	Intersection ID	Approach	Movement		hroughput ph)	Average (sec/		LC	)S*	Vissim Th (vp			ge Delay :/veh)	LC	os*		hroughput ph)	Average (sec/		LO	os*	Vissim Th (vp		Average (sec/		LOS	
		NB	LT	79	115	50	37	D	D	121	157	52	43	D	D	41	65	61	40	E	D	39	64	60	38	E	D
		, ND	RT	36		8	0.	A		36		12	.0	В		24		6		Α		26	•	6	00	Α	
		SB	LT RT	20 8	28	52 48	51	D D	D	20 8	28	48 48	48	D D	D	0	0	-	-	-	-	0	0	-	-	-	-
	Benning Road NE at		LT	12		19		В		11		40		D		5		10		-		4		29		-	
101	Anacostia Avenue	EB	TH	971	1,008	3	3	Α	Α	971	1,006	8	9	A	Α	2,136	2,239	7	7	Α	Α	2,062	2,161	20	20	С	С
	NE		RT	26		6		Α		24		11		В		98		7		Α		94		21		С	
			LT TH	20	0.707	9		A		17 2,565	0.504	15 9		B		5 1,180	4.405	16		B		5 1,140	4.450	34 4	_	C	
		WB	RT	13	2,797	6	2	A	Α	12	2,594	12	9	A B	A	1,100	1,195	3	1	Α -	Α	1,140	1,150	6	5	A -	Α
			Intersection	3,	949	3	3		A	3,7	'85		11		В	3,4	499	6	3	F	A	3,3	75	1	5	B	
		ND	LT	20	79	41	20	D	С	20	79	3	7	A	А	29	104	37	19	D	В	22	94	36	17	D	В
		NB	RT	55	79	12	20	В	C	56	79	6	′	A	A	75	104	12	19	В	Ь	72	94	11	17	В	ь
		SB	LT	30 40	70	36 34	35	D	С	28 36	64	35 35	35	C D	D	43 10	55	37 34	36	D C	D	41 12	55	35	35	C D	D
			RT LT	12		21		C		8		35 25		С		3		13		В		0		36 38		D	
	Benning Road NE at	EB	TH	992	1,025	22	22	C	С	992	1,020	24	24	C	С	2,115	2,153	12	12	В	В	2,049	2,085	24	24	C	С
102	34th Street NE		RT	21	1	17		В		20		21	1	С		35	,	7		Α		35	,	29		С	
			LT	251		23		С		249		20		В		172		27		С		159		25		С	
		WB	TH	2,734 48	3,111	15 21	16	В	В	2,534	2,920	17 19	17	В	В	1,158	1,373	9	11	A	В	1,112	1,315	10	12	В	В
			RT U	78	1	26		C		57 80		22	4	B C		21		9 28		A C		21		8 24		A C	
			Intersection	4,	208	1	8	Ü	В	4,0	102	22	20	Ü	B	3,6	662	1	3	Ü	3	3,5	26	2	0	В	
		NB	RT	132	132	2	2	Α	Α	132	132	2	2	Α	Α	249	249	58	58	F	F	254	254	7	7	Α	Α
103	Benning Road NE Ramp to DC-295 at	EB	TH	453	458	2	2	Α	Α	468	475	9	9	A	Α	881	881	28	28	D	D	841	841	21	21	С	С
103	36th Street NE		RT Intersection	5	100	2		Α	, ,	7	17	3		Α		0	130	1 3		-		0	011	-		-	Ů
			LT	301	190	65	4	Е	<u> </u>	252	И	46	1	D	A .	203	130	50	5	D	)	195	94	38	0	D	
		NB	TH	427	764	21	38	C	D	412	693	16	27	В	С	526	795	17	26	В	С	539	801	28	30	C	С
			RT	37		19		В		30		15		В		65		24		С		67		26		С	
			LT	58		55		E	_	57		46		D		55		64		E	_	55		53		D	_
	Benning Road NE at	SB	TH RT	522 372	951	45 49	47	D D	D	529 338	924	36 39	38	D D	D	473 199	728	32 24	32	C	С	475 192	722	41 27	38	D C	D
104	Minnesota Avenue		LT	193		36		D		186		32		C		250		15		В		236		26		C	
	NE	EB	TH	298	691	18	24	В	С	289	669	10	17	В	В	811	1,375	17	17	В	В	796	1,356	21	23	C	С
			RT	201		20		С		194		14		В		314		20		В		324		25		С	
			TH	687		37		D		661		37		D		314	0	39	0	D	0	308	0	42	0	D	0
			RT Intersection	47	140	40	7	D	)	48	196	49	31	D	Ċ	95	306	44	6	D		89	76	60	1	E	
			LT	13	1	59		Е	_	6		50	T	D		0		- 1		-	_	11		56		E	
		NB	RT	67	81	59	59	E	Е	61	69	55	54	D	D	141	141	57	57	E	E	136	148	58	57	E	E
		SB	LT	3	8	61	62	E	Е	3	8	60	61	E	Е	4	4	47	50	D	D	3	8	39	40	D	D
	Barrella a Barrella a		RT LT	10	<u> </u>	14		- B		4 10		67 15	ļ	E B	<u> </u>	0 12		7		- A		4 11		39 10		D A	
105	Benning Road NE at 39th Street	EB	TH	363	392	5	5	A	Α	349	376	3	3	A	A	889	901	2	2	A	Α	877	918	7	7	A	Α
100	NE/Driveway		RT	19		7	•	A		18	0.0	4	1 ັ	A	1	0	1	-	-	-		30	0.0	8		A	
			LT	139		9		Α		140		9		Α		87		18		В		94		14		В	
		WB	TH	715	864	7	7	A	Α	698	849	7	7	A	Α	0	96	-	17	-	В	373	477	7	8	A	Α
			RT Intersection	10	344	6	0	Α	R .	11	102	6	9	Α	A	9	142	6	0	Α	3	10	51	7	2	A	
			LT	6		12		В		5		13		В		40		21		С		42		18		С	
		NB	RT	67	73	6	6	Α	Α	64	69	6	7	Α	Α	143	183	13	15	В	В	141	183	13	14	В	В
	Benning Road NE at	EB	TH	407	432	1	1	Α	А	387	413	1	1	Α	А	986	1,033	1	1	Α	А	965	1,015	2	2	A	Α
106	40th Street NE		RT LT	25	.02	2	'	A		26	.13	1	<u> </u>	A	_^	47	.,555	2		A		50	.,515	2	_	A	
		WB	LT TH	194 857	1,051	2	2	A	Α	190 844	1,034	4	2	A	Α	99 449	548	9	3	A A	Α	96 434	530	9	3	A	Α
			Intersection	1,	556	2	2	^	A .	1,5	16		2	^	A	1.1	765	- 3	3	^	A .	434	28	- 3	3	A	
										1,0						.,											

						2025	NB AM					2025 B	uild AM					2025 NE	B PM								
No.	Intersection ID	Approach	Movement		hroughpu		ge Delay :/veh)	L	OS*		roughput ph)		je Delay /veh)	LC	os*		nroughput ph)	Averag (sec		LO	S*	Vissim Th		Average (sec/		LO	S*
		NB	LT	4	27	11	7	В	А	5	27	11	7	В	Α	3	103	16	10	С	Α	3	103	14	10	В	Α
	Benning Road NE at		RT TH	23 464		6		A	<u> </u>	22 441		6		A A		100 1,123		10		A		99 1,094		10		A A	
107		EB	RT	10	474	1	0	A	A	10	451	1	0	A	Α	5	1,127	1	1	A	Α	6	1,099	1	1	A	Α
	41st Street NE	WB	LT	109	1,156	5	2	Α	А	106	1,133	4	2	Α	Α	39	584	12	2	В	Α	39	566	10	2	Α	Α
			TH Intersection	1,047	657	2	_	Α		1,027	1,100	2		Α		545	001	1		Α		527	67	1		Α	
_			LT	25	,007	26		С	A T	24	)	25		С	A .	26	010	31	2	C		23	07	32	2	C	•
		NB	TH	60	89	21	22	С	С	61	89	23	23	С	С	106	144	28	28	С	С	105	138	27	27	С	С
			RT	4		12		В		4		13		В		12		18		В		10		17		В	
		SB	LT TH	9 141	483	48 51	45	D	D	9	478	52 55	49	D	D	20 89	254	38	26	D C	С	20 92	255	41 37	30	D D	С
		OB	RT	333		43	3	D	1 ້	328	470	47	3	D		145	254	20	20	В	Ü	143	200	23	30	C	O
108	Benning Road NE at 42nd Street NE		LT	97		18		В		95		16		В		420		21		С		427		25		С	
	4211d Olicet NE	EB	TH	347	487	11	12	В	В	327	462	4	7	A	Α	737	1,222	13	16	В	В	700	1,187	14	18	В	В
			RT LT	43		7		A B	<u> </u>	41 6		9 15		A B		65 4		10 24		A C		60 5		10 15		A B	
		WB	TH	793	830	12	12	В	В	775	814	11	11	В	В	414	444	13	13	В	В	399	431	7	7	A	Α
			RT	33		12		В		34		11		В		27	0	10		В		27		5		Α	
			Intersection LT	174	,889	60	21	E	С	176	343	55	20	Е	С	148	)64	50 50	7	D	3	2,0 154	10	48	7	D B	3
		SB	RT	55	228	46	56	D	E	53	229	44	53	D	D	16	164	30	48	C	D	14	167	34	47	C	D
	Barrello o Barrel NE at	EB	LT	71	947	37	11	D	В	71	927	45	10	D	В	11	2,070	11	8	В	Α	10	2,035	14	7	В	Α
208	Benning Road NE at 26th Street NE	EB	TH	877	341	9	'''	Α		857	921	7	10	Α	Ь	2,059	2,070	8	0	Α		2,026	2,000	7	,	Α	^
		WB	TH RT	1,948 463	2,410	12	8	A B	Α	1,826 464	2,289	7 13	8	A B	Α	1,069 126	1,194	4 6	4	A	Α	1,033	1,166	6	4	A A	Α
			Intersection	403	.586	12	12	В	В	3.4	145	13	12	ь	В	3.4	128	0	3	^		3.3	69		3	A A	\
		NB	LT	73	123	55	34	Е	С	76	129	57	35	Е	С	36	118	55	21	D	С	51	80	54	36	D	D
	Benning Road NE at Oklahoma Avenue NE	NB	RT	50	123	4	34	Α	Ü	53	129	4	33	A	Ü	82	110	7	21	A	C	29	00	4	30	A	ь
209		EB	TH RT	974 78	1,053	10	10	A B	Α	957 78	1,035	8 11	8	A B	Α	2,165 52	2,217	3	2	A	Α	2,137 53	2,190	1	1	A A	Α
205			LT	523		42		D		463		41		D	_	70	4.000	24	4	C		61		26	_	C	
		WB	TH	2,340	2,863	26	29	С	С	2,216	2,679	25	28	С	С	1,158	1,228	3	4	Α	Α	1,117	1,178	6	7	Α	Α
			Intersection TH	650	,038	20	24	С	С	3,8 586	343	10	23	В	С	653	563	24	3	C	١.	3,4 659	48	26	1	C	١
			RT	63	-	25		C	-	59		11		В		108	0	25	0	C	0	112	0	26	0	C	0
			LT	50		15		В		54		4		Α		110		22		С		112		27		С	
		SB	TH	651	722	7	7	A	Α	649	722	3	3	A	Α	652	788	15	16	В	В	661	801	13	15	В	В
	Minnesota Avenue		RT LT	20 12	<del>                                     </del>	8 51		A D	<u> </u>	19 10		8 44		A D		26 23		11 31		B C		28 26		16 33		B C	
210	NE at Dix Street NE	EB	TH	13	49	43	46	D	D	11	43	38	44	D	D	18	73	30	31	C	С	20	79	30	32	C	С
			RT	24		45		D		23		46		D		31		33		С		33		32		С	
		WB	LT TH	96 12	208	63 68	64	E	Е	96	202	58 58	58	E F	F	177 32	333	45 41	42	D	D	174 31	325	46 47	46	D D	D
		WD	RT	99	200	64	04	E		95	202	58	56	E	-	124	333	37	42	D	D	120	323	47	40	D	D
			Intersection	1	,692	2	21		С	1,6	315	1	14		В	1,9	963	2	4	C	)	1,9	82	2	5	C	)
آوا		NB	TH	560	560	14	14	В	В	540	540	15	15	В	В	873	873	13	13	В	В	876	876	13	13	В	В
211	Minnesota Avenue NE at Bus Exit	SB	TH LT	928	928	9	9	Α -	Α	903	903	9	9	A	A	579 1	579	5	5	A	Α	564 10	564	5 52	5	A D	Α
	South	EB	RT	21	21	15	15	В	В	20	20	17	17	В	В	17	18	10	9	A	Α	30	40	28	34	C	С
			Intersection	1.	,509	1	11		В	1,4	163	1	11		В	1,4	170	1	0	A	l	1,4	80	1	1	В	3
		NB	LT TH	0 488	543	31	31	- C	С	0 485	531	32	32	- C	С	0 745	854	- 13	13	- B	В	13 750	868	23 14	15	C B	В
		ND	RT	488 55	343	32	31	C	ქ ്	485	551	32	32	C	1	110	004	13	13	В	٥	105	000	17	15	В	D
	Minnesota Avenue NE at Grant Street		LT	17		20		В		15		20		В		33	471	23	13	С	В	29	462	24	13	С	В
212	NE at Grant Street NE and Bus	SB	TH	752	769	15	15	В	В	734	749	14	14	В	В	438	0	13	0	В	0	432	0	13	0	В	0
	Entrance North		RT LT	169	1	18		- В	<del> </del>	170		18		- В		139	0	17	0	- B	0	0 144	0	17	0	- B	0
	Z.Arance Horar				208	11	17	В	В	38	208	12	17	В	В	35	174	10	16	В	В	35	179	10	16	A	В
		WB	RT	40																							
			Intersection	1.	,520	2	21		С	1,4	189	2	21		С	1,4	199	1	4	Е	3	1,5	08	1	4		
			Intersection RT	48	,520	8	21	A	C	48	189	7	21	A	C	30	0 760	10	0	A	0	29	08	10	0	A	0
213	Benning Road NE at	EB	Intersection RT TH	48 359	, <b>520</b> 360	3	3	Α	A	336	339	1	1	Α	A	765	769	1	1	Α	A	725	729	1	1	Α	A
213	Benning Road NE at Blaine Street NE		Intersection RT	48	,520 360 1,063		3		A A		339 1,046	7 1 5	1 2		A A			10 1 8 0						10 1 1 5 0			

	Intersection ID					2025 N	NB AM					2025 B	Build AM					2025 N	В РМ			2025 Build PM					
No.		Approach	Movement	Vissim Ti (v	hroughput ph)	Averag (sec		LC	os*	Vissim Th			ge Delay c/veh)	LC	os*		hroughput ph)		e Delay /veh)	LC	os*		hroughput ph)		e Delay /veh)	LC	os*
		SB	LT	135	400	30	24	С	С	129	396	36	30	D	С	132	211	39	35	D	С	132	206	38	33	D	С
	Benning Road NE at 44th Street NE	- OD	RT	265	700	21	2-7	С	Ŭ	267	330	27	30	С	Ŭ	79	211	28	55	С		74	200	25	55	С	<u> </u>
214		EB	LT TH	99 306	405	95 20	38	C	D	92 291	383	23 11	13	C B	В	131 665	796	13	6	B A	Α	139 615	753	14 7	9	B A	Α
214			TH	794		14		В		777		15		В		390		4		A		377		5		A	
		WB	RT	166	960	15	15	В	В	164	940	23	16	С	В	167	556	4	4	Α	Α	158	535	5	5	Α	A
			Intersection	1,7	765	2	2	1	C	1,7	719		19		В	1,	563		9	1	A	1,	494	1	1		B
		SB	LT RT	55	97	93	85	F E	F	54	95	80	68	F	Е	14	29	16 8	12	В	В	14	29	44	27	D	С
			LT	42 24		76 37		D		41 23		52 28	-	D C		15 20		8		A A		15 20		11 10		B A	<del></del>
215	Benning Road NE at	EB	TH	419	443	31	31	C	С	394	417	23	23	C	С	777	797	6	6	A	Α	723	742	7	8	A	A
	45th Street NE	WB	TH	916	979	1	2	Α	Α	891	949	8	8	Α	Α	542	592	1	-1	Α	А	516	565	3	3	Α	А
			RT	63	979	3	2	Α	А	57	949	10	۰	Α	Α.	50	592	2		Α	А	50	303	5	3	Α	Α
			Intersection	1,5	519	1	6		В	1,4	160	0	16		В	19	418		4		A	1,	337		6	لــــا	A.
	Benning Road NE at Central Avenue NE	WB	TH RT	591 20	610	0	1	A	Α	565 24	589	0	3	A	Α	499 21	520	0	1	A A	Α	472 19	491	0	1	A A	Α
216		EB	TH	359	359	79	79	F	F	350	350	61	61	F	F	618	618	27	27	D	D	559	559	21	21	C	С
		SB	RT	384	384	9	9	Α	Α	380	380	17	17	С	С	92	92	4	4	Α	Α	93	93	4	4	Α	Α
			Intersection	1,0	353	2	4		C	1,3	319	:	22		Ċ	1,	230	1	4		В	1,	143	1	1		3
		NB	LT	122 318	500	36 7	40	D	_	131	570	34	40	C	_	90 1.433	4.055	87	40	F	١.	66 1.433	4 000	93	9	F	١. '
		NB	TH RT	130	569	2	12	A	В	315 130	576	7	12	A	В	332	1,855	2	10	A A	Α	331	1,830	3	9	A A	A
	Benning Road NE at East Capitol Street SE		LT	13		201		F		14		263		F		42		61		E		53		64		E	$\vdash$
		SB	TH	1,254	1,278	214	214	F	F	1,269	1,295	210	210	F	F	644	739	32	33	С	С	634	741	33	34	С	С
			RT	11		210		F		12		198		F		53		19		В		54		21		С	
217			LT	111	470	25	40	С	В	96	445	25	40	С	_	173	704	17	40	В	_	178	707	19	40	В	
		EB	TH RT	278 80	470	21	19	C A	В	279 71	445	21	19	C A	В	356 262	791	12	10	B A	В	339 220	737	15 4	13	B A	В
			LT	297		49		D		322		56		E		50		115		F		43		97		F	
		WB	TH	472	789	47	48	D	D	443	785	47	50	D	D	377	483	43	51	D	D	371	468	44	48	D	D
			RT	20		42		D		20		42		D		56		41		D		55		39		D	
			Intersection	3,	106	10	05		F	3,1	101	1	106		F	3,	867	1	9	_	В	30	776	447	20	F	3
		NB	RT RT2	73	330	12	38	В	D	69	323	13	39	В	D	40 0	157	86 0	82	0	F	0	145	117	87	0	F
		NR	LT	236	000	49	00	D		233	020	50	- 00	D	1	103		77	02	Ē	i '	115		79	0,	E	1 ' '
	East Capitol Street	EB	TH	372	389	33	33	С	С	484	495	26	26	С	С	1,543	1,601	114	114	F	F	1,555	1,633	113	113	F	F
218	SE at Texas Avenue		RT	18	309	17	33	В	C	12	433	21	20	С	C	58	1,001	109	114	F		79	1,000	109	113	F	
	SE	WB	U LT	80	1,881	10	2	В	А	0 84	1,920	11	2	В	А	0 255	959	27	8	- C	А	0 222	899	23	6	- C	A
		***5	TH	1,801	1,001	2		A	^	1,836	1,520	2	- f	A	^	704	303	0	0	A	^	677	033	0	0	A	l ^
			Intersection	2,6	601	1	1		В	2,7	738		11		В	2,	718		4		Ė	2,	677	7	5		È
		SB	LT	224	224	398	398	F	F	212	212	426	426	F	F	77	77	2	2	Α	Α	79	79	2	2	Α	Α
			RT LT												·												
310	Deane Avenue NE at	EB	TH	0	4	-	321	-	F	0	4	674	387	-	F	0	0	-	-	-	-	0	0	-	-	- '	-
0.0	Lee St NE		TH			_							<u> </u>												1		
		WB	RT	- 5	88	2	1	-	Α	6	86	4	1	-	Α	0	118	-	1	-	A	0	122	_	1	-	Α
			Intersection	3	16	28	87		F	3	02	3	304		F	1	95		1	1	A	2	01		1		A .
		NB	LT	11	281	119	91	F	F	11	280	183	154	F	F	13	319	79	47	E	D	11	244	72	39	E	D
		ND	RT	270	201	89	31	F		269	200	153	104	F	'	306	313	46	47	D	"	232	244	37	35	D	
	 	SB	LT	581	753	70	71	E	Е	599	779	105	107	F	F	219	307	44	45	D	D	264	355	55	53	E	D
311	Deane Avenue NE at Kenilworth Terrace	28	TH	172	155	74	/ 1	E		181	119	111	107	F	г	88	307	45	45	D		91	300	47	55	D	U
•	NE NE	EB	TH RT	178 57	235	68 39	61	E D	E	169	223	70 41	63	E	E	72 12	84	50	46	D	D n	73 13	86	48	43	D B	D
	-		LT	198		39 5		A		54 188		41 5		D A	<u> </u>	12		20		C A		13		18 1		A A	<del></del>
		WB	TH	54	252	0	4	A	Α	52	240	0	4	A	Α	108	239	0	1	A	Α	117	246	0	1	A	Α
			Intersection	1,5	521	6	62		E	1,5	522		93		F	9	49	3	34	(	C	9	30	3	35		C

						2025 1	NB AM					2025 B	uild AM					2025 NE	В РМ					2025 Buil	ld PM		
No.	Intersection ID	Approach	Movement		hroughput /ph)		je Delay /veh)	LC	os*	Vissim Th (vp		Averag (sec		LC	os*		nroughput ph)	Averag		LO	S*	Vissim TI (v	roughput oh)	Average (sec/		LOS	S*
		SB	LT	105	198	288	291	F	F	105	199	288	291	F	F	307	328	53	53	D	D	313	335	57	58	E	Е
		36	TH	93	190	294	291	F	Г	93	199	293	291	F	г	22	320	50	55	D	D	22	333	60	56	E	
	Deane Avenue NE at	EB	TH	715	1,038	10	8	Α	Α	722	1,046	11	9	В	Α	560	597	14	13	В	В	534	569	14	14	В	В
312	Kenilworth Avenue NE		RT LT	322 572		3 72		A E		324 581	- 1	3 77		A E		38 344		4		A B		35 339		3		A D	
	NE	WB	TH	252	825	30	59	C	E	240	821	40	66	D	E	239	583	15 23	18	С	В	246	585	37 21	30	C	С
			Intersection		.060	5	55		Ė	2,0	66	.0	8	,		1,5	508	2	4	Ů		1,4	89	3	0	Ċ	,
			U	122		20		В		124		17		В		158		145		F		156		148		F	
		NB	LT	30	483	63	12	E	В	32	488	59	11	E	В	37	474	160	152	F	F	42	472	159	154	F	F
	Nannie Helen		TH	27		54		D		26		53		D	_	279		155		F	_	274		157		F	
040	Burroughs Avenue NE at Kenilworth		RT LT	305 170	305	36 42	36	D D	D	306 170	306	33 48	33	C D	С	408 155	408	175 38	175	F D	F	405 158	405	170 42	170	F D	F
313	Avenue NE and DC-	EB	TH	671	841	35	36	C	D	678	848	36	38	D	D	715	870	28	30	С	С	695	853	28	31	С	С
	295 U-turns		TH	811		30		C		810		35		C		546		24		C		541		24		C	
		WB	RT	532	1,343	26	28	C	С	532	1,342	26	31	Č	С	618	1,164	24	24	Č	С	609	1,151	23	23	Č	С
			Intersection	2,	,667	3	2		Ċ	2,6	78	3	4	(		2,9	16	6	8	Ė		2,8	81	6	7	Ė	
			LT	263		318		F		232		421		F		442		47		D		449		50		D	
		NB	TH	130	455	159	249	F	F	110	392	251	349	F	F	445	1,004	33	36	С	D	437	1,005	34	38	С	D
			RT	62		147		F		51		232		F		118		10		В		120		10		A	
	Namela Halan	SB	TH RT	341 377	718	48 109	80	D F	E	328 389	716	52 140	100	D F	F	130 174	304	79 75	76	E	E	132 168	300	80 74	77	F E	E
	Nannie Helen Burroughs Avenue		LT	86		45		D		90		46		D		124		22		C		120		22		C	
314	NE at Minnesota	EB	TH	482	975	29	24	C	С	483	983	29	24	C	С	683	1,125	32	27	C	С	673	1,101	31	26	C	С
	Avenue NE		RT	408		14	1	В	1	411		14	1	В		318	1	19		В		308		19		В	
			LT	48		82		F		46		85		F		66		96		F		68		91		F	
		WB	TH	679	733	41	43	D	D	686	743	40	43	D	D	543	629	104	104	F	F	528	617	91	91	F	F
			RT	6		45		-		11		44		-		20		126		F		21		103		F	
_			Intersection LT	132	,880	71	8	Е	E	133	34	60	3	E	-	155	063	42	1	D	)	151	)24	40	-8	D	
		NB	TH	54	224	42	57	D	Е	56	226	69 43	56	D	Е	84	337	38	39	D	D	87	340	35	34	D	С
		2	RT	37		29	٠.	C	_	37	LLO	29	- 00	C	_	98	007	37	00	D		102	0.0	23	٥.	C	·
			LT	4		30		С		4		32		С		17		32		С		16		31		C	
		SB	TH	205	354	32	32	С	С	202	350	32	32	С	С	73	145	29	27	С	С	69	141	28	20	С	С
		36	SBR - NHB EB	48	334	27	32	С		47	330	26	32	С	C	31	145	18	21	В	C	29	141	20	20	С	C
			SBR - Hunt	96		34		С		96		33		С		25		28		С		28		32		С	
	Nannie Helen		LT TH	14 393	_	25 12	ł	C B	4	13 388		26 11	ł	C B		27 489	ł	21 18		C B		27 489		22 18		C B	
315	Burroughs Avenue	EB	EBR - Hunt Street	3	547	11	13	В	В	4	542	14	13	В	В	2	799	20	20	В	В	2	790	34	20	C	В
3.3	NE at 44th Street		EBR - 44th Street	136	-	16	ł	В	-	136		15	ł	В		281	ł	23		C		272		23		C	
	NE/Hunt Place NE		NBL- NHB EB	29		46		D		29		46		D		96		66		E		93		65		E	
		Hunt Street	NBU - 44th NB	1	49	54	44	-	D	0	49	54	44	-	D	0	204	-	66	-	E	0	203	-	64	-	Е
			NBT - 44th SB	11		47		D		11		47		D		53		66		E		46		64		E	
			WBL - Hunt Street	56		60		E	1	54		73		E		42		63		E		43		88		F	
			WBL - 44th Street	5	578	52 41	43	D D	D	5 519	586	59 49	51	E D	D	335	397	50 29	33	D C	С	334	398	69 33	39	E C	D
		WB	TU	E00				U		9		49	ł	D		16		18		В		17		23		C	
		WB	TH	508				D								10										Ü	
			RT	508 10	.751	36	33	D	C		53	3	36		)	1.8	382	3	2			1.8	373	3	2	С	, ,
					,751		33	D F	C	1,7 1,187		192	400	F	0	1,8 492	500	2	2	A		461	373	2	2	A	ب
246	Kenilworth Avenue	SB	RT Intersection	10	, <b>751</b>	36	189	F F	C D	1,7	1,236	3	190	F	D	492 70	562	2 2	2	(	A	461 68	529	2 2	2	A A	A
316	NE at Foote Street	SB EB	RT Intersection TH RT RT	10 1,277	,751 1,330 237	36 3 191	189 17	F	C D B	1,7 1,187		192	190 14		D B		562 104		2 6	A	A A		529		2 6		A
316		SB EB	RT Intersection TH RT RT	10 1,277 53 237		36 191 151 17		F F B	_	1,7 1,187 50 225	1,236	192 141 14		F B		70 104		6		A A		68	529 34	6		Α	A
316	NE at Foote Street	SB EB	RT Intersection TH RT RT RT LT Intersection LT	10 1,277 53 237 1,105		36 191 151 17 23		F F B	_	1,7 1,187 50 225 1,4	1,236	192 141 14 14 26		F B C		70 104 6		2 6		A A A		68 105 6	529 34 866	2 6 14		A A	A A B
316	NE at Foote Street NE	SB EB NB	RT Intersection TH RT RT Intersection LT TH	10 1,277 53 237 105 561	237	36 3191 151 17 10 23 9	17 63	F F B	B F	1,7 1,187 50 225 1,4 107 539	1,236 225	192 141 14 14 26	14 63 13	F B C B	В	70 104 6 6 867	104	2 6 11 12	6	A A A -	A	68 105 6 1 864	34	2 6 14 11	6	A A - B	\
	NE at Foote Street NE Minnesota Avenue	SB EB	RT   Intersection	10 1,277 53 237 105 561 946	237	36 191 151 17 23 9	17 63	F F B	В	1,7 1,187 50 225 1,4 107 539 917	1,236 225	192 141 14 14 26 11 12	14	F B C B	В	70 104 6 6 867 565	104	2 6 11 12 4	6	A A A B A	A	68 105 6 1 864 562	34	2 6 14	30	A A - B A	\
313 314 315 316	NE at Foote Street NE	SB EB NB SB	RT Intersection TH RT RT Intersection LT TH	10 1,277 53 237 105 561	237 .566 667 956	36 3191 151 17 10 23 9	17 63 12	F F B	B F	1,7 1,187 50 225 1,4 107 539	1,236 225 62 646 927	192 141 14 14 26	14 63 13	F B C B	В	70 104 6 6 867	104 66 873 595	2 6 11 12	6 3 12 4	A A A -	A B A	68 105 6 1 864	866 595	2 6 14 11 3 1	6 3 11 3	A A - B	B A
	NE at Foote Street NE Minnesota Avenue NE at DOES Parking	SB EB NB	RT Intersection RT RT RT Intersection LT TH RT RT	10 1,277 53 237 105 561 946 9	237	36 191 151 17 23 9 18	17 63 12	F F B C A B	B F	1,7 1,187 50 225 1,4 107 539 917	1,236 225 62 646	192 141 14 26 11 12 5	14 63 13	F B C B A	В	70 104 6 6 867 565 31	104 66 873	2 6 11 12 4 2	6	A A A B A A	A A B	68 105 6 1 864 562 33	866	2 6 14 11 3	6 3 11 3	A A A A A A	В

<sup>\*</sup>Simulated level of service is approximated based on delay but is not equivalent to that produced using Highway Capacity Manual methodology.

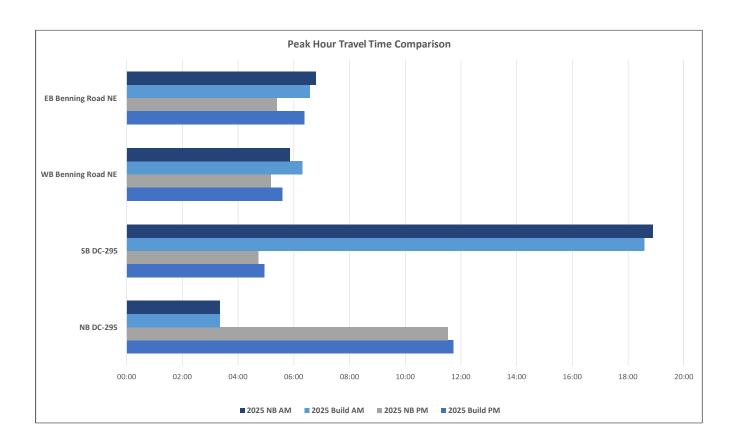
<sup>\*</sup>The intersections are numbered based on their inclusion in both the Streetcar and IMR projects (100s), the Streetcar project (200s), and the IMR project (300s).

<sup>\*</sup>MOEs represent an average of 10 simulation runs.

#### 7.1.1.3 Travel Time Comparison

# 7.1.1.3.1 AM Peak Period (7:00 AM - 9:00 AM); PM Peak Period (4:30 PM - 6:30 PM)

Route	2025 NB AM	2025 Build AM	2025 NB PM	2025 Build PM
NB DC-295 from Nelson Place NE to East Capitol Street Bridge	01:15	01:15	03:51	02:20
NB DC-295 from East Capitol Street to Benning Road NE	00:37	00:37	02:21	02:20
NB DC-295 from Benning Road NE to Nannie Helen Burroughs Ave NE	00:44	00:44	02:30	02:28
NB DC-295 from Nannie Helen Burroughs Ave NE to Polk Street NE	00:45	00:45	02:50	02:50
NB DC-295	03:21	03:21	11:32	11:44
SB DC-295 from Polk Street NE to Lane Place NE	03:11	03:12	00:31	00:31
SB DC-295 from Lane Place NE to Nannie Helen Burroughs Ave NE	01:29	01:30	00:13	00:13
SB DC-295 from Nannie Helen Burroughs Ave NE to Benning Road NE	06:38	06:25	00:45	00:45
SB DC-295 from Benning Road NE to East Capitol Street	03:44	03:37	00:53	00:59
SB DC-295 from East Capitol Street NE to N Street NE	03:51	03:51	02:22	02:29
SB DC-295	18:53	18:35	04:44	04:57
WB Benning Road NE from East Capitol Street to Minnesota Ave NE	03:04	03:05	02:49	02:51
WB Benning Road NE from Minnesota Ave NE to 36th Street NE	00:42	00:41	00:39	00:39
WB Benning Road NE from 36th Street NE to Anacostia Ave NE	00:47	01:02	00:40	00:51
WB Benning Road NE from Anacostia Ave NE to 26th Street NE	01:19	01:30	01:03	02:36
WB Benning Road NE	05:52	06:18	05:11	05:36
EB Benning Road NE from 26th Street NE to Anacostia Ave NE	01:09	01:21	01:06	01:04
EB Benning Road NE from Anacostia Ave NE to 36th Street NE	00:54	01:03	00:45	00:57
EB Benning Road NE from 36th Street NE to Minnesota Ave NE	00:50	00:48	00:49	02:12
EB Benning Road NE from Minnesota Ave NE to East Capitol Street	03:55	03:23	02:44	02:55
EB Benning Road NE	06:48	06:35	05:24	06:23

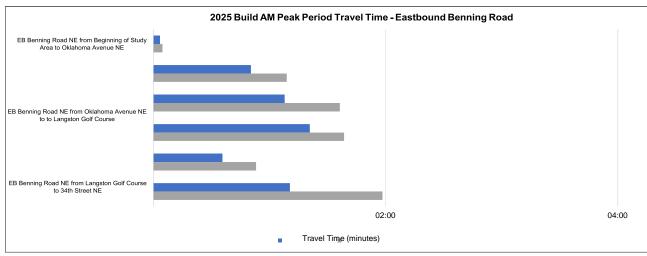


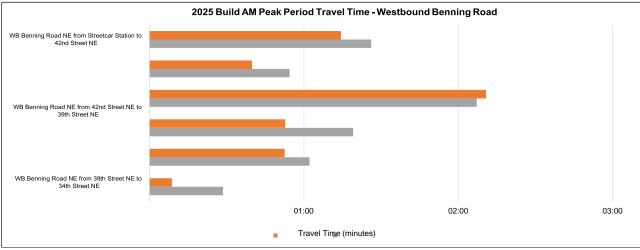
#### 7.1.1.4 2025 Build Streetcar Travel Time Comparison

#### 7.1.1.4.1 AM Peak Period (7:00 AM - 9:00 AM); PM Peak Period (4:30 PM - 6:30 PM)

Streetcar Travel Time Results			
Route	Vehicular Travel Time (MM:SS)	Streetcar Travel Time (MM:SS)*	Difference (MM:SS)
WB Benning Road NE from Streetcar Station to 42nd Street NE	01:14	01:26	00:12
WB Benning Road NE from 42nd Street NE to 39th Street NE	00:40	00:54	00:15
WB Benning Road NE from 39th Street NE to 34th Street NE	02:11	02:07	-00:04
WB Benning Road NE from 34th Street NE to Langston Golf Course	00:53	01:19	00:26
WB Benning Road NE from Langston Golf Course to Oklahoma Avenue NE	00:53	01:02	00:10
WB Benning Road NE from Oklahoma Avenue NE to End of Study Area	00:09	00:29	00:20
WB Benning Road NE	05:59	07:18	01:19
EB Benning Road NE from Beginning of Study Area to Oklahoma Avenue NE	00:04	00:05	00:01
EB Benning Road NE from Oklahoma Avenue NE to Langston Golf Course	00:51	01:09	00:18
EB Benning Road NE from Langston Golf Course to 34th Street NE	01:08	01:36	00:29
EB Benning Road NE from 34th Street NE to 39th Street NE	01:21	01:39	00:18
EB Benning Road NE from 39th Street NE to 42nd Street NE	00:36	00:53	00:17
EB Benning Road NE from 42nd Street NE to Streetcar Station	01:11	01:58	00:48
EB Benning Road NE	05:09	07:20	02:11

<sup>\*</sup>Streetcar travel time does not include dwell time at stops (assuming 20 seconds per stop, on average)



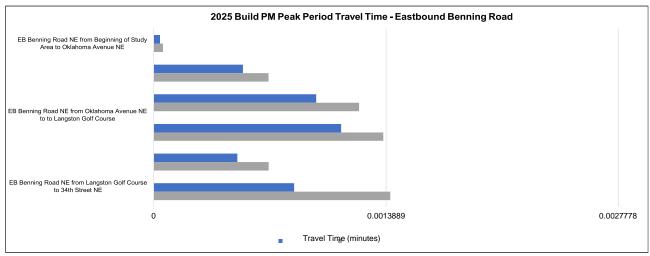


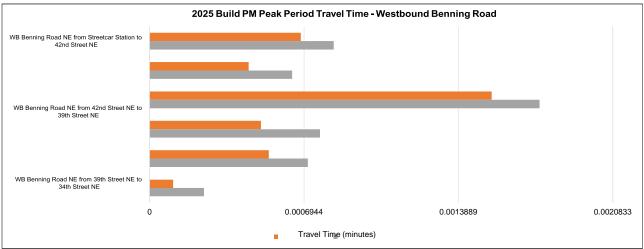
#### 7.1.1.5 2025 Build Streetcar Travel Time Comparison

#### 7.1.1.5.1 AM Peak Period (7:00 AM - 9:00 AM); PM Peak Period (4:30 PM - 6:30 PM)

Streetcar Travel Time Results			
Route	Vehicular Travel Time (MM:SS)	Streetcar Travel Time (MM:SS)*	Difference (MM:SS)
WB Benning Road NE from Streetcar Station to 42nd Street NE	00:59	01:12	00:13
WB Benning Road NE from 42nd Street NE to 39th Street NE	00:39	00:55	00:17
WB Benning Road NE from 39th Street NE to 34th Street NE	02:13	02:32	00:19
WB Benning Road NE from 34th Street NE to Langston Golf Course	00:43	01:06	00:23
WB Benning Road NE from Langston Golf Course to Oklahoma Avenue NE	00:46	01:01	00:15
WB Benning Road NE from Oklahoma Avenue NE to End of Study Area	00:09	00:21	00:12
WB Benning Road NE	05:29	07:07	01:38
EB Benning Road NE from Beginning of Study Area to Oklahoma Avenue NE	00:03	00:05	00:02
EB Benning Road NE from Oklahoma Avenue NE to Langston Golf Course	00:46	00:59	00:13
EB Benning Road NE from Langston Golf Course to 34th Street NE	01:24	01:46	00:22
EB Benning Road NE from 34th Street NE to 39th Street NE	01:37	01:59	00:22
EB Benning Road NE from 39th Street NE to 42nd Street NE	00:43	00:59	00:16
EB Benning Road NE from 42nd Street NE to Streetcar Station	01:13	02:02	00:50
EB Benning Road NE	05:46	07:51	02:05

\*Streetcar travel time does not include dwell time at stops (assuming 20 seconds per stop, on average)





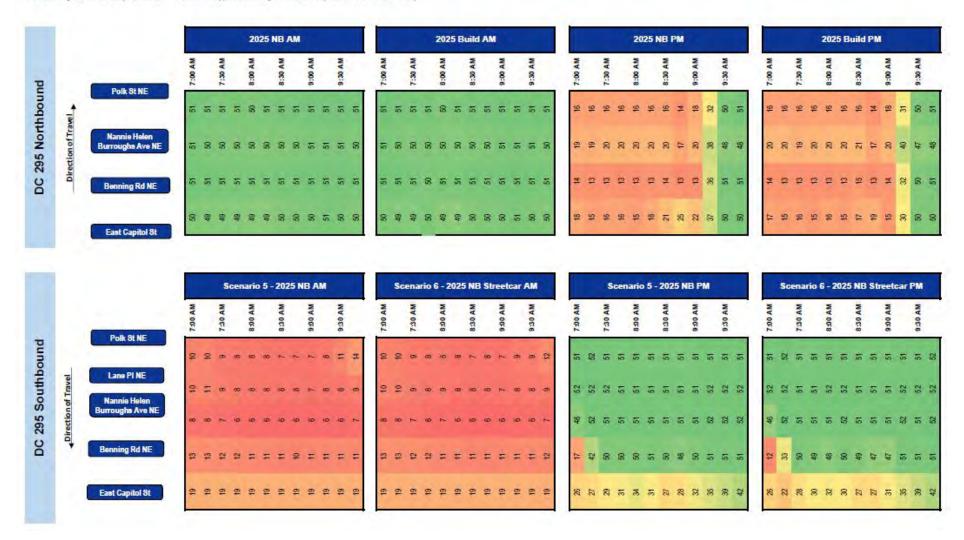
# 7.1.1.6 Queue Length Comparison

# 7.1.1.6.1 AM Peak Period (7:00 AM - 9:00 AM); PM Peak Period (4:30 PM - 6:30 PM)

			Vissim Max	Queue (feet)	
	Location	2025 NB AM	2025 Build AM	2025 NB PM	2025 Build PM
	Eastbound Benning Road NE to Southbound DC-295	0	0	25	20
	Eastbound Benning Road NE to Northbound DC-295	11	3	1411	1627
DC 295 and Benning Road NE Interchange	Southbound Kenilworth Avenue NE to Northbound DC-295	16	14	0	0
	Northbound DC-295 to Westbound Benning Road NE	375	308	0	0
	Southbound Kenilworth Avenue NE to Westbound Benning Road NE	0	444	0	93
	Southbound Benning Road NE at East Capitol Street	533	410	492	402
Benning Road NE and East Capitol Street	Northbound Benning Road NE at East Capitol Street	417	438	244	235
	Westbound East Capitol Street at Benning Road NE	1775	1781	229	219
	Eastbound East Capitol Street at Benning Road NE	220	232	1332	1335
East Capitol Street and Texas Ave	Northbound Texas Avenue at East Capitol Street	432	450	207	232
	Southbound Texas Avenue at East Capitol Street (left turn from East Capitol to Texas)	74	95	253	156
	Eastbound Benning Road NE at Minnesota Avenue NE	372	270	560	673
	Westbound Benning Road NE at Minnesota Avenue NE	378	412	284	285
Minnesota Avenue NE	Northbound Minnesota Avenue NE at Benning Road NE	460	353	285	442
	Southbound Minnesota Avenue NE at Benning Road NE	530	533	447	445

#### Speed Heat Map Comparison

AM Analysis Period (7:00 AM - 10:00 AM); PM Analysis Period (4:30 PM - 7:30 PM)



# 7.1.1.6.2 2025 Build Curbside Alignment Morning Peak Hour Intersection Delay and Levels of Service (LOS)

Intersection	Traffic	Inters	ection	Northb Appr		Southb Appro		Westh Appr			oound roach
	Control	Delay	LOS	Delay	LOS	Delay	LOS	Delay	LOS	Delay	LOS
Benning Road and 26th Street	Signalized	7.5	A	-	-	28.5	С	6.0	A	9.3	A
Benning Road and Oklahoma Avenue	Signalized	25.0	С	21.9	С	-	-	29.5	С	12.1	В
Benning Road and Anacostia Avenue	Signalized	7.2	A	32.8	С	32.9	С	3.7	A	12.5	В
Benning Road and 34th Street	Signalized	12.5	В	16.8	В	35.2	D	12.1	В	10.1	В
Benning Road and Minnesota Avenue	Signalized	54.2	D	44.0	D	72.2	Е	61.0	Е	37.4	D
Benning Road and 42 <sup>nd</sup> Street	Signalized	11.4	В	38.6	D	34.5	С	7.7	A	6.2	A
Benning Road and 45 <sup>nd</sup> Street	Signalized	8.0	A	5.8	A	10.5	В	21.3	C	26.0	С
Benning Road and Central Avenue*	Unsignalized	11.1	В	-	-	-	-	11.1	В	-	-
Benning Road and E Capitol Street	Signalized	180.5	F	284.5	F	62.2	Е	127.2	F	386.2	F
Minnesota Avenue and Dix Street	Signalized	9.1	A	7.5	A	8.9	A	20.3	С	18.0	В
Minnesota Avenue and Grant Street	Signalized	15.5	В	13.8	В	10.9	В	39.0	D	-	-
Minnesota Avenue and Gault Place*	Unsignalized	20.7	С	0.5	A	1.6	A	20.7	С	14.2	В
Minnesota Avenue and Hayes Street*	Unsignalized	15.4	C	1.7	A	0.9	A	15.4	C	-	-
Minnesota Avenue and Nannie Helen Burroughs Avenue	Signalized	30.7	С	37.9	D	29.1	С	27.6	С	29.6	С
Benning Road and 44th Street	Signalized	17.1	В	15.9	В	11.6	В	27.7	С	-	-

<sup>\*</sup> Indicates unsignalized intersections, for which intersection LOS is expressed in terms of the average vehicle delay of the worst approach delay Note: Benning Road is considered east-west at all intersections except for at 44th Street, 45th Street, and E Capitol Street where it is considered to be north-south.

# 7.1.1.6.3 2025 Build Curbside Alignment Morning Peak Hour Maximum Queue Length (feet)

Intersection	No	rthbou	nd	So	uthbou	nd	W	estbour	nd	E	astbour	nd
Intersection	L	T	R	L	T	R	L	T	R	L	T	R
Benning Road and 26 <sup>th</sup> Street	-	-	-	124	-	151	-	302	285	90	127	-
Benning Road and Oklahoma Avenue	115	-	98	-	-	-	740	740	-	-	209	214
Benning Road and Anacostia Avenue	186	-	176	65	-	ı	281	281	281	173	173	159
Benning Road and 34 <sup>th</sup> Street	155	155	163	155	155	127	295	384	384	40	217	192
Benning Road and Minnesota Avenue	364	207	207	785	785	921	-	581	581	304	283	283
Benning Road and 42 <sup>nd</sup> Street	118	118	133	198	198	213	241	241	255	149	149	180
Benning Road and 45 <sup>nd</sup> Street	246	246	246	163	163	163	70	70	70	38	38	38
Benning Road and Central Avenue	-	-	-	-	-	-	-	-	257	-	-	-
Benning Road and E Capitol Street	1072	1072	1072	320	320	330	1674	1674	1674	1463	1463	1463
Minnesota Avenue and Dix Street	191	191	191	169	169	169	121	121	121	22	22	22
Minnesota Avenue and Grant Street	94	227	227	203	203	203	245	-	245	ı	-	-
Minnesota Avenue and Gault Place	-	47	47	117	112	-	53	-	49	104	102	103
Minnesota Avenue and Hayes Street	71	67	67	8	3	3	43	24	47	1	1	-
Minnesota Avenue and Nannie Helen Burroughs Avenue	466	466	466	-	274	292	230	230	251	401	400	401
Benning Road and 44 <sup>th</sup> Street	-	392	392	148	148	-	295	-	294	-	1	-

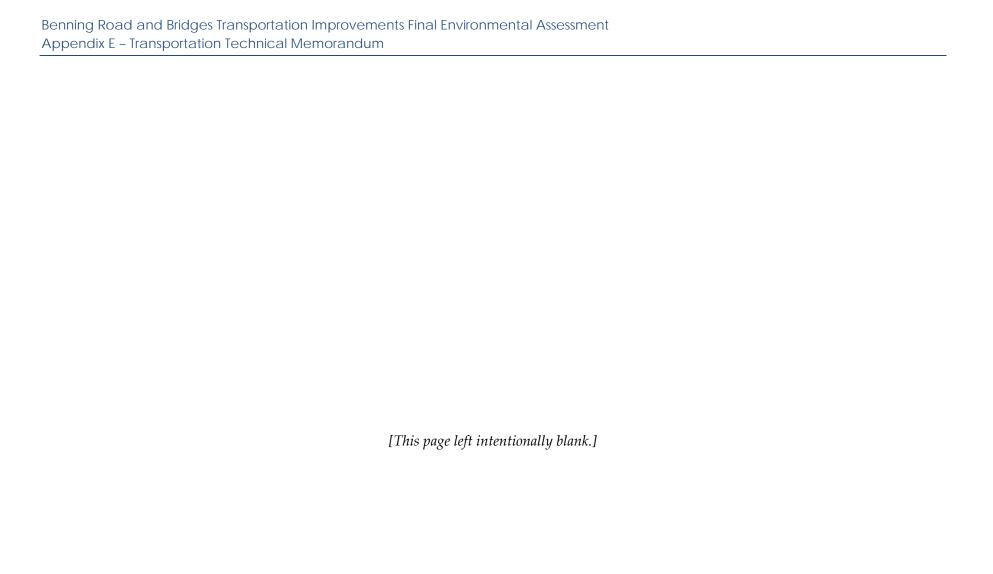
# 7.1.1.6.4 2025 Build Curbside Alignment Evening Peak Hour Intersection Delay and Levels of Service (LOS)

	Tueffie	Interse	ation	North	oound	Southb	ound	Westb	ound	Easth	ound
Intersection	Traffic Control	Interse	Cuon	Appr	oach	Appro	oach	Appr	oach	Appı	roach
	Control	Delay	LOS	Delay	LOS	Delay	LOS	Delay	LOS	Delay	LOS
Benning Road and 26 <sup>th</sup> Street	Signalized	10.5	В	-	-	40.5	D	3.7	A	11.3	В
Benning Road and Oklahoma Avenue	Signalized	7.5	A	21.4	C	1	-	9.3	A	5.8	A
Benning Road and Anacostia Avenue	Signalized	4.5	A	32.2	C	44.6	D	2.9	A	4.3	A
Benning Road and 34 <sup>th</sup> Street	Signalized	19.4	В	18.1	В	36.8	D	12.3	В	22.3	C
Benning Road and Minnesota Avenue	Signalized	41.9	D	36.7	D	45.8	D	52.4	D	40.9	D
Benning Road and 42 <sup>nd</sup> Street	Signalized	19.1	В	35.3	D	33.3	C	5.4	A	21.8	C
Benning Road and 45 <sup>nd</sup> Street	Signalized	54.8	D	3.0	A	99.2	F	28.0	C	34.4	C
Benning Road and Central Avenue*	Unsignalized	4.2	A	-	-	-	-	4.2	A	1	-
Benning Road and E Capitol Street	Signalized	165.9	F	286.8	F	69.1	Е	251.0	F	148.2	F
Minnesota Avenue and Dix Street	Signalized	11.5	В	8.7	A	11.6	В	22.1	C	19.1	В
Minnesota Avenue and Grant Street	Signalized	14.9	В	16.9	В	8.0	A	30.0	C	1	-
Minnesota Avenue and Gault Place*	Unsignalized	12.4	В	1.0	A	1.2	A	11.6	В	12.4	В
Minnesota Avenue and Hayes Street*	Unsignalized	19.9	C	1.9	A	2.5	A	19.9	C	1	-
Minnesota Avenue and Nannie Helen Burroughs Avenue	Signalized	32.6	С	39.5	D	27.8	С	27.1	С	31.7	С
Benning Road and 44 <sup>th</sup> Street	Signalized	60.0	Е	18.4	В	99.4	F	34.7	С	ı	-

<sup>\*</sup> Indicates unsignalized intersections, for which intersection LOS is expressed in terms of the average vehicle delay of the worst approach delay Note: Benning Road is considered east-west at all intersections except for at 44th Street, 45th Street, and E Capitol Street where it is considered to be north-south.

# 7.1.1.6.5 2025 Build Curbside Alignment Evening Peak Hour Maximum Queue Length (feet)

Intongostica	N	orthbour	ıd	So	uthbou	nd	V	Vestboun	d		Eastbound	
Intersection	L	T	R	L	T	R	L	T	R	L	T	R
Benning Road and 26 <sup>th</sup> Street	-	-	-	191	-	218	-	157	140	41	285	-
Benning Road and Oklahoma Avenue	102	-	85	-	-	-	122	122	-	-	287	290
Benning Road and Anacostia Avenue	114	-	104	11	-	-	105	105	105	273	273	260
Benning Road and 34 <sup>th</sup> Street	116	116	124	78	78	29	210	148	148	8	566	542
Benning Road and Minnesota Avenue	194	273	273	446	446	285	-	291	291	833	982	982
Benning Road and 42 <sup>nd</sup> Street	135	135	151	168	168	183	140	140	154	592	592	626
Benning Road and 45 <sup>nd</sup> Street	136	136	136	469	469	469	105	57	85	29	29	29
Benning Road and Central Avenue	-	-	-	1	-	-	-	-	68	-	1	-
Benning Road and E Capitol Street	1,017	1,017	1,017	363	363	373	1,032	1,032	1,032	1,466	1,466	1,466
Minnesota Avenue and Dix Street	171	171	171	255	255	255	154	154	154	90	90	90
Minnesota Avenue and Grant Street	86	607	607	141	141	141	104	-	104	-	-	-
Minnesota Avenue and Gault Place	-	97	97	97	95	-	55	-	51	74	97	69
Minnesota Avenue and Hayes Street	131	126	126	31	9	9	40	21	44	-	-	-
Minnesota Avenue and Nannie Helen Burroughs Avenue	435	435	435	-	192	210	183	183	203	582	582	582
Benning Road and 44 <sup>th</sup> Street	-	234	234	830	830	-	254	-	255	-	-	-



# 8.0 Attachment C - Design Year 2045 Results

### 8.1.1.1 Freeway and Ramp MOEs Comparison

#### 8.1.1.1.1 AM Peak Hour (7:45 AM - 8:45 AM) PM Peak Hour (4:30 PM - 5:30 PM)

			20-	45 No Build Al	VI	2	2045 Build AM		20	45 No Build PM	M	2	045 Build PM	
Facility	Segment	Туре	VISSIM Throughput (vph)	Average Speed (mph)	Average Density (vpmpl)									
	Mainline south of EB Capitol Street On-Ramp	Basic	2,895	48	30	2,895	48	30	2,900	13	112	2,900	14	103
	Between EB Capitol Street On-Ramp and WB Benning Road Off- Ramp	Merge/Diverge	3,260	50	21	3,260	51	21	3,825	11	111	3,865	13	100
NB DC-295	Between WB Benning Road Off-Ramp and EB Benning Road/Kenilworth Avenue On-Ramp	Basic	2,990	51	20	2,990	51	20	3,720	10	121	3,760	12	109
	Between EB Benning Road/Kenilworth Avenue On-Ramp and Nannie Helen Burroughs Avenue Off-Ramp	Merge/Diverge	3,475	48	23	3,475	48	23	4,700	17	87	4,705	20	77
	Between Nannie Helen Burroughs Avenue Off-Ramp and Kenilworth Avenue On-Ramp	Basic	2,795	52	18	2,795	52	18	3,710	41	38	3,710	33	54
	Mainline north of Nannie Helen Burroughs Avenue	Merge/Basic	3,330	43	21	3,330	44	21	4,015	34	42	4,015	23	59
	Mainline North of Deane Avenue Off-Ramp	Merge/Diverge	3,290	8	115	3,290	8	114	3,405	51	20	3,405	51	21
	Between Deane Avenue Off-Ramp and Kenilworth Avenue Off-Ramp	Basic/Diverge	3,000	7	135	3,000	7	134	3,015	51	19	3,015	51	19
	Between Kenilworth Avenue Off-Ramp and Kenilworth Avenue On-Ramp	Basic	2,190	4	169	2,250	4	165	2,785	52	18	2,805	52	18
SB DC-295	Between Kenilworth Avenue On-Ramp and EB Benning Road On-Ramp	Merge/Basic	2,930	6	147	3,050	6	142	3,060	51	19	3,100	51	20
	Between EB Benning Road On-Ramp and Baker Street Off-Ramp	Weave	3,245	7	103	3,365	8	101	3,390	51	17	3,410	51	17
	Between Baker Street Off-Ramp and WB Capitol Street Off-Ramp	Diverge	3,205	11	93	3,265	11	93	3,325	49	23	3,345	49	23
	Between WB Capitol Street Off-Ramp and EB Capitol Street On-Ramp	Basic	2,690	12	106	2,690	12	106	2,925	44	35	2,925	43	36
	Mainline south of EB Capitol Street On-Ramp	Merge/Basic	3,145	15	91	3,145	15	90	3,080	33	49	3,080	33	51

			20	45 No Build Al	М	2	045 Build AM		20	45 No Build Pl	М	2	045 Build PM	
Facility	Segment	Туре	VISSIM Throughput (vph)	Average Speed (mph)	Average Density (vpmpl)									
	EB Capitol Street to SB DC-295	Ramp	449	24	20	449	24	20	159	29	6	162	29	6
East Capitol Street	EB Capitol Street to NB DC-295	Ramp	367	32	12	366	32	12	913	25	39	955	26	40
	SB DC-295 to WB Capitol Street	Ramp	486	40	13	537	40	14	407	43	10	421	43	10
Baker Street	SB DC-295 to WB Baker Street	Ramp	38	20	2	100	20	5	63	20	3	65	20	3
Baker Street	EB Benning Road to SB DC-295	Ramp	307	36	9	304	36	8	296	35	8	284	36	8
	EB Benning Road to NB DC-295	Ramp	417	20	36	417	23	23	880	7	138	852	8	121
	NB DC-295 to WB Benning Road	Ramp	272	29	16	273	31	11	89	26	5	95	25	7
Benning Road	NB DC-295/EB Benning Road (U-turns) to WB Benning Road	Ramp	282	3	120	288	5	69	90	13	9	94	12	10
	SB Kenilworth Avenue NE to NB DC-295	Ramp	63	22	3	62	23	3	61	14	8	61	13	9
	EB Benning Road/SB Kenilworth Avenue NE to NB DC-295	Ramp	471	37	13	465	37	12	940	8	121	917	8	116
	SB Kenilworth Avenue to SB DC-295 (South)	Ramp	634	4	145	660	5	140	266	25	10	284	25	11
Kenilworth Avenue	SB DC-295 to SB Kenilworth Avenue	Ramp	757	12	77	712	11	73	239	39	6	216	39	6
	NB Kenilworth Avenue to NB DC-295	Ramp	536	24	22	539	24	22	233	24	10	295	20	17
Nannie Helen	NB DC-295 to Nannie Helen Burroughs Avenue/Kenilworth Avenue	Ramp	678	40	22	663	40	24	923	8	139	987	14	106
Burroughs Avenue	SB DC-295 to Deane Avenue/Kenilworth Terrace/Kenilworth Avenue	Ramp	274	28	23	292	29	18	386	39	12	385	32	26

# 8.1.1.2 Intersection MOE Comparison

# 8.1.1.2.1 AM Peak Hour (7:45 AM - 8:45 AM) PM Peak Hour (4:30 PM - 5:30 PM)

					2	045 No Bı	uild AM					2045 Bui	ld AM				2	2045 No Bu	uild PM					2045 Buil	ld PM		
No.	Intersection ID	Approach	Movement		hroughput ph)	Average (sec/		LC	os*		nroughput ph)	Averag		LC	os*		hroughput ph)	Average (sec/		LO	S*		roughput oh)	Average (sec/		LOS	S*
		NB	LT RT	120 79	199	54 15	39	D B	D	184 75	259	60 23	49	E C	D	42 31	73	57 9	37	E A	D	43 30	73	59 9	38	E A	D
		SB	LT	24	32	49	49	D	D	25	32	44	45	D	D	0	0	-		-	-	0	0	-	-	-	
	Benning Road NE		RT LT	8 15		48 22		D C		8 14		46 40		D D		6		- 28		-		5		32		-	
101	at Anacostia	EB	TH	1,068	1,116	3	4	Α	Α	1,047	1,097	7	7	Α	Α	2,235	2,344	32	32	С	С	2,141	2,250	36	36	D	D
	Avenue NE		RT LT	34 27		7 13		A B		36 24		10 19		A B		104 5		28 24		C		104 5		37 40		D D	
		WB	TH	2,790	2,836	4	4	Α	Α	2,582	2,624	11	11	В	В	1,208	1,220	1	1	Α	Α	1,170	1,182	5	5	Α	Α
			RT Intersection	19	183	7	ì	Α	Δ	18	112	15	3	В	R	8 3 6	637	3 2:	2	-	,	6	504	4 2	6	- (	
			LT	20	1	36		D	<u> </u>	19		3		A		35		40		D		30		39		D	
		NB	RT	62	84	10	17	В	В	60	83	5	6	Α	Α	84	119	15	22	В	С	84	113	15	21	В	С
		SB	LT RT	38 41	78	34 36	35	C	С	37 36	72	34 36	35	C D	D	51 11	64	35 41	36	D D	D	51 12	64	35 40	36	D D	D
			LT	9		24		C		5		33		C		0		-		-		0		-		-	
102	Benning Road NE	EB	TH	1,137	1,166	27	27	С	С	1,116	1,141	29	29	С	С	2,206	2,268	32	32	С	С	2,100	2,161	42	42	D	D
102	at 34th Street NE		RT	20		19		В		19		22		С		61		25		С		62		50		D	
			LT TH	279 2,779	4	28 16		C B		282 2,566	ł	25 20		C B	ł	260 1,174		50 9		D A		264 1,131		43 11	ŀ	D B	
		WB	RT	56	3,178	21	17	C	В	59	2,969	22	21	C	С	29	1,468	9	16	A	В	29	1,429	8	17	A	В
			U	63		30		С		61		25		С		5		36		D		6		43		D	
		NB	Intersection	- 1	443	2	0		C	4,2	-00	2	23		C	3,9	914	2000	•	F	;	3,7	761	3:	2	C	
	Benning Road NE		RT TH	157 570	157	3	3	A	Α	157 567	157	7	2	A A	A	244 925	244	280 104	280	F	г	301 870	301	19 64	19	C F	С
103	Ramp to DC-295 at 36th Street NE	EB	RT	3	573	2	4	Α	Α	3	570	2	7	A	А	0	925	-	104	-	F	0	870	-	64	-	F
	00 0001.112		Intersection LT	305	30	67	1	E	A I	258	27 I	47	6	D	A I	1,1 188	169	59 59	11	E		1,1 185	71	44 44	3	D F	
		NB	TH	442	782	24	41	C	D	434	721	16	27	В	С	555	819	19	28	В	С	560	819	28	32	C	С
			RT	35		20		В		29		14		В	-	76		24		С		74	***	27		C	
			LT TH	60 528		58		E D	_	60 534		54		D D	D	70 528		68		E	_	70 523		56		E D	_
	Benning Road NE	SB	RT	404	992	49 54	51	D	D	366	960	39 43	41	D	D	214	813	35 25	35	C	С	207	799	45 28	42	C	D
104	at Minnesota Avenue NE		LT	207		47		D		200		39		D		254		18		В		233		28		C	
	Avenue NE	EB	TH RT	304	718	17	27	В	С	295	694	11	20	В	В	859	1,415	14	15	В	В	838	1,382	22	24	С	С
			TH	207 769	<b> </b>	20 41		C D	<u> </u>	199 727	<b> </b>	14 38		B D	<u> </u>	303 338		17 38		B D		312 335		27 44		C D	
		WB	RT	58	41	44	136	D	D	64	39	52	123	D	D	129	41	46	72	D	D	124	48	61	85	E	D
			Intersection		318	4	1		Ď	3,	165	3	3	_	Ċ	-,	514	20	6	Ċ	)	3,4	159	3	3	C	
		NB	LT RT	22 82	105	68 69	69	E	E	10 78	89	62 59	60	E	E	0 165	166	57	57	- E	Е	18 162	180	60 56	57	E E	E
		ep.	LT	3	8	51	56	D	Е	3		60	50	E	Е	3	_	43	41	D	D	3	0	39	40	D	_
		SB	RT	0	8	-	56		E	3	8	65	59	E	E	0	4	-	41	-	ט	4	8	42	40	D	D
105	Benning Road NE at 39th Street	EB	LT TH	11 367	399	15 5	5	B A	Α	10 353	383	14	3	B A	А	11 940	951	10	3	A	Α	12 922	984	13 9	9	B A	Α
105	NE/Driveway	EB	RT	21	355	6	5	Ä	l ^	21	303	5	٦	A	1 ^	0	301	-	J	-	^	50	304	11	3	В	^
			LT	137		12		В		136		10		A		122		23		С		126		16		В	
		WB	TH RT	799 10	945	12 10	12	B B	В	780 11	927	9	9	A A	Α	9	131	9	22	- A	С	427 11	564	6	9	A A	Α
			Intersection		457	10	4		I B		1 107	, 1	1	Α	B _		252	1:	2	АВ	3	1,7	37	1.	4	B	
										-1						-,-						.,.					

					20	045 No B	uild AM					2045 Buil	ld AM				2	045 No Bui	ld PM					2045 Buil	ld PM		
No.	Intersection ID	Approach	Movement		hroughput ph)	Averag (sec	e Delay /veh)	LO	os*	Vissim Th (v)	roughput oh)	Average (sec/		LO	S*		nroughput ph)	Average (sec/v		LO	S*		nroughput ph)	Average (sec/		LOS	S*
		ND	LT	4	88	11	6	В	А	4	84	10	6	В	А	59	000	28	00	D	С	61	000	25	00	D	0
		NB	RT	84	88	6	ь	Α	A	80	84	6	ь	Α	А	167	226	19	22	С	C	160	222	18	20	С	С
	Benning Road NE	EB	TH	421	453	1	1	Α	Α	403	434	1	1	Α	Α	1,035	1,105	1	1	Α	Α	1,019	1,086	2	2	Α	Α
106	at 40th Street NE		RT	33	100	1		Α		31	101	1		Α	,,	70	1,100	2		A	,,	67	1,000	3		Α	
		WB	LT	191 940	1,131	5	3	A	Α	184	1,105	4	2	A	Α	128	649	12	4	В	Α	126	631	12	4	В	Α
			TH Intersection	940	672	2		Α		921		2		Α		521	200	3		Α		505	20	2		Α	
$\longrightarrow$			LT	4	0/3	11	2	В	M.	4	123	12	2	В	`	3	900	16		С	`	3	330	20		C	l .
		NB	RT	29	33	6	6	A	Α	29	33	6	7	A	Α	111	114	10	10	A	В	111	114	11	11	В	В
	-		TH	489		0		A		471		0		A		1,195		1		A		1,168		2		A	
107	Benning Road NE	EB	RT	15	504	1	0	Α	A	13	483	1	0	Α	Α	3	1,198	1	1	Α	Α	4	1,172	2	2	Α	Α
	at 41st Street NE	WB	LT	138	4.000	5		Α	Α	136	4.007	5	3	Α		40	000	12		В		41	669	10	2	В	
		WB	TH	1,128	1,266	3	3	Α	A	1,101	1,237	3	3	Α	Α	646	686	1	2	Α	Α	628	669	1	2	Α	Α
		i	ntersection	1,	804		2	Ā	À	1,7	'54	2	2	A	١	1,9	98	2		À	١	1,9	955	2	2	Ä	l .
			LT	11		20		С		5		20		С		31		31		С		34		32		С	
		NB	TH	65	99	21	18	С	В	71	99	21	19	С	В	125	167	29	29	С	С	122	167	32	31	С	С
			RT LT	23		11		В		24		12		B F		11		24		С		11		20		С	
		SB	TH	139	475	111	103	F	F	136	459	120 118	113	F	F	24 104	312	48 49	40	D D	D	26 106	313	63 66	56	E	Е
		35	RT	327	4/5	101	103	F	-	316	459	110	113	F	г	183	312	34	40	C	D	181	313	49	56	D	_
108	Benning Road NE		LT	119		23		C		120		18		В.		478		29		C		475		32		C	
100	at 42nd Street NE		TH	352	513	12	14	В	В	333	498	5	8	A	Α	763	1,304	16	21	В	С	733	1,274	16	22	В	С
			RT	42		7		Α	1	45		9		Α		63		12	ľ	В		66		12	l	В	
			LT	4		16		В		4		18		В		5		24		С		4		11		В	
		WB	TH	888	938	15	15	В	В	869	921	14	14	В	В	472	504	13	13	В	В	455	487	9	9	Α	Α
			RT	47		15		В		48		14		В		27		11		В		28		9		Α	
		!	ntersection		025	)	6		D	1,9	977	31	6		)	2,2	287	22			)	2,2	241	2	5	С	;
		SB	LT RT	191	254	78	75	E	Е	191	253	80	77	E	E	169	187	49	48	D	D	178	197	49	48	D	D
	-		LT	62 86		66 47		E D		62 86		68 54		E D		18 10		37 15		D B		19 9		35 13		C B	
208	Benning Road NE	EB	TH	970	1,056	11	14	В	В	949	1,035	10	13	A	В	2,146	2,155	8	8	A	Α	2,102	2,111	8	8	A	Α
200	at 26th Street NE		TH	1,887		8		A		1,805		9		A		1,077		4		A		1,052		4		A	
		WB	RT	492	2,379	16	10	В	Α	508	2,313	17	11	В	В	148	1,225	6	4	Α	Α	154	1,207	6	4	Α	Α
		i	ntersection	3,	688	1	6	Ē	В	3,6	01	10	6	É	3	3,	67	9		À	١	3,5	515	ģ	)	Á	l .
		NB	LT	89	147	63	39	E	D	91	148	60	38	E	D	39	124	53	21	D	С	58	84	54	39	D	D
		ND	RT	59	147	4	33	Α		57	140	4	30	Α		84	12.4	6	21	Α		26	07	5	55	Α	
	Benning Road NE	EB	TH	1,071	1,163	12	13	В	В	1,052	1,144	11	11	В	В	2,268	2,330	2	2	A	Α	2,236	2,295	1	1	A	Α
209	at Oklahoma Avenue NE		RT LT	92 537	ļ ,	19		B E		92		17 58		В		61	,,,,,	3		A		59	,	1		A C	
	Avenue NE	WB	TH	2,298	2,835	73 76	75	E	E	492 2,229	2,721	55	56	E	E	73 1.184	1,257	29	5	C A	Α	60 1,149	1,209	26 6	7	A	Α
	_		ntersection	4.	146	. 5	6		E	4.0	013	4:	2		)	3.7	710	3		, , ,	· ·	3.5	588	- 4	ı	, A	
			TH	645	l	40		D	_	588		11		В		677		27		С		682		28		С	
		NB	RT	120	41	49	125	D	D	111	11	11	27	В	В	154	27	28	80	С	С	157	29	29	87	С	С
			LT	46		25		С		52		4		Α		153		31		С		153		36		D	
		SB	TH	659	734	7	8	Α	Α	656	733	3	3	A	Α	629	833	17	20	В	В	630	836	15	19	В	В
			RT	30	<b> </b>	10		В		25		8		A		51		14		В		53		18		В	
210	Minnesota Avenue	EB	LT TH	27	109	115 99	105	F	F	28 30	113	102 96	99	F	_	36 33	114	37	35	D C	С	37 31	113	39 35	35	D C	D
	NE at Dix Street NE	EB	RT	53	109	104	105	F	「	55	113	100	99	F	г	45	114	35	35	C	C	46	113	33	35	C	U
	-		LT	118		219		F	1	122		197		F		260		161		F		253		164		F	
		WB	TH	20	243	207	213	F	F	19	243	191	195	F	F	84	447	159	158	F	F	85	438	165	164	F	F
			RT	106	L	207		F	<u> </u>	102		194		F		103	<u> </u>	152		F		100		163		F	
			ntersection		853	5	i4		D	1,7		3	8	C	)		240	51			)	2,2	241	5	2	D	)
		NB	TH	599	599	14	14	В	В	594	594	15	15	В	В	927	927	13	13	В	В	932	932	14	14	В	В
	Minnesota Avenue	SB	TH	959	959	10	10	Α	A	933	933	9	9	A	Α	636	636	5	5	A	Α	622	622	5	5	A	A
211	NE at Bus Exit	EB	LT	0	30	-	14	-	В	0	29	-	14	-	В	1	27	4	8	A	Α	11	48	50	30	D	С
الدوء	South		RT	30	500	14		В		29		14	4	В		26		8		Α	_	37	202	24		С	
			ntersection	1.	300	- 1	1		0	1,5	000	1				1,5	209	10		P		1,0	JUZ	- 1		В	

					20	045 No Bı	uild AM					2045 Bui	ild AM				2	:045 No Bu	uild PM					2045 Buil	ld PM	
No.	Intersection ID	Approach	Movement	Vissim Th	roughput oh)	Average (sec/	e Delay veh)	LO	S*	Vissim TI (v	roughput oh)	Averag (sec		LO	S*		nroughput ph)	Average (sec/	e Delay veh)	LO	S*	Vissim Th (vp		Average (sec/	e Delay veh)	LOS*
		NB	LT TH	0 507	572	31	31	- C	С	0 509	574	32	32	- C	С	735	897	14	15	- В	В	13 753	915	24 15	16	ВВВ
	Minnesota Avenue	5	RT	65	0.2	32	0.	С		65	0	32	- 02	С	ŭ	162	007	18	.0	В		149	0.0	18		В
212	NE at Grant Street	SB	LT TH	21 762	783	22 15	16	C B	В	21 742	764	20 14	15	C B	В	54 478	532	30 14	15	C B	В	54 471	525	29 14	15	C B B
	NE and Bus Entrance North		RT	0	700	-		-		0		-		-	Ü	0	002	-	.0	-		0	020	-	.0	-
		WB	LT RT	189 44	233	19 12	18	B	В	190 43	233	19 12	18	B B	В	158 42	199	18 11	17	B B	В	162 41	203	18 11	17	ВВВ
			Intersection	1,5	587	2	2		)	1,	571	2	21		)	1,6	629	1	5	В		1,6	44	16	6	В
		NB EB	RT TH	58 370	15	15	4	С	C	58	7	7	3	A	A	34	11	9	2	A	В	34	10	10	2	B B
213	Benning Road NE		LT	277	387	16 7	15	C A		359 272	366	5	-	A	A	797 32	800	11	-	A B	Α .	765 30	769	9		A A
	at Blaine Street NE	WB	TH	937	1,214	1	3	Α	Α	922	1,194	1	2	Α	Α	498	530	0	1	Α	Α	480	510	0	1	A A
			Intersection LT	139	559	79	6	E	1	122	518	153	2	F	4	135	370	43		D A		1,3 134		45		D D
		SB	RT	350	488	69	72	E	E	315	437	143	146	F	F	101	236	30	37	С	D	97	231	32	40	C D
214	Benning Road NE	EB	LT TH	104 321	425	148 29	58	F C	Е	106 311	416	25 12	15	C B	В	169 663	832	17 5	7	B A	Α	166 632	798	11	8	A A
214	at 44th Street NE	WB	TH	856	1,018	16	15	В	В	845	1,000	21	23	C	С	429	593	5	5	A	Α	413	571	5	6	A A
			RT Intersection	163		14	15	В	ь	155	1,000	35	23	С	C	164	595	5	5	A	М	158	5/1	8	2	A A
			LT	49		477	9	F	, F	58		357		F	-	12	001	16		В		14	99	47	2	D
		SB	RT	39	87	447	464	F	F	43	101	337	348	F	F	17	29	8	11	A	В	15	29	10	28	B C
215	Benning Road NE	EB	LT TH	36 428	464	78 70	71	E	E	33 402	435	136 80	85	F	F	15 782	797 0	8	8	A A	A 0	15 740	755 0	10 10	10	B A 0
213	at 45th Street NE	WB	TH	977	1.059	2	2	A	A	952	1.039	11	11	В	В	577	629	1	1	A	Δ	551	601	14	14	ВВВ
			RT Intersection	82	1,000	3 4		Α		87	75	10	3	В	,	52	023	2	'	A		51	001	18	2	В
		WB	TH	656	684	1		Α	^	639	662	6	6	Α	A	533	558	1	1	A	^	506	530	7	7	A A
	Benning RoadNE	EB	RT TH	28		0	1	A	A	23		0		A	A	25		0		A	A	24		0		A
216	at Central Avenue NE	SB	RT	388 399	388 399	97 12	97 12	F B	В	383 397	383 397	83 41	83 41	F E	E	672 96	672 96	28 4	28 4	D A	D A	630 97	630 97	23 9	23 9	C C
			Intersection	1,4	71	2	9		)	1,4	41	3	36	Ė		1,0	326	1	5	Ċ		1,2	57	15	5	B
		NB	LT TH	207 312	659	42 8	17	D A	В	215 301	654	47 6	19	D A	В	85 1,426	1,866	89 7	9	F A	Α	62 1,413	1,822	93	9	A A
			RT	140		2		Α		137		2		A		355	.,	2	-	A		347	.,	3		A
		SB	LT TH	14 1,261	1,283	211 215	215	F	F	13 1,294	1,317	263 209	209	F	F	52 651	759	60 33	34	E C	С	65 648	768	63 34	35	E D
	Benning Road NE	OB	RT	8	1,200	214	210	F		10	1,017	194	203	F		57	733	18	34	В	Ü	55	700	22	33	С
217	at East Capitol Street SE	EB	LT TH	84 304	472	29 20	19	C	В	72 308	455	31 20	19	C	В	119 388	791	22 11	10	C B	Α	123 387	753	24 14	12	ВВВ
	Street SE	EB	RT	84	4/2	3	19	A	ь	75	455	3	19	A	ь	283	791	3	10	A	М	244	755	4	12	A
		WD	LT	411 462	000	58	50	E		450 432	007	134	440	F	F	60 415	540	164		F	-	53	500	130		F
		WB	TH RT	15	888	49 44	53	D D	D	15	897	87 83	110	F	F	65	540	45 36	57	D D	E	411 65	529	46 40	54	D D
			Intersection	3,3	302	10	)4	F		3,	322	1	19	F		3,9	55	2	1	Ċ		3,8	72	21	1	Ċ
		NB	RT RT2	90	423	52	99	D	F	71	348	77	124	Е	F	38	153	82	80	F	П	31	145	116	87	F F
		110	LT	288	723	130	33	F		239	540	158	127	F		102	100	78	00	E		114	143	79	0,	E
218	East Capitol Street SE at Texas Avenue	EB	TH RT	354 14	368	33 21	33	C	С	536 10	546	23	23	C	С	1,555 69	1,624	113	112	F	F	1,552 98	1,650	112 107	111	F F
218	SE at Texas Avenue SE		U	0		-		-		0		-		-		0		106		-		0		107		-
		WB	LT TU	88	1,995	13	2	В	Α	92	2,050	14	2	В	Α	267	998	28	8	C	Α	231	947	25	6	C A
			TH Intersection	1,908	786	2 2	1	A	)	1,958	945	2	21	A		731	T75	0 7	3	A E		716 2,7	43	0 74	4	A E
				-,-												_,						_,,				

					2	045 No Bı	uild AM					2045 Bui	ld AM				2	045 No Bu	uild PM					2045 Bui	ild PM		
No.	Intersection ID	Approach	Movement		nroughput ph)	Average (sec/		LC	S*		nroughput ph)	Averag (sec		LO	S*	Vissim Th (v)	roughput oh)	Average (sec/		LC	os*	Vissim Th (vp	roughput oh)	Averag (sec		LO	S*
		SB	LT RT	215	215	516	516	F	F	212	212	515	515	F	F	82	82	6	6	Α	Α	83	83	3	3	Α	Α
			LT																								
310	Deane Avenue NE	EB	TH	0	4	-	394	-	F	0	4	-	297	-	F	0	0	-	-	-	-	0	0	-	-	-	1 -
	at Lee St NE	WB	TH	2	72	3	1		Α	2	73	1	1	-	А	4	120	1	1		А	0	127		1		А
			RT Intersection		91	20			^.	- 2	, 0	. 20	32		^	. 20		·	·		^	2			<u> </u>		
		NB	LT	7		288	268	F	F	7	000	331		F	F	18		83		F	D	18		101	70	F	-
		NB	RT	251	258	268	268	F	F	229	236	296	297	F	F	274	292	51	53	D	- 0	217	235	69	72	Е	E
		SB	LT	583	796	257	258	F	F	543	767	281	281	F	F	198	315	46	47	D	D	254	374	71	69	E	Е
311	Deane Avenue NE at Kenilworth	35	TH	213	790	259	200	F	г	224	707	281	201	F	Г	118	315	48	47	D	U	119	3/4	65	69	E	
311	Terrace NE	EB	TH RT	150 84	235	68 38	57	E D	Е	147 81	228	71 36	59	E D	Е	66 22	87	59 35	53	E	D	68 21	89	56 22	48	E C	D
			LT	271		6		A		253		36 6		A		163		35		D A		187		22		A	
		WB	TH	58	328	1	5	A	Α	58	312	1	5	A	Α	108	270	0	1	A	A	116	303	0	1	A	Α
			Intersection	1,0	616	17	79	ı		1,5	42	19	95	Ė			35	3	6		Ď	1,0	01		7	ĺ	)
		SB	LT TH	111 104	216	308 320	314	F	F	107 96	203	288 300	294	F	F	333 26	359	53 51	53	D D	D	339 28	366	82 86	83	F	F
	Deane Avenue NE		TH	707		11		В		670		11		В		497		14		В		497		16		В	
312	at Kenilworth	EB	RT	364	1,071	2	8	A	Α	341	1,012	2	8	A	Α	41	538	5	14	A	В	43	540	5	15	A	В
	Avenue NE	WB	LT	505	833	82	66	F	Е	515	826	83	69	F	Е	304	575	14	21	В	С	352	655	46	35	D	D
			TH Intersection	328	119	40		D		312		46		D		271		29		С		303		23	39	С	
			U	128	119	45	2	D		127	J4 I	39		D		147	12	333	0	F	1	146	001	284	9	F	
		NB	LT	59	668	77	21	Е	С	58	656	70	18	E	В	47	483	350	339	F	F	51	495	301	287	F	F
	Nannie Helen	ND	TH	53		65		E		51		62		E		289		340		F		298		286		F	
240	Burroughs Avenue		RT LT	428 193	428	70	70	E	E	421	421	69	69	E	E	438 144	438	440	440	F	F	457	457	378	378	F	F
313	NE at Kenilworth Avenue NE and DC-	EB	TH	649	843	40 39	40	D D	D	183 622	804	52 38	41	D D	D	691	835	39 32	33	D C	С	152 690	842	49 32	35	D C	D
	295 U-turns	WB	TH	794	1,320	38	36	D	D	789	1,317	40	36	D	D	532	1.047	46	45	D	D	603	1,213	28	27	C	С
			RT	527		32		С	D	529		30		С		515	1,047	44	45	D	J 0	610	-	26		С	
			Intersection LT	219	831	444	4	F	)	221	778	442	4	E [	)	380	302	211	54	E	F	466	800	71	25	E	
		NB	TH	130	406	280	364	F	F	133	412	258	355	F	F	403	892	166	182	F	F	459	1,053	42	52	D	D
			RT	57		253		F		58		250		F		109		139		F	1	128		18		В	
		SB	TH	302	624	237	366	F	F	254	559	359	458	F	F	150	328	130	130	F	F	153	331	115	112	F	F
	Nannie Helen		RT LT	322 112		487 51		F D	-	304 112		542 53		F D		178 123		130 32		F C	<u> </u>	179 130		109 26		F C	·
314	Burroughs Avenue NE at Minnesota	EB	TH	544	1,075	34	29	C	С	526	1.041	35	31	D	С	672	1,128	34	31	C	С	691	1.149	32	29	C	С
	Avenue NE		RT	419	1 .,	17		В	-	403	.,	18		В	_	332	.,	23		C	1 -	328	.,	23		C	
			LT	53		93		F		70		186		F		64		396		F		79		225		F	1
		WB	TH	726	786	46 47	49	D	D	728	805	49	60	D	E	439	516	525	512	F	F	563	660	213	216	F	F
			RT Intersection	7	891	47	55	-		6	316	40	71	-		13	65	626	76	F	F	18	94	277	4	F	
																,						J., .					

	lutare attento	A			2	045 No Bı	ıild AM					2045 Buil	d AM				2	045 No B	ild PM					2045 Buil	ld PM		
No.	Intersection ID	Approach	Movement	(vr	nroughput ph)	Average (sec/		LO	S*	(v	roughput ph)	Average (sec/		LO	S*	Vissim Th (vr		Averag (sec		LO	S*	(v)	roughput oh)	Average (sec/		LOS	S*
		NB	LT	124	226	69	55	E	Е	127	235	81	66	F	Е	117	261	443	360	F	_	163	369	46	37	D	D
		NB	RT	41	220	32	55	С	_	40	233	41	00	D	_	83	201	290	300	F		115	305	25	31	С	D
			LT	4		40		D		4		40		D		21		59		E		25		31		С	
		SB	TH	212	360	33	32	С	С	211	360	33	32	С	С	77	152	67	81	E	F	57	152	30	28	С	С
		02	SBR - NHB EB	49	000	27	02	С		50	000	27	02	С		29	.02	128	٥.	F		37	.02	19	20	В	ŭ
			SBR - Hunt	95		33		С		94		33		С		26		89		F		35		33		С	
	Nannie Helen		LT	20		33		С		20		34		С		26		35		D		27		22		С	
	Burroughs Avenue	EB	TH	445	607	14	15	В	В	434	592	14	16	В	В	506	794	21	23	С	С	521	816	18	20	В	В
315	NE at 44th Street		EBR - Hunt Street	5		11		В		4		11	-	В		0		-		-		3		22		С	
	NE and Hunt Place		EBR - 44th Street NBL- NHB EB	136		16		В		134		18		В		262		26		С		265		23		С	
	NE	Hunt Street	NBL- NHB EB	25	49	45 40	45	D	-	24		42 53	45	D	D	93	202	306	004	r	-	99	222	85	82	-	-
		Hunt Street	NBU - 44th NB NBT - 44th SB	13	49	40	45	- D	D	6 12	51	53 47	45	- D	D	0 46	202	258	281	- F	F	50	222	79	82	- E	F
	-		WBL - Hunt Street	60		108		F		59		136		F		32		434		F		43		84		F	
		-	WBL - 44th Street	6		104		F		5		121		F		4		408		F		4		75		E	
		WB	TH	564	643	79	82	Ē	F	562	639	101	105	F	F	254	302	403	404	F	F	345	407	40	44	D	D
			RT	13		75		F		13		95		F		12		339		F		16		25		C	
			ntersection		385	4	7	_	)	1.8	378	5	6			1.7	12	17	7			1.9	166	3	6	D	)
			TH	1,224		227		F		1,151		227		F		488		2		Α		459		2		Α	
	Kenilworth Avenue	SB	RT	45	1,269	183	226	F	D	44	1,195	176	225	F	D	89	577	3	2	Α	A	86	545	3	2	A	A
316	NE at Foote Street NE	EB	RT	241	241	16	16	В	В	229	229	15	15	В	В	214	214	9	9	A	A	214	214	9	9	A	Α
	INE	İ	ntersection	1,5	510	19	12	F		1,4	23	19	91	F		79	91	4		F	A .	7	59	4		À	
		NB	LT	107	707	23.2	10.6	С	В	105	699	24.8	12.8	С	В	17	939	12	12	-	В	0	919	-	11	-	В
		ND	TH	600	707	8.4	10.6	Α	ь	594	699	10.6	12.0	В	ь	922	939	12	12	В	ь	919	919	11	- 11	В	ь
	Minnesota Avenue	SB	TH	988	998	24.9	24.8	С	С	956	966	12.7	12.7	В	В	628	662	4	1	Α	Δ	626	660	3	3	Α	Δ
219	NE at DOES	08	RT	9	330	15.2	27.0	В	٠	9	300	5.8	12.7	Α	ь	34	002	1	+	A	^	34	000	1	3	A	^
	Parking Garage	EB	LT	0	0	-	-	-	_	0	0	-	_	-	_	10	192	20	18	В	В	10	188	21	20	С	В
			RT	0		-		-		0		-				182		18		В		178		20		В	
			ntersection	1,7	704	1	9	Е	3		9	6	i	F	A .	22	25	1	5	Е	3	2:	22	1	7	В	

<sup>\*</sup>Simulated level of service is approximated based on delay but is not equivalent to that produced using Highway Capacity Manual methodology.

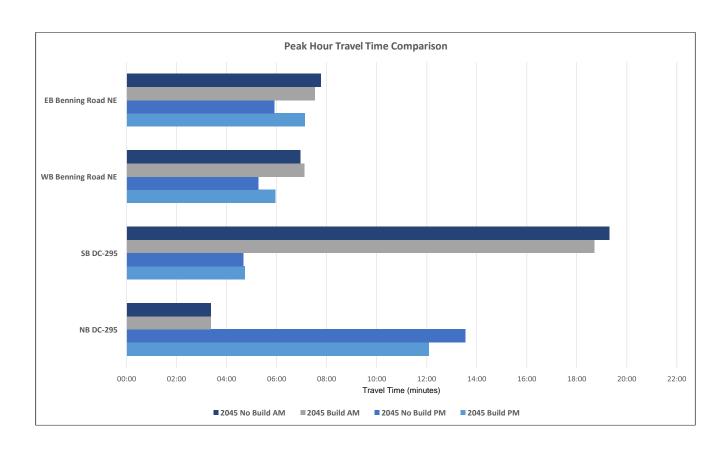
\*The intersections are numbered based on their inclusion in both the Streetcar and IMR projects (100s), the Streetcar project (200s), and the IMR project (300s).

\*MOEs represent an average of 10 simulation runs.

# 8.1.1.3 Travel Time Comparison

# 8.1.1.3.1 AM Peak Period (7:00 AM - 9:00 AM) PM Peak Period (4:30 PM - 6:30 PM)

Route	2045 No Build AM	2045 Build AM	2045 No Build PM	2045 Build PM
NB DC-295 from East Capitol Street to Benning Road NE	00:37	00:37	05:58	05:01
NB DC-295 from Benning Road NE to Nannie Helen Burroughs Ave NE	00:44	00:44	03:33	02:47
NB DC-295 from Nannie Helen Burroughs Ave NE to Polk Street NE	00:45	00:45	02:14	02:04
NB DC-295 from Nelson Place NE to East Capitol Street Bridge	01:16	01:16	01:48	02:13
NB DC-295	03:22	03:22	13:33	12:05
SB DC-295 from Polk Street NE to Lane Place NE	03:10	03:04	00:31	00:31
SB DC-295 from Lane Place NE to Nannie Helen Burroughs Ave NE	01:29	01:27	00:13	00:13
SB DC-295 from Nannie Helen Burroughs Ave NE to Benning Road NE	06:50	06:30	00:45	00:45
SB DC-295 from Benning Road NE to East Capitol Street	03:55	03:47	00:50	00:52
SB DC-295 from East Capitol Street NE to N Street NE	03:54	03:54	02:21	02:23
SB DC-295	19:18	18:42	04:40	04:44
WB Benning Road NE from East Capitol Street to Minnesota Ave NE	03:16	03:20	02:53	03:11
WB Benning Road NE from Minnesota Ave NE to 36th Street NE	00:42	00:42	00:39	00:39
WB Benning Road NE from 36th Street NE to Anacostia Ave NE	00:51	01:08	00:41	00:51
WB Benning Road NE from Anacostia Ave NE to 26th Street NE	02:08	02:59	01:03	02:40
WB Benning Road NE	06:57	07:07	05:16	05:57
EB Benning Road NE from 26th Street NE to Anacostia Ave NE	01:13	01:22	01:20	01:24
EB Benning Road NE from Anacostia Ave NE to 36th Street NE	00:59	01:05	00:59	00:59
EB Benning Road NE from 36th Street NE to Minnesota Ave NE	00:49	00:48	00:46	02:19
EB Benning Road NE from Minnesota Ave NE to East Capitol Street	04:45	04:16	02:50	03:03
EB Benning Road NE	07:46	07:31	05:55	07:08

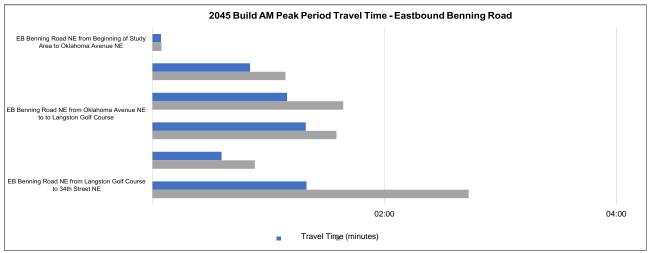


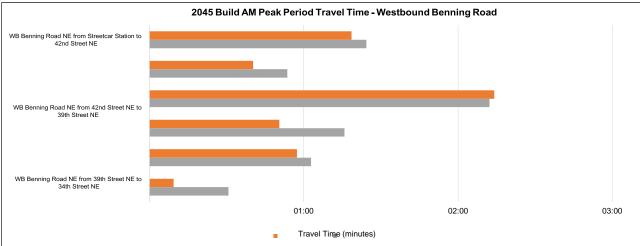
#### 8.1.1.4 2045 Build AM Streetcar Travel Time Comparison

#### 8.1.1.4.1 AM Peak Period (7:00 AM - 9:00 AM) PM Peak Period (4:30 PM - 6:30 PM)

Streetcar Travel Time Results	•		
Route	Vehicular Travel Time (MM:SS)	Streetcar Travel Time (MM:SS)*	Difference (MM:SS)
WB Benning Road NE from Streetcar Station to 42nd Street NE	01:19	01:24	00:06
WB Benning Road NE from 42nd Street NE to 39th Street NE	00:40	00:54	00:13
WB Benning Road NE from 39th Street NE to 34th Street NE	02:14	02:12	00:02
WB Benning Road NE from 34th Street NE to Langston Golf Course	00:51	01:16	00:25
WB Benning Road NE from Langston Golf Course to Oklahoma Avenue NE	00:57	01:03	00:06
WB Benning Road NE from Oklahoma Avenue NE to End of Study Area	00:09	00:31	00:21
WB Benning Road NE	06:10	07:20	01:09
EB Benning Road NE from Beginning of Study Area to Oklahoma Avenue NE	00:04	00:05	00:00
EB Benning Road NE from Oklahoma Avenue NE to Langston Golf Course	00:51	01:09	00:18
EB Benning Road NE from Langston Golf Course to 34th Street NE	01:10	01:39	00:29
EB Benning Road NE from 34th Street NE to 39th Street NE	01:19	01:35	00:16
EB Benning Road NE from 39th Street NE to 42nd Street NE	00:36	00:53	00:17
EB Benning Road NE from 42nd Street NE to Streetcar Station	01:20	02:44	01:24
EB Benning Road NE	05:20	08:05	02:45

<sup>\*</sup>Streetcar travel time does not include dwell time at stops (assuming 20 seconds per stop, on average)



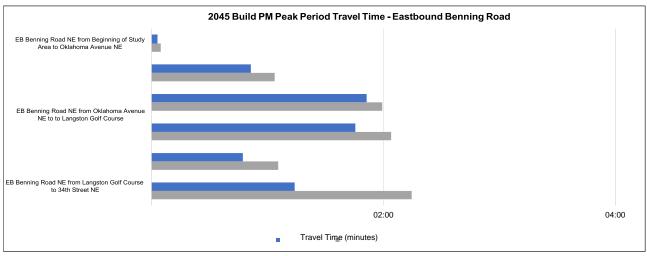


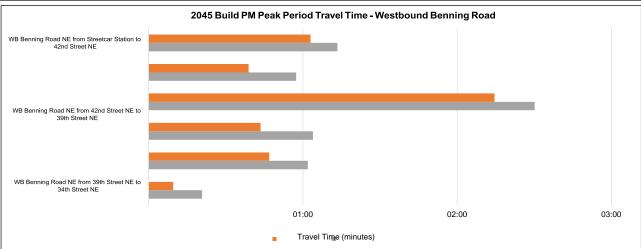
#### 8.1.1.5 2045 Build PM Streetcar Travel Time Comparison

#### 8.1.1.5.1 AM Peak Period (7:00 AM - 9:00 AM) PM Peak Period (4:30 PM - 6:30 PM)

Streetcar Travel Time Results			
Route	Vehicular Travel Time (MM:SS)	Streetcar Travel Time (MM:SS)*	Difference (MM:SS)
WB Benning Road NE from Streetcar Station to 42nd Street NE	01:03	01:13	00:10
WB Benning Road NE from 42nd Street NE to 39th Street NE	00:39	00:57	00:19
WB Benning Road NE from 39th Street NE to 34th Street NE	02:15	02:30	00:16
WB Benning Road NE from 34th Street NE to Langston Golf Course	00:44	01:04	00:20
WB Benning Road NE from Langston Golf Course to Oklahoma Avenue NE	00:47	01:02	00:15
WB Benning Road NE from Oklahoma Avenue NE to End of Study Area	00:10	00:21	00:11
WB Benning Road NE	05:36	07:08	01:31
EB Benning Road NE from Beginning of Study Area to Oklahoma Avenue NE	00:03	00:05	00:02
EB Benning Road NE from Oklahoma Avenue NE to Langston Golf Course	00:51	01:04	00:12
EB Benning Road NE from Langston Golf Course to 34th Street NE	01:51	01:59	00:08
EB Benning Road NE from 34th Street NE to 39th Street NE	01:45	02:04	00:19
EB Benning Road NE from 39th Street NE to 42nd Street NE	00:47	01:06	00:18
EB Benning Road NE from 42nd Street NE to Streetcar Station	01:14	02:15	01:01
EB Benning Road NE	06:33	08:32	01:59

<sup>\*</sup>Streetcar travel time does not include dwell time at stops (assuming 20 seconds per stop, on average)





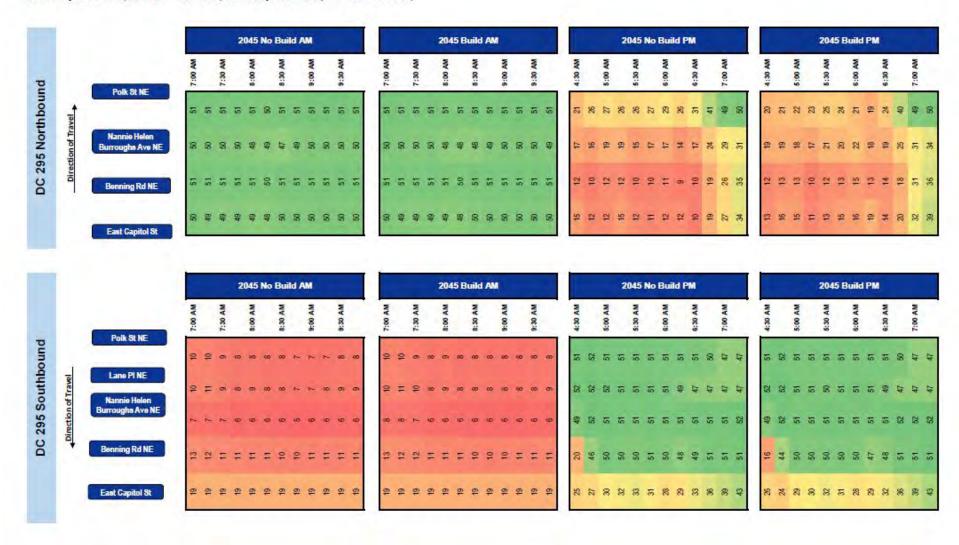
# 8.1.1.6 Queue Length Comparison

## 8.1.1.6.1 AM Peak Period (7:00 AM - 9:00 AM) PM Peak Period (4:30 PM - 6:30 PM)

			Vissim Max	Queue (feet)	
	Location	2045 No Build AM	2045 Build AM	2045 No Build PM	2045 Build PM
	Eastbound Benning Road NE to Southbound DC-295	0	0	290	37
	Eastbound Benning Road NE to Northbound DC-295	46	25	2655	2783
DC 295 and Benning Road NE Interchange	Southbound Kenilworth Avenue NE to Northbound DC-295	30	21	0	0
Road NE Interchange	Northbound DC-295 to Westbound Benning Road NE	704	382	0	0
	Northbound DC-295/Southbound Kenilworth Avenue NE to Westbound Benning Road NE	0	167	65	59
	Southbound Benning Road NE at East Capitol Street	903	913	522	459
Benning Road NE and East Capitol Street	Northbound Benning Road NE at East Capitol Street	555	862	292	271
Intersection	Westbound East Capitol Street at Benning Road NE	1776	1784	240	220
	Eastbound East Capitol Street at Benning Road NE	247	320	1335	1334
East Capitol Street and Texas Ave	Northbound Texas Avenue at East Capitol Street	540	541	204	232
	Southbound Texas Avenue at East Capitol Street (left turn from East Capitol to Texas)	86	79	278	238
	Eastbound Benning Road NE at Minnesota Avenue NE	382	304	535	777
Benning Road NE and	Westbound Benning Road NE at Minnesota Avenue NE	389	416	313	330
Minnesota Avenue NE	Northbound Minnesota Avenue NE at Benning Road NE	478	403	373	482
	Southbound Minnesota Avenue NE at Benning Road NE	532	549	454	482

## Speed Heat Map Comparison

AM Analysis Period (7:00 AM - 10:00 AM) PM Analysis Period (4:30 PM - 7:30 PM)



## 8.1.1.6.2 2045 Build Curbside Alignment Morning Peak Hour Intersection Delay and Levels of Service (LOS)

Intersection	Traffic	Interse	ection	Northb Appro		Southb Appro		Westb Appr			ound
intersection	Control	Delay	LOS	Delay	LOS	Delay	LOS	Delay	LOS	Delay	LOS
Benning Road and 26 <sup>th</sup> Street	Signalized	8.6	A	-	-	29.0	С	7.2	A	10.0	В
Benning Road and Oklahoma Avenue	Signalized	30.9	С	22.9	С	-	-	37.5	D	12.5	В
Benning Road and Anacostia Avenue	Signalized	7.6	A	33.8	C	30.8	C	4.0	A	13.0	В
Benning Road and 34 <sup>th</sup> Street	Signalized	13.2	В	15.7	В	34.8	C	12.8	В	10.7	В
Benning Road and Minnesota Avenue	Signalized	72.6	Е	62.7	Е	104.6	F	69.6	Е	51.3	D
Benning Road and 42 <sup>nd</sup> Street	Signalized	12.2	В	39.8	D	36.6	D	8.1	A	6.9	A
Benning Road and 45 <sup>nd</sup> Street	Signalized	14.0	В	8.3	A	25.1	C	25.4	C	31.1	C
Benning Road and Central Avenue*	Unsignalized	12.4	В	-	1	-	-	12.4	В	1	1
Benning Road and E Capitol Street	Signalized	188.9	F	255.5	F	74.6	Е	197.9	F	286.6	F
Minnesota Avenue and Dix Street	Signalized	16.1	В	18.2	В	11.3	В	21.6	C	19.5	В
Minnesota Avenue and Grant Street	Signalized	16.9	В	14.8	В	12.3	В	41.2	D	ı	ı
Minnesota Avenue and Gault Place*	Unsignalized	21.9	C	0.5	A	1.8	A	21.9	C	18.6	C
Minnesota Avenue and Hayes Street*	Unsignalized	19.2	C	1.8	A	0.9	A	19.2	C	-	į
Minnesota Avenue and Nannie Helen Burroughs Avenue	Signalized	42.0	D	71.2	Е	31.4	С	28.2	С	41.1	D
Benning Road and 44 <sup>th</sup> Street	Signalized	24.4	С	24.9	С	12.6	В	36.6	D	-	-

<sup>\*</sup> Indicates unsignalized intersections, for which intersection LOS is expressed in terms of the average vehicle delay of the worst approach delay Note: Benning Road is considered east-west at all intersections except for at 44th Street, 45th Street, and E Capitol Street where it is considered to be north-south.

### 8.1.1.6.3 2045 Build Curbside Alignment Morning Peak Hour Maximum Queue Length (feet)

Intographica	N	orthbou	nd	S	outhbour	ıd	V	Vestboun	d		Eastboun	d
Intersection	L	T	R	L	T	R	L	T	R	L	T	R
Benning Road and 26 <sup>th</sup> Street	-	-	-	140	-	166	-	301	284	109	141	-
Benning Road and Oklahoma Avenue	114	-	96	-	-	-	983	983	-	-	231	236
Benning Road and Anacostia Avenue	199	-	189	68	-	-	306	306	306	196	196	181
Benning Road and 34 <sup>th</sup> Street	163	163	171	142	142	110	346	399	399	49	241	216
Benning Road and Minnesota Avenue	545	264	264	1,344	1,344	1,346	-	747	747	478	324	324
Benning Road and 42 <sup>nd</sup> Street	129	129	144	224	224	239	238	238	252	148	148	179
Benning Road and 45 <sup>nd</sup> Street	262	262	262	224	224	224	77	77	77	40	40	40
Benning Road and Central Avenue	-	-	-	-	-	-	-	-	332	-	-	-
Benning Road and E Capitol Street	1,076	1,076	1,076	350	350	360	1,674	1,674	1,674	1,467	1,467	1,467
Minnesota Avenue and Dix Street	347	347	347	199	199	199	144	144	144	24	24	24
Minnesota Avenue and Grant Street	102	251	251	267	267	267	288	-	288	-	-	-
Minnesota Avenue and Gault Place	-	63	63	128	123	-	55	-	51	108	104	107
Minnesota Avenue and Hayes Street	97	92	92	15	4	4	47	28	50	-	-	-
Minnesota Avenue and Nannie Helen Burroughs Avenue	630	630	630	-	304	322	278	278	298	504	504	505
Benning Road and 44 <sup>th</sup> Street	-	484	484	188	188	-	387	-	386	-	-	-

### 8.1.1.6.4 2045 Build Curbside Alignment Evening Peak Hour Intersection Delay and Levels of Service (LOS)

	Traffic	Inters	ection	North		Southb		Westh		Eastb	
Intersection	Control			Appr	oach	Appro	oach	Appr	oach	Appr	
	Control	Delay	LOS	Delay	LOS	Delay	LOS	Delay	LOS	Delay	LOS
Benning Road and 26 <sup>th</sup> Street	Signalized	11.9	В	-	-	41.6	D	4.3	A	12.9	В
Benning Road and Oklahoma Avenue	Signalized	7.7	A	23.1	C	1	-	9.7	A	5.9	A
Benning Road and Anacostia Avenue	Signalized	4.9	A	33.4	C	44.6	D	2.7	A	4.9	A
Benning Road and 34 <sup>th</sup> Street	Signalized	21.9	С	19.9	В	37.4	D	12.2	В	25.9	C
Benning Road and Minnesota Avenue	Signalized	53.9	D	44.6	D	46.1	D	54.3	D	61.8	Е
Benning Road and 42 <sup>nd</sup> Street	Signalized	13.7	В	35.5	D	35.1	D	5.2	A	12.2	В
Benning Road and 45 <sup>nd</sup> Street	Signalized	41.0	D	3.6	A	70.5	Е	28.8	C	28.4	C
Benning Road and Central Avenue*	Unsignalized	4.8	A	-	-	-	-	4.8	A	-	-
Benning Road and E Capitol Street	Signalized	214.1	F	445.9	F	54.3	D	424.1	F	158.3	F
Minnesota Avenue and Dix Street	Signalized	13.5	В	9.4	A	15.4	В	23.8	C	18.9	В
Minnesota Avenue and Grant Street	Signalized	15.7	В	17.6	В	9.1	A	30.6	C	-	-
Minnesota Avenue and Gault Place*	Unsignalized	18.4	С	1.3	A	1.4	A	13.1	В	18.4	C
Minnesota Avenue and Hayes Street*	Unsignalized	24.5	С	3.1	A	4.0	A	24.5	С	-	-
Minnesota Avenue and Nannie Helen	Cionelizad	65.9	Е	68.1	Е	31.9	С	27.1	С	92.8	F
Burroughs Avenue	Signalized	03.9	E	06.1	E	31.9	C	27.1	C	92.0	Г
Benning Road and 44 <sup>th</sup> Street	Signalized	50.6	D	19.6	В	78.8	Е	30.3	C	-	-

<sup>\*</sup> Indicates unsignalized intersections, for which intersection LOS is expressed in terms of the average vehicle delay of the worst approach delay Note: Benning Road is considered east-west at all intersections except for at 44th Street, 45th Street, and E Capitol Street where it is considered to be north-south.

# 8.1.1.6.5 2045 Build Curbside Alignment Evening Peak Hour Maximum Queue Length (feet)

Intongostica	N	orthbour	ıd	So	uthbou	ınd	7	Westbour	ıd		Eastbo	und
Intersection	L	T	R	L	T	R	L	T	R	L	T	R
Benning Road and 26 <sup>th</sup> Street	-	-	-	204	-	231	-	198	181	22	342	-
Benning Road and Oklahoma Avenue	112	-	95	-	-	-	118	118	-	-	290	293
Benning Road and Anacostia Avenue	113	-	103	11	-	-	116	116	116	325	325	311
Benning Road and 34 <sup>th</sup> Street	141	141	149	75	75	33	207	153	153	13	639	614
Benning Road and Minnesota Avenue	258	333	333	527	527	247	-	284	284	1,507	1,531	1,531
Benning Road and 42nd Street	144	144	159	186	186	201	140	140	154	432	432	466
Benning Road and 45 <sup>nd</sup> Street	156	156	156	459	459	459	108	60	88	29	29	29
Benning Road and Central Avenue	-	-	-	-	-	-	-	-	79	-	-	-
Benning Road and E Capitol Street	1,072	1,072	1,072	364	364	374	1,673	1,673	1,673	1,465	1,465	1,465
Minnesota Avenue and Dix Street	209	209	209	322	322	322	165	165	165	90	90	90
Minnesota Avenue and Grant Street	86	696	696	173	173	173	114	-	114	-	-	-
Minnesota Avenue and Gault Place	-	164	164	108	103	-	55	-	51	100	98	92
Minnesota Avenue and Hayes Street	138	133	133	34	19	19	42	23	46	-	-	-
Minnesota Avenue and Nannie Helen Burroughs Avenue	811	811	811	-	192	210	207	207	227	952	952	952
Benning Road and 44 <sup>th</sup> Street	-	275	275	810	810	-	228	-	228	-	-	-